

TABULATION OF QUANTITIES																						
PAY ITEM NO.	DESCRIPTION	UNIT	SHEET NUMBERS														TOTAL THIS SHEET		GRAND TOTAL			
			S - 4																			
			PLAN	FINAL	PLAN	FINAL	PLAN	FINAL	PLAN	FINAL	PLAN	FINAL	PLAN	FINAL	PLAN	FINAL	PLAN	FINAL	PLAN	FINAL		
0700 1 11B	SINGLE POST SIGN, F&I, UP TO 12 SF	AS	1														1		1			
0700 1 12C	SINGLE POST SIGN, F&I,12-20 SF	AS	4														4		4			
0700 1 60	SINGLE POST SIGN, REMOVE	AS	17														17		17			
0705 10 3	OBJECT MARKER, TYPE 3	AS	4														4		4			
0706 1 12	RAISED PAVEMENT MARKER, CLASS B, MONO OR BI DIRECTIONAL, ALL COLORS	EA	55														55		55			
0710 11121	PAINTED PAVEMENT MARKINGS (STANDARD, WHITE, SOLID, 6")	LF	1085														1085		1085			
0710 11123	PAINTED PAVEMENT MARKINGS (STANDARD, WHITE, SOLID, 12")	LF	314														314		314			
0710 11124	PAINTED PAVEMENT MARKINGS (STANDARD, WHITE, SOLID, 18")	LF	34														34		34			
0710 11125	PAINTED PAVEMENT MARKINGS (STANDARD, WHITE, SOLID, 24")	LF	44														44		44			
0710 11221	PAINTED PAVEMENT MARKINGS (STANDARD, YELLOW, SOLID, 6")	LF	952														952		952			
0710 11222	PAINTED PAVEMENT MARKINGS (STANDARD, YELLOW, SOLID, 18")	LF	10														10		10			
0710 11241	PAINTED PAVEMENT MARKINGS (STANDARD) (YELLOW) (SKIP) (6")	LF	64														64		64			
0711 11121	THERMOPLASTIC, WHITE, SOLID, 6"	LF	1085														1085		1085			
0711 11123	THERMOPLASTIC, WHITE, SOLID, 12"	LF	314														314		314			
0711 11124	THERMOPLASTIC, WHITE, SOLID, 18"	LF	34														34		34			
0711 11125	THERMOPLASTIC, WHITE, SOLID, 24"	LF	44														44		44			
0711 11221	THERMOPLASTIC, YELLOW, SOLID, 6"	LF	952														952		952			
0711 11224	THERMOPLASTIC, YELLOW, SOLID, 18"	LF	10														10		10			
0711 11241	THERMOPLASTIC, YELLOW, SKIP, 6"	LF	64														64		64			
0713 103101A	PERMANENT TAPE, WHITE, SOLID, 6" FOR CONCRETE BRIDGES	LF	90														90		90			
0713 103201A	PERMANENT TAPE, YELLOW, SOLID, 6" FOR CONCRETE BRIDGES	LF	90														90		90			

THE QUANTITIES SHOWN ARE FOR ONE APPLICATION; SEE SPECIFICATION 710 FOR THE NUMBER OF APPLICATIONS REQUIRED.

REVISIONS				ASA CONSULTANTS, INC. 8935 NW 35th LANE, SUITE 200 DORAL, FL 33172 (754) 216-2027 CARLOS M. RODRIGUEZ, PE; LIC. NO. 79134	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			TABULATION OF QUANTITIES	SHEET NO.
DATE	DESCRIPTION	DATE	DESCRIPTION						
					ROAD NO.	COUNTY	PROJECT NUMBER		
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190196		S-2

THE OFFICIAL RECORD OF THIS SHEET IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, F.A.C.

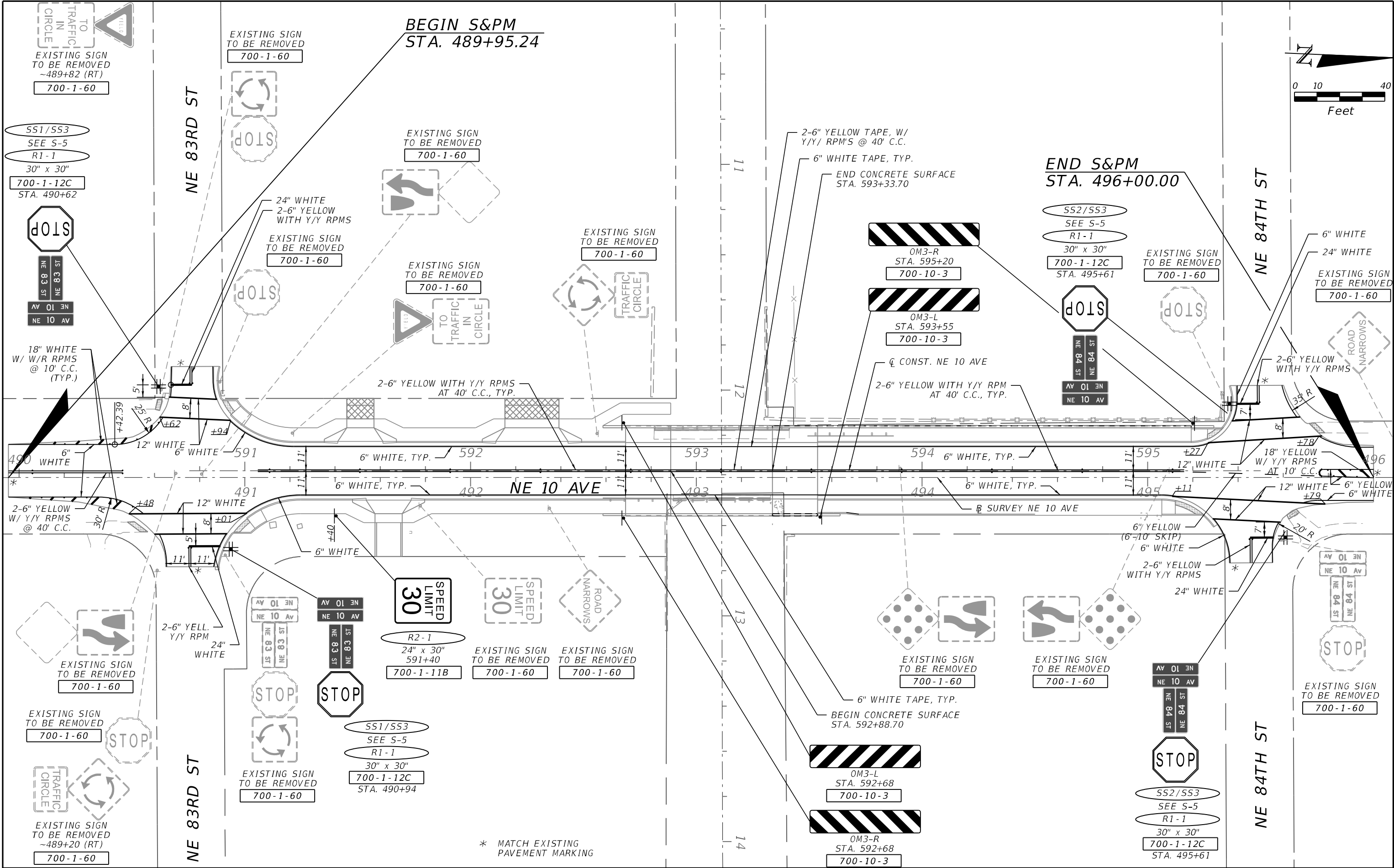
GENERAL NOTES:

1. DELIVER AND UNLOAD REMOVED STREET SIGNS TO:
MIAMI-DADE COUNTY DEPARTMENT OF TRANSPORTATION AND PUBLIC WORKS, TRAFFIC SIGNALS AND SIGN DIVISION
- 7100 NW 36TH STREET
MIAMI, FLORIDA 33166

PAY ITEM NOTES:

700-1-60 INCLUDES COST OF DELIVERING AND UNLOADING REMOVED STREET SIGNS TO MIAMI-DADE COUNTY.

REVISIONS				ASA CONSULTANTS, INC. 8935 NW 35th LANE, SUITE 200 DORAL, FL 33172 (754) 216-2027 CARLOS M. RODRIGUEZ, PE; LIC. NO. 79134	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			GENERAL NOTES	SHEET NO.
DATE	DESCRIPTION	DATE	DESCRIPTION		ROAD NO.	COUNTY	PROJECT NUMBER		S-3
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190196		



REVISIONS				ASA CONSULTANTS, INC. 8935 NW 35th LANE, SUITE 200 DORAL, FL 33172 (754) 216-2027 CARLOS M. RODRIGUEZ, PE; LIC. NO. 79134	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SIGNING & PAVEMENT MARKING PLAN		SHEET NO. S-4
DATE	DESCRIPTION	DATE	DESCRIPTION		ROAD NO.	COUNTY	PROJECT NUMBER			
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190196			

MIAMI-DADE COUNTY
DEPARTMENT OF TRANSPORTATION & PUBLIC WORKS

PLANS FOR PROPOSED BRIDGE REPLACEMENT

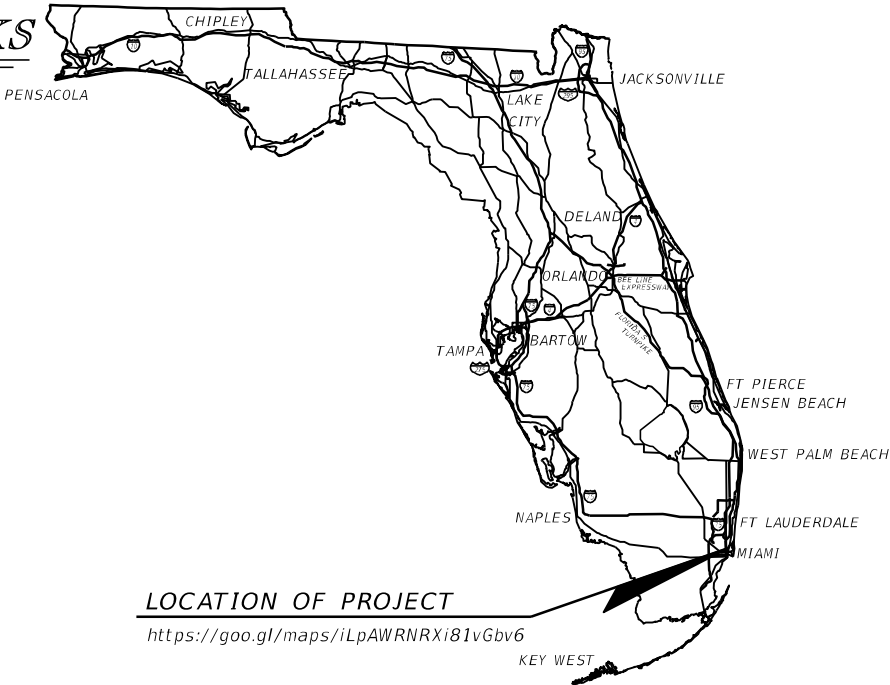
NE 10TH AVENUE BRIDGE 874178 REPLACEMENT
OVER THE ADEMAR CANAL (BRIDGE NO. 874185)
FROM NE 83RD STREET TO NE 84TH STREET

FUNDS: MOBILITY IMPACT FEE (M.I.F.)
MIAMI-DADE COUNTY DTPW PROJECT NO. EDP-MT-20190196

LIGHTING PLANS

INDEX OF LIGHTING PLANS

SHEET NO.	SHEET DESCRIPTION
L-1	KEY SHEET
L-2	SIGNATURE SHEET
L-3	LIGHTING GENERAL NOTES
L-4	POLE DATA & LEGEND
L-5	LIGHTING PLAN



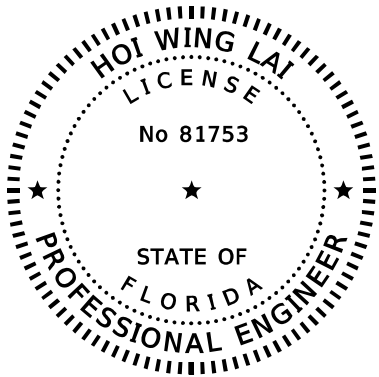
LIGHTING PLANS
ENGINEER OF RECORD:

HOI WING LAI, P.E.
P.E. LICENSE NO. 81753
STANLEY CONSULTANTS, INC.
1641 WORTHINGTON ROAD, SUITE 400
WEST PALM BEACH, FL. 33409
(561) 689-7444
CONTRACT NO. EDP-PSA-2017 R
VENDOR NO. VF-421320758-001

MIAMI-DADE COUNTY
PROJECT MANAGER:

JACQUELINE ALCINA, P.E.

FISCAL YEAR	SHEET NO.
26	L-1



THIS ITEM HAS BEEN DIGITALLY
SIGNED AND SEALED BY

Digitally signed
by Hoi Wing Lai
Date:
2024.12.19
11:14:44 -05'00'

ON THE DATE ADJACENT TO THE SEAL

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STANLEY CONSULTANTS, INC.
1641 WORTHINGTON ROAD, SUITE 400
WEST PALM BEACH, FLORIDA 33409
HOI WING LAI, P.E. NO. 81753

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE
FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

SHEET NO.	SHEET DESCRIPTION
L-1	KEY SHEET
L-2	SIGNATURE SHEET
L-3	LIGHTING GENERAL NOTES
L-4	POLE DATA & LEGEND
L-5	LIGHTING PLAN

REVISIONS				HOI WING LAI, P.E. P.E. LICENSE NO. 81753 STANLEY CONSULTANTS INC. 1641 WORTHINGTON ROAD, SUITE 400 WEST PALM BEACH, FLORIDA 33409	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SIGNATURE SHEET	SHEET NO.
DATE	DESCRIPTION	DATE	DESCRIPTION		ROAD NO.	COUNTY	PROJECT NUMBER		
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190916		
									L-2

LIGHTING GENERAL NOTES

1. FP&L IS THE MAINTAINING AGENCY FOR THE LIGHTING SYSTEM. MIAMI-DADE COUNTY LEASES THE LIGHTING SYSTEM FROM FP&L.
2. SEE THE LEAD (ROADWAY) COMPONENT'S GENERAL NOTES SHEET FOR AGENCY AND UTILITY OWNERS, AND THEIR CONTACT INFORMATION, AS WELL AS CONTACT INFORMATION FOR OTHER REQUIRED ACTIONS.
3. SEE THE LEAD (ROADWAY) COMPONENT'S VERIFIED UTILITY LOCATE SHEET FOR HORIZONTAL AND VERTICAL VERIFIED UTILITY LOCATE INFORMATION.

REVISIONS				HOI WING LAI, P.E. P.E. LICENSE NO. 81753 STANLEY CONSULTANTS INC. 1641 WORTHINGTON ROAD, SUITE 400 WEST PALM BEACH, FLORIDA 33409	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			LIGHTING GENERAL NOTES	SHEET NO.
DATE	DESCRIPTION	DATE	DESCRIPTION		ROAD NO.	COUNTY	PROJECT NUMBER		L-3
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190916		

POLE DATA

POLE NO.	STATION	DIST. OR BRACKET ARM	LUMINAIRE LUMENS	MOUNTING HEIGHT	PULLBOX/ SLAB TYPE	POLE OFFSET	PAY ITEM
1	591+87.70	6'	17,125	30'	BY FP&L	21.00' LT	

CONVENTIONAL
LIGHTING DESIGN CRITERIA

Average Maintained Intensity
at all locations:

Horizontal 0.5 Foot Candles
Uniformity Ratios Avg./Min. 4:1 Or Less

Wind Speed 160 MPH

LEGEND

SYMBOLS



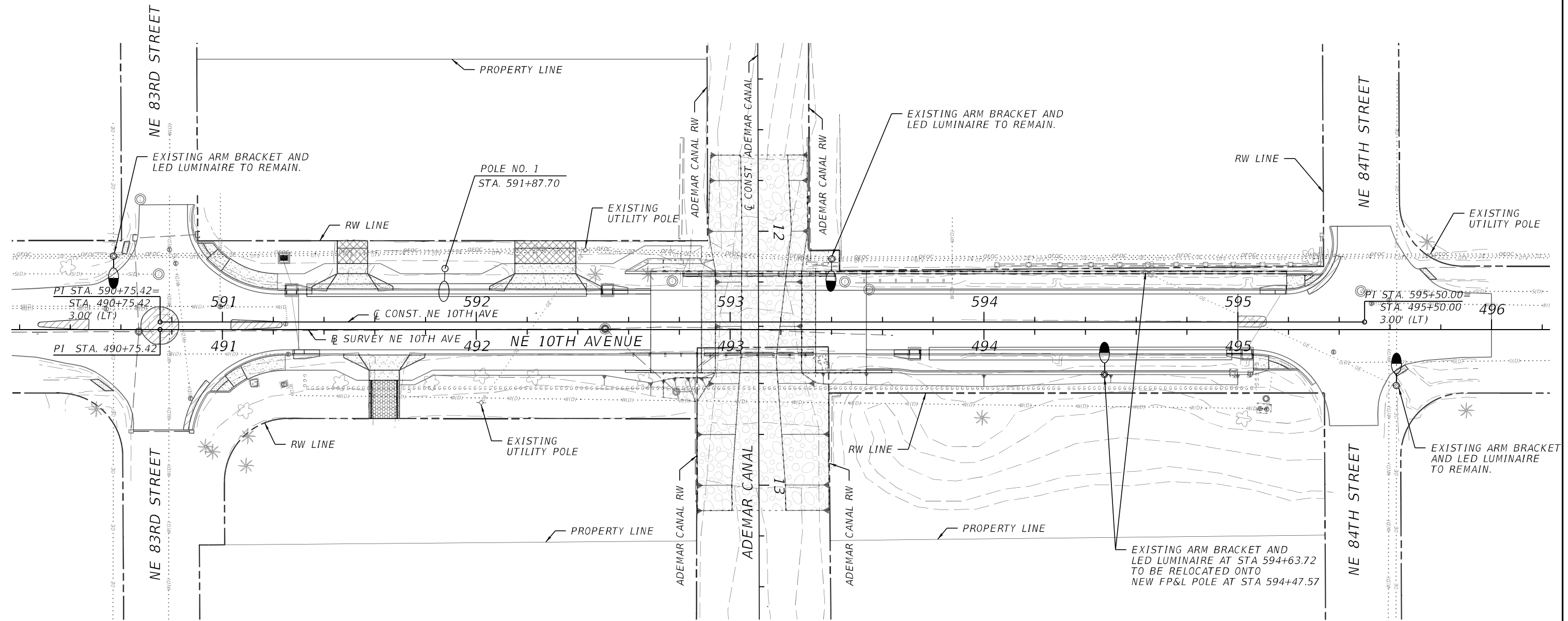
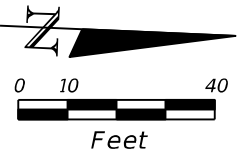
LED luminaire with 4,000k CCT, Type IV distribution and 120 volt operation mounted on new light pole.
Photometric curve ATB2_40X_MVOLT_R4_4K (17,125 Lumens) or equal.
See Pole Data Table for luminaire mounting height above the road surface.



Existing arm bracket and LED luminaire to remain.

DESCRIPTION

REVISIONS				HOI WING LAI, P.E. P.E. LICENSE NO. 81753 STANLEY CONSULTANTS INC. 1641 WORTHINGTON ROAD, SUITE 400 WEST PALM BEACH, FLORIDA 33409	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			POLE DATA & LEGEND	SHEET NO.
DATE	DESCRIPTION	DATE	DESCRIPTION		ROAD NO.	COUNTY	PROJECT NUMBER		
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190916		L-4



REVISIONS				HOI WING LAI, P.E. P.E. LICENSE NO. 81753 STANLEY CONSULTANTS INC. 1641 WORTHINGTON ROAD, SUITE 400 WEST PALM BEACH, FLORIDA 33409	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			LIGHTING PLAN	SHEET NO.
DATE	DESCRIPTION	DATE	DESCRIPTION		ROAD NO.	COUNTY	PROJECT NUMBER		
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190916		L-5

MIAMI-DADE
DEPARTMENT OF TRANSPORTATION & PUBLIC WORKS

INDEX OF STRUCTURE PLANS

SHEET NO.	SHEET DESCRIPTION
B1-1	KEY SHEET
B1-2	SIGNATURE SHEET
BQ-1	SUMMARY OF QUANTITIES
B1-3	GENERAL NOTES
B1-4	SURFACE FINISH DETAILS
B1-5	SLOPE PROTECTION DETAILS
B1-6	PLAN AND ELEVATION
B1-7	TYPICAL SECTION
B1-8	BRIDGE HYDRAULICS RECOMMENDATIONS
B1-9	REPORT OF CORE BORINGS NE 10TH AVENUE BRIDGE
B1-10	FOUNDATION LAYOUT
B1-11	PILE DATA TABLE
B1-12	ACIP PILE DETAILS
B1-13	END BENT 1
B1-14	END BENT 2
B1-15	END BENT DETAILS
B1-16	FINISH GRADE ELEVATIONS
B1-17	BEAM TABLE OF VARIABLES
B1-18	SUPERSTRUCTURE PLAN
B1-19	SUPERSTRUCTURE SECTION
B1-20	SUPERSTRUCTURE DETAILS
B1-21	APPROACH SLAB DETAILS
B1-22	REINFORCING BAR LIST
B1-23	LOAD RATING SUMMARY SHEET
B1-24	RETAINING WALL DETAILS (1 OF 2)
B1-25	RETAINING WALL DETAILS (2 OF 2)

STANDARD PLANS FOR BRIDGE CONSTRUCTION

400-010	CANTILEVER RETAINING WALL (C.I.P)
415-001	STANDARD BAR BENDING DETAILS (STEEL)
450-450	FLORIDA SLAB BEAM TYPICAL DETAILS AND NOTES
450-452	15" FLORIDA SLAB BEAM
458-110	EXPANSION JOINT SYSTEM - POURED JOINT WITH BACKER ROD
515-021	PEDESTRIAN/BICYCLE BULLET RAILING FOR TRAFFIC RAILING
515-022	PEDESTRIAN/BICYCLE BULLET RAILING DETAILS
521-423	TRAFFIC RAILING - (32" VERTICAL SHAPE)

DEVELOPMENTAL STANDARD PLANS

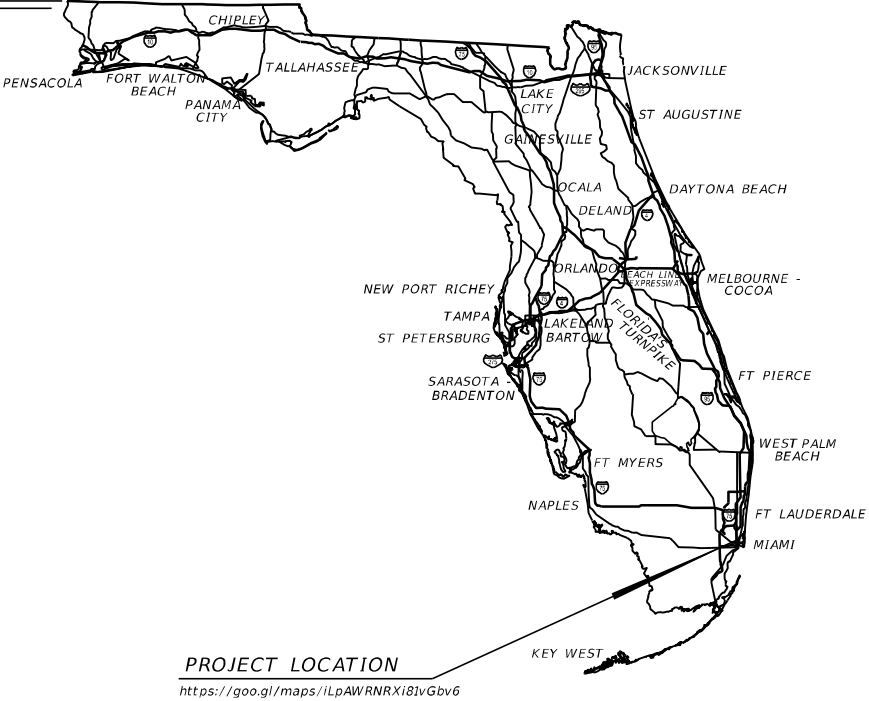
D400-092	APPROACH SLABS (20 FT) (FLEXIBLE PAVEMENT APPROACHES)
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PLANS FOR PROPOSED BRIDGE REPLACEMENT

NE 10TH AVENUE BRIDGE 874178 REPLACEMENT
OVER THE ADEMAR CANAL (BRIDGE NO. 874185)
FROM NE 83RD STREET TO NE 84TH STREET

FUNDS: MOBILITY IMPACT FEE (M.I.F.)
MIAMI-DADE COUNTY DTPW PROJECT NO. EDP-MT-20190196

STRUCTURE PLANS



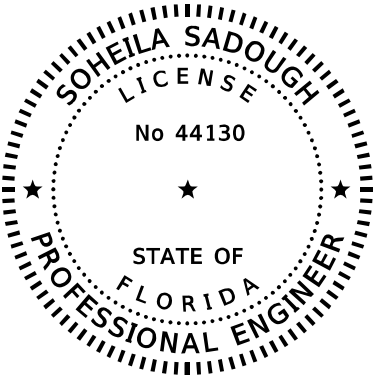
STRUCTURE PLANS
ENGINEER OF RECORD:

SOHEILA SADOUGH, P.E.
P.E. NO.: 44130
ASA CONSULTANTS, INC.
510 SHOTGUN ROAD, SUITE 402
SUNRISE, FL 33326
CONTRACT NO.: CA424
VENDOR NO.: F465512620

MIAMI-DADE COUNTY
PROJECT MANAGER:

JAQUELINE ALCINA, PE

FISCAL YEAR	SHEET NO.
26	B1-1



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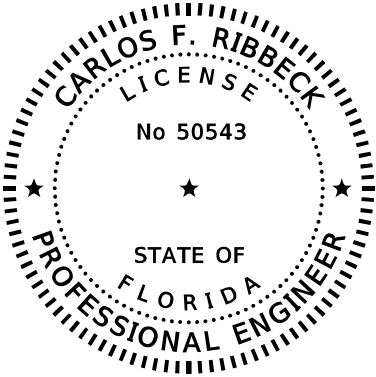
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ASA CONSULTANTS, INC.
510 SHOTGUN ROAD, SUITE 402
SUNRISE, FLORIDA 33326
SOHEILA SADOUGH, P.E. NO. 44130

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE
FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

STRUCTURES PLANS

SHEET NO.	SHEET DESCRIPTION
B1-1	KEY SHEET
B1-2	SIGNATURE SHEET
BQ-1	SUMMARY OF QUANTITIES
B1-3	GENERAL NOTES
B1-4	SURFACE FINISH DETAILS
B1-5	SLOPE PROTECTION DETAILS
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B1-23	LOAD RATING SUMMARY SHEET
B1-24	RETAINING WALL DETAILS (1 OF 2)
B1-25	RETAINING WALL DETAILS (2 OF 2)



THIS ITEM HAS BEEN DIGITALLY
SIGNED AND SEALED BY:

Carlos F Digitally signed by Carlos F
Ribbeck Ribbeck
Date: 2024.12.19 13:48:24
-05'00'

ON THE DATE ADJACENT TO THE SEAL

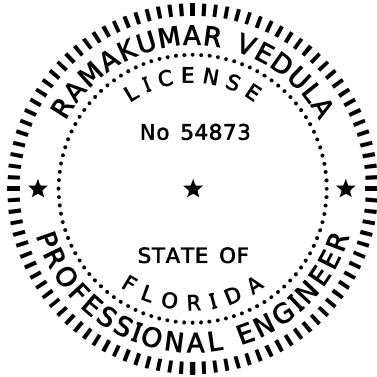
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RIBBECK ENGINEERING, INC.
14335 SW 120TH STREET, SUITE 205
MIAMI, FLORIDA 33186
CARLOS F. RIBBECK, PE. NO. 50543

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE
FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

STRUCTURES PLANS

SHEET NO.	SHEET DESCRIPTION
B1-2	SIGNATURE SHEET
B1-8	BRIDGE HYDRAULICS RECOMMENDATIONS



THIS ITEM HAS BEEN DIGITALLY
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Ramakumar V Vedula Digitally signed by Ramakumar V Vedula
Date: 2024.12.19 09:34:02
-05'00'

ON THE DATE ADJACENT TO THE SEAL

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TIERRA SOUTH FLORIDA, INC.
2765 VISTA PARKWAY, SUITE 10
WEST PALM BEACH, FLORIDA 33411
RAMAKUMAR VEDULA, PE. NO. 54873

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE
FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

STRUCTURES PLANS

SHEET NO.	SHEET DESCRIPTION
B1-2	SIGNATURE SHEET
B1-9	REPORT OF CORE BORINGS NE 10TH AVENUE BRIDGE

BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07 - 20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07 - 20				SIGNATURE SHEET		
							DESIGNED BY: SS 07 - 20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
							CHECKED BY: RA 07 - 20	NE 10 AVE	MIAMI -DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET		B1 - 2

GENERAL NOTES

A. DESIGN SPECIFICATIONS

1. FDOT STRUCTURES MANUAL DATED JANUARY 2024 AND SUBSEQUENT STRUCTURES DESIGN BULLETINS.
2. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) LOAD AND RESISTANCE FACTOR (LRFD) BRIDGE DESIGN SPECIFICATIONS, [9TH] EDITION.
3. FDOT DESIGN MANUAL DATED JANUARY 2024 AND SUBSEQUENT ROADWAY DESIGN BULLETINS.

B. GOVERNING STANDARDS AND CONSTRUCTION SPECIFICATIONS

FLORIDA DEPARTMENT OF TRANSPORTATION, FY 2025-26 STANDARD PLANS AND REVISED INDEX DRAWINGS AS APPENDED HEREIN, AND FY 2025-26 STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AS AMENDED BY CONTRACT DOCUMENTS.

C. VERTICAL DATUM

NGVD 29

D. ENVIRONMENT

BRIDGE NUMBER	SUPERSTRUCTURE	SUBSTRUCTURE	
		CONCRETE	STEEL
874185	EXTREMELY	EXTREMELY	EXTREMELY

E. DESIGN METHODOLOGY

LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD USING STRENGTH, SERVICE, AND FATIGUE LIMIT STATES.

F. DESIGN LOADINGS

1. LIVE LOADS: HL-93 WITH DYNAMIC LOAD ALLOWANCE
2. DEAD LOADS:

32" VERTICAL SHAPE TRAFFIC RAILING 385 PLF

ALUMINUM BULLET RAILING 10 PLF

REINFORCED CONCRETE 150 PCF

FUTURE WEARING SURFACE 15 PSF
3. CONSTRUCTION LOADS:

FINISHING MACHINE LOAD: 11 KIPS

FINISHING MACHINE WHEEL LOCATION BEYOND THE EDGE OF DECK OVERHANG: 6 INCHES

CONSTRUCTION LIVE LOAD: 20 PSF EXTENDED OVER THE ENTIRE BRIDGE WIDTH AND 50-FEET IN LONGITUDINAL LENGTH CENTERED ON THE FINISHING MACHINE.

REMOVABLE DECK CANTILEVER TIMBER FORMS WITH OVERHANG BRACKETS: 15 PSF

LIVE LOAD AT OR NEAR THE OUTSIDE EDGE OF DECK DURING DECK CASTING: 75 PLF APPLIED AS A MOVING LOAD OVER A LENGTH OF 20 FEET.

CONSTRUCTION INACTIVE DESIGN WIND SPEED: 90 MPH


VELOCITY PRESSURE EXPOSURE COEFFICIENT (KZ): 0.85

CONSTRUCTION ACTIVE DESIGN WIND SPEED: 30 MPH
4. UTILITIES: NO ALLOWANCE FOR UTILITY LOADS HAS BEEN INCLUDED IN THE DESIGN.
5. C-I-P TOPPING THICKNESS: 6"

G. MATERIALS

1. REINFORCING STEEL: GRADE 60 CARBON STEEL PER SPECIFICATIONS SECTION 931 EXCEPT AS NOTED BELOW.

GRADE 75, 2205 STAINLESS STEEL FOR AUGER-CAST PILES, END BENT CAPS, FLORIDA SLAB BEAMS (BARS 3C, & 5E ONLY) AND BARS 5T3 (REFER TO SUPERSTRUCTURE PLAN SHEET)
2. CONCRETE:

CONCRETE CLASS	MIN. 28-DAY COMPRESSIVE STRENGTH (PSI)	LOCATION OF CONCRETE IN STRUCTURE
II	3400	TRAFFIC RAILING
IV (C.I.P TOPPING)	5500	C.I.P TOPPING
IV	5500	C.I.P. SUBSTRUCTURE
GROUT-DEV455 ACP	5500	AUGER CAST PILES
VI	8500 	PRESTRESSED CONCRETE BEAMS
II (BRIDGE DECK)	4500	APPROACH SLAB

USE SHRINKAGE REDUCING ADMIXTURE FOR C-I-P TOPPING CONCRETE.

-  USE HIGHLY REACTIVE POZZOLANS (SILICA FUME, METAKAOLIN, OR ULTRA FINE FLY ASH PER SPECIFICATIONS SECTION 929)

3. CONCRETE COVER:

CAST-IN-PLACE SUPERSTRUCTURE (TOP OF DECK)	2"
CAST-IN-PLACE SUPERSTRUCTURE (EXCEPT TOP OF DECK)	2"
PRECAST PRESTRESED BEAMS (EXCEPT TOP SURFACE)	2"
CAST-IN-PLACE SUBSTRUCTURE (CAST AGAINST EARTH)	4"
CAST-IN-PLACE SUBSTRUCTURE (FORMED SURFACES)	4"

CONCRETE COVER DIMENSIONS SHOWN IN THE PLANS DO NOT INCLUDE PLACEMENT AND FABRICATION TOLERANCES UNLESS SHOWN AS "MINIMUM COVER". SEE SPECIFICATIONS SECTION 415 FOR ALLOWABLE TOLERANCES. ALL DIMENSIONS PERTAINING TO THE LOCATION OF REINFORCING STEEL ARE TO CENTERLINE OF BAR EXCEPT WHERE CLEAR DIMENSION IS NOTED TO FACE OF CONCRETE.

H. CONCRETE SURFACE FINISH

A CLASS 5 FINISH COATING SHALL BE APPLIED TO THE PORTIONS OF THE STRUCTURES SHOWN ON THE SURFACE FINISH DETAIL SHEET(S).

I. PLAN DIMENSIONS

ALL DIMENSIONS IN THESE PLANS ARE MEASURED IN FEET EITHER HORIZONTALLY OR VERTICALLY UNLESS OTHERWISE NOTED.

J. UTILITIES

FOR PLAN LOCATIONS OF EXISTING UTILITIES, SEE PLAN AND ELEVATION SHEET(S). LOCATIONS OF UTILITIES, SHOWN IN THE PLANS ARE APPROXIMATE. FOR DISPOSITION OF UTILITIES, SEE THE UTILITY ADJUSTMENT SHEET(S) IN THE ROADWAY PLANS.

K. BRIDGE NAME AND NUMBER

PLACE THE FOLLOWING BRIDGE NAME AND NUMBER ON THE TRAFFIC RAILINGS IN ACCORDANCE WITH THE TRAFFIC RAILING STANDARD PLANS:

NAME	NUMBER
NE 10TH AVE OVER ADEMAR CANAL	874185

L. SCREEDING DECKS

SCREED THE RIDING SURFACE OF THE BRIDGE DECK AND APPROACH SLABS TO ACHIEVE THE FINISH GRADE ELEVATIONS SHOWN IN THE PLANS. ACCOUNT FOR THEORETICAL DEFLECTIONS DUE TO SELF WEIGHT, DECK CASTING SEQUENCE, DECK FORMING SYSTEMS, CONSTRUCTION LOADS, OVERLAYS AND TEMPORARY SHORING, ETC. AS REQUIRED.

M. STAY-IN-PLACE DECK FORMS

NOT APPLICABLE.

N. JOINTS IN CONCRETE

CONSTRUCTION JOINTS WILL BE PERMITTED ONLY AT THE LOCATIONS INDICATED IN THE PLANS. ADDITIONAL CONSTRUCTION JOINTS OR ALTERATIONS TO THOSE SHOWN SHALL REQUIRE APPROVAL OF THE ENGINEER.

O. EXISTING BRIDGE CONSTRUCTION CONSIDERATIONS

1. DIMENSION VERIFICATION: UNLESS OTHERWISE NOTED, THE DIMENSIONS, ELEVATIONS AND INTERSECTING ANGLES SHOWN ARE BASED ON THE INFORMATION AS DETAILED IN THE ORIGINAL CONSTRUCTION PLANS OF THE EXISTING BRIDGES AND MAY NOT REPRESENT AS-BUILT CONDITIONS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THIS DATA BEFORE BEGINNING CONSTRUCTION AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES.

P. TRAFFIC CONTROL PLANS

SEE THE TRAFFIC CONTROL PLANS IN THE ROADWAY PLANS. REFER TO STANDARD PLANS INDEX 102-600 FOR SPECIFIC REQUIREMENTS RELATED TO OVERHEAD BRIDGE CONSTRUCTION.

Q. PHASING OF WORK




WORK PHASING AND PROGRESSION OF THE WORK SHALL CONFORM TO THE TRAFFIC CONTROL PLANS LOCATED IN THE ROADWAY PLANS AND THE NOTES ON THE DETOUR DRAWINGS.

R. C-I-P TOPPING

THOROUGHLY SATURATE THE TOP SURFACE OF THE FSB WITH WATER IN ACCORDANCE WITH SPECIFICATION SECTION 400 FOR 12 HOURS IMMEDIATELY PRIOR TO PLACING THE C-I-P TOPPING. REMOVE STANDING WATER PRIOR TO PLACING THE C-I-P TOPPING. CURE THE C-I-P TOPPING IN ACCORDANCE WITH THE SPECIFICATION 400 REQUIREMENTS FOR BRIDGE DECKS.

EXISTING BRIDGE DEMOLITION: DEMOLISH EXISTING BRIDGE IN ACCORDANCE WITH SPECIFICATION 110-6.

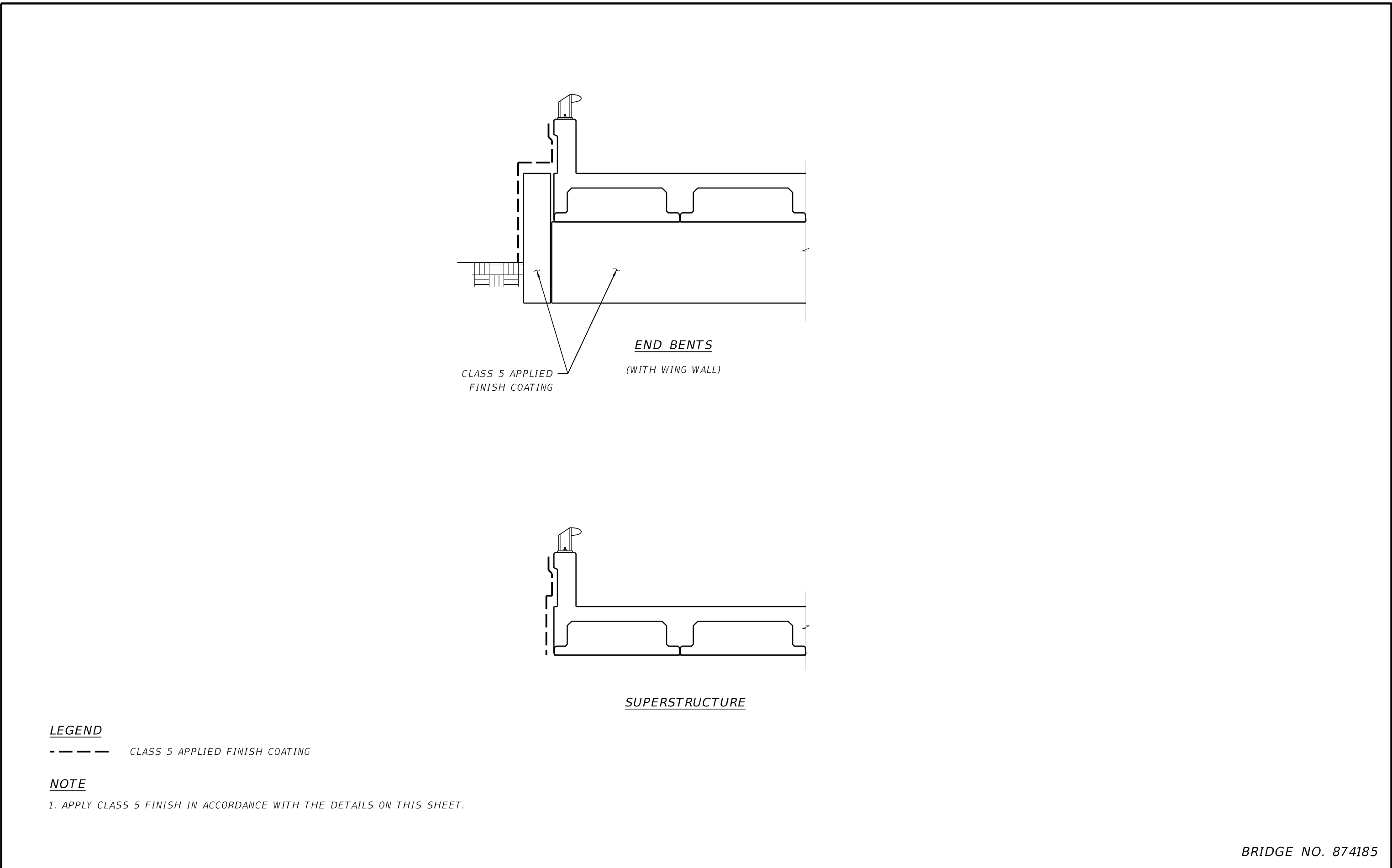
FAA COORDINATION

THIS PROJECT LIES WITHIN 10 NAUTICAL MILES OF AN AIRPORT. ALL PERMANENT FEATURES OF THE PROJECT HAVE BEEN FOUND IN COMPLIANCE WITH FEDERAL AVIATION ADMINISTRATION (FAA) 14 CFR PART 77 ( 77.7). DURING CONSTRUCTION, COMPLY WITH FEDERAL AVIATION ADMINISTRATION (FAA) 14 CFR PART 77 ( 77.7). THE CONTRACTOR SHALL CONTACT APPROPRIATE FAA PERSONNEL TO COORDINATE SUCH COMPLIANCE FOR CONSTRUCTION OPERATIONS AND EQUIPMENT TO BE USED ON THE PROJECT SITE. BE AWARE THAT 14 CFR PART 77 ( 77.7) ESTABLISHES THAT NOTIFICATION MUST BE SUBMITTED 45-DAYS PRIOR TO CONSTRUCTION ACTIVITIES WHICH MAY IMPACT AIRPORT-CONTROLLED AIRSPACE OR FACILITIES. GIVEN THE TIME REQUIRED TO CONDUCT AN AERONAUTICAL STUDY, A 45-60 DAY ADVANCE FILING IS RECOMMENDED TO ACCOMMODATE THE REVIEW PROCESS AND ALLOW TIMELY ISSUANCE OF THE FAA DETERMINATION LETTER. THE CONTRACTOR SHALL FILE THE REQUIRED NOTICE OF CONSTRUCTION COMMENCEMENT FORM (7460-2 PART 1) PRIOR TO STARTING WORK. THE CONTRACTOR SHALL FILE THE REQUIRED FORM (7460-2 PART 2) WITHIN 5-DAYS OF COMPLETION OF THE HIGHEST ELEMENT OF PERMANENT CONSTRUCTION. SEE [HTTPS://WWW.FAA.GOV/AIRPORTS/CENTRAL/ENGINEERING/PART77/](https://www.faa.gov/airports/central/engineering/part77/) FOR ADDITIONAL INFORMATION.

REQUEST THAT AVIATION AND SEAPORTS OFFICE PERSONNEL BE INVITED TO THE PRECONSTRUCTION CONFERENCE TO DISCUSS. SEE THE UTILITIES AND AGENCY CONTACTS LISTING FOR AVIATION AND SEAPORTS OFFICE PERSONNEL CONTACT INFORMATION, AND FOR CONTACT INFORMATION FOR OTHER REQUIRED ACTIONS.

BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: GENERAL NOTES	REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION							
								ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME: NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET	SHEET NO. B1-3
								NE 10 AVE	MIAMI -DADE	EDP-MT-20190196		



LEGEND

----- CLASS 5 APPLIED FINISH COATING

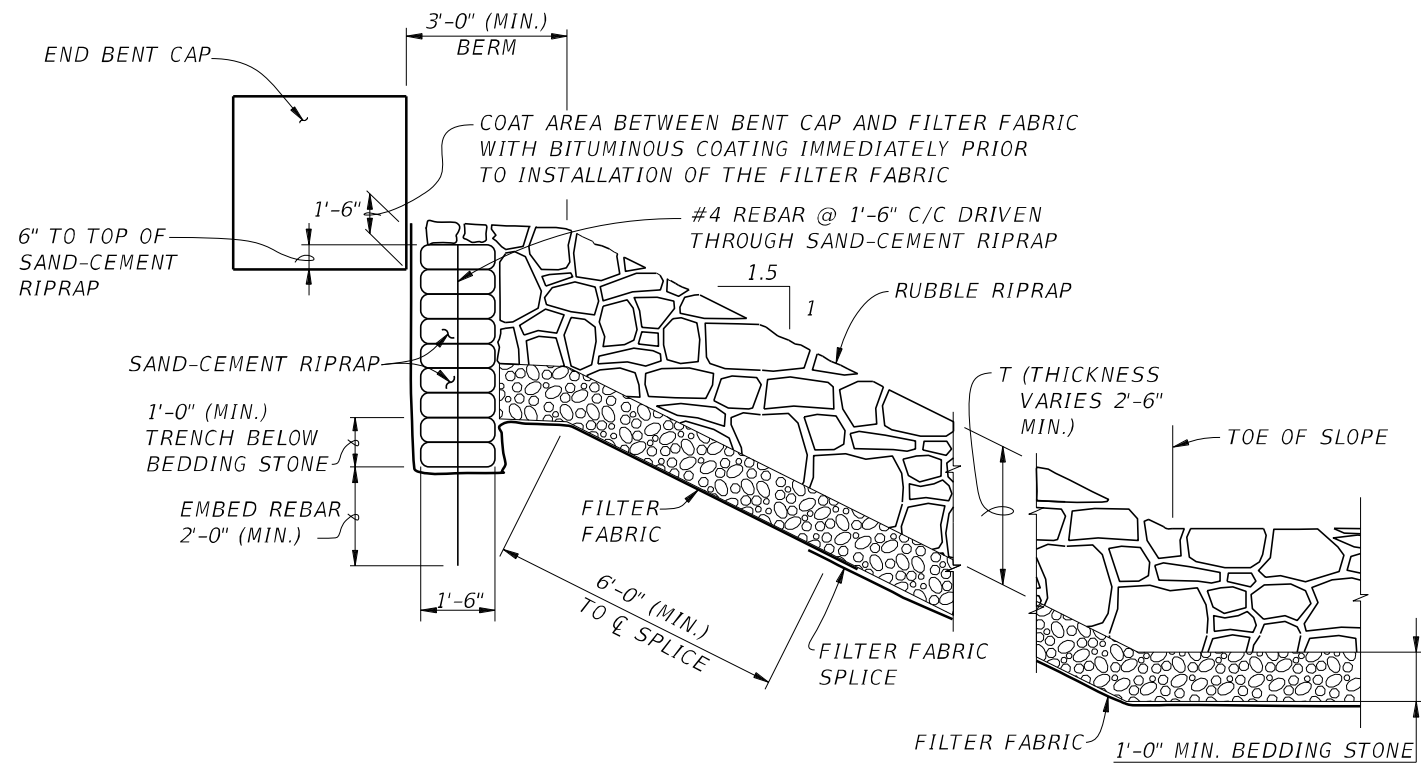
NOTE

1. APPLY CLASS 5 FINISH IN ACCORDANCE WITH THE DETAILS ON THIS SHEET.

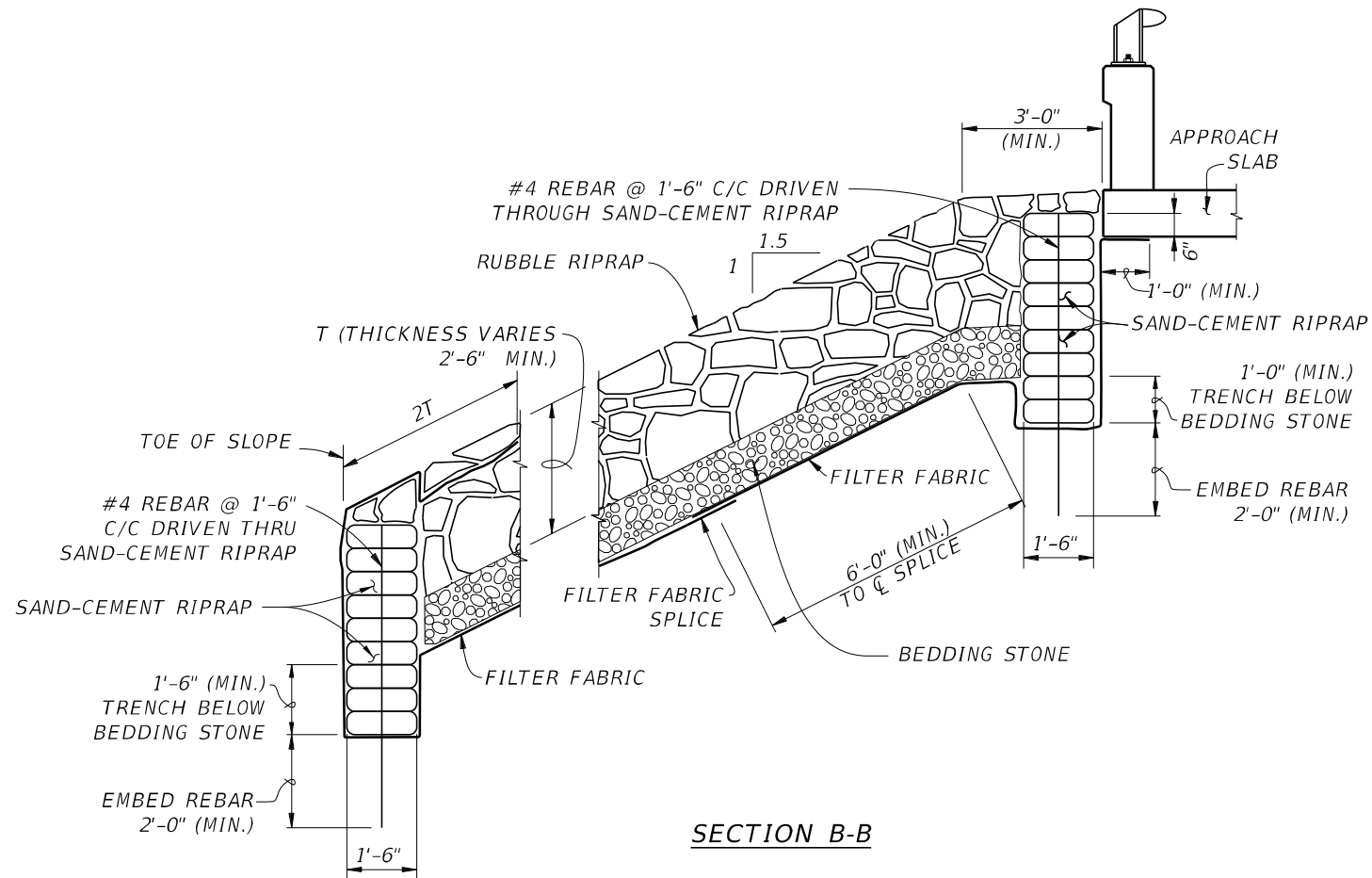
BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION						SURFACE FINISH DETAILS		
								ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
								NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET		B1-4

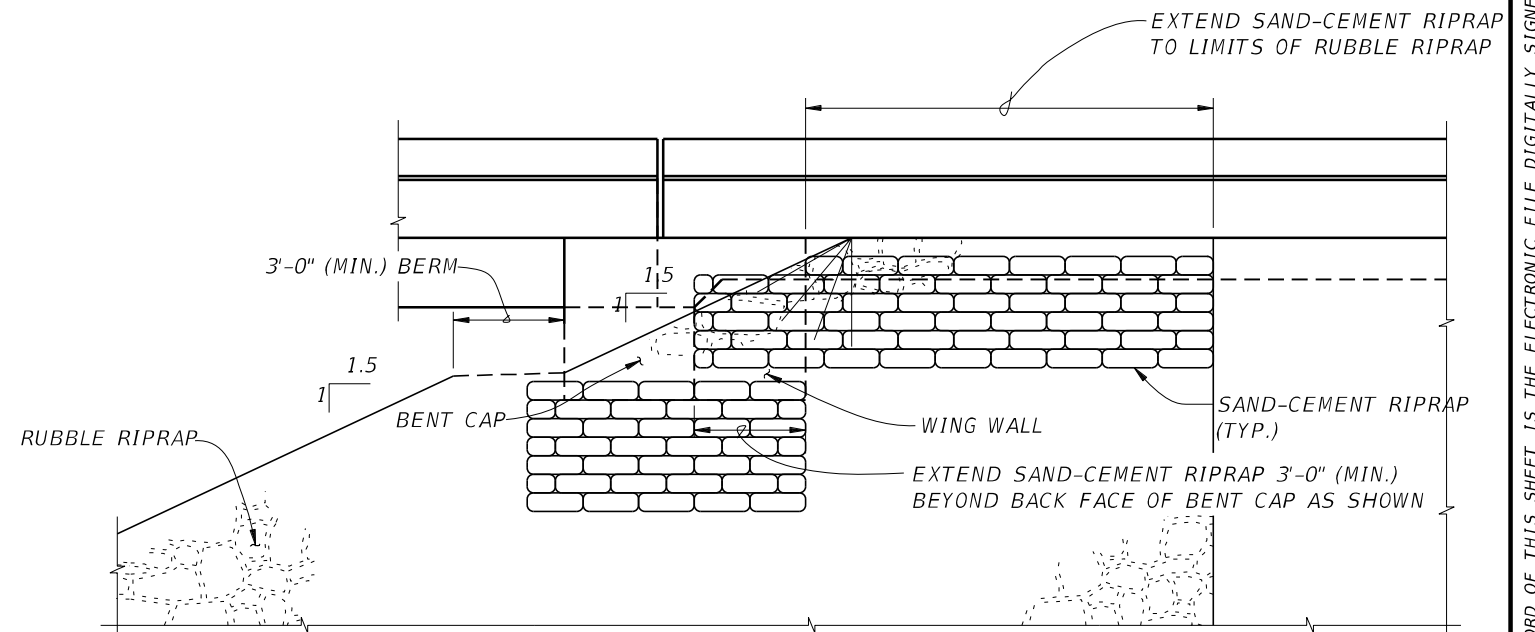
THE OFFICIAL RECORD OF THIS SHEET IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, F.A.C.



SECTION A-A



SECTION B-B



PARTIAL SIDE ELEVATION

NOTES:

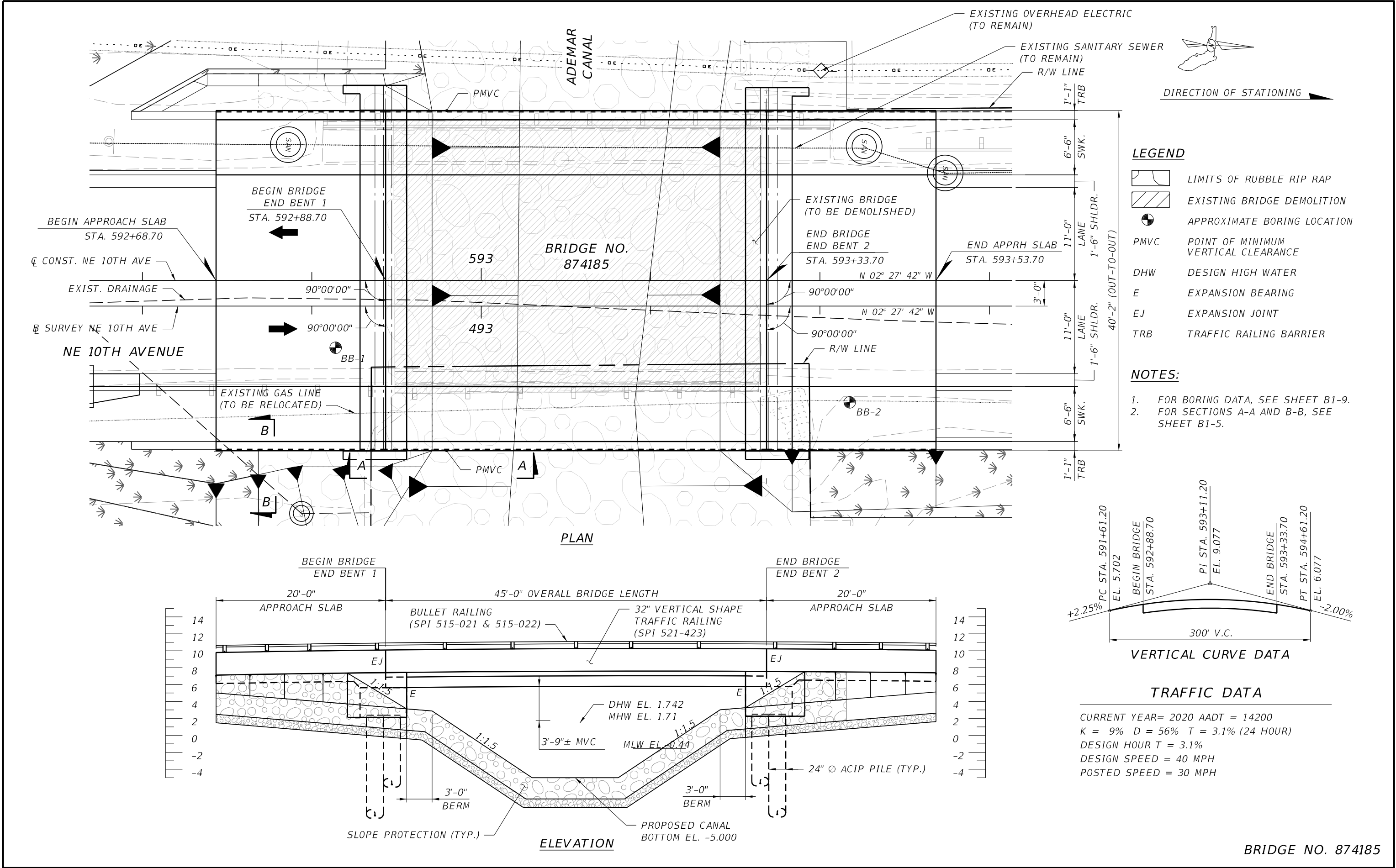
FILTER FABRIC SHALL BE TYPE D-2, IN ACCORDANCE WITH SPECIFICATIONS SECTION 985. SPLICE LENGTH SHALL BE IN ACCORDANCE WITH SPECIFICATIONS SECTION 514.

FOR LOCATION OF SECTION A-A & B-B, SEE SHEET B1-6.

COST OF #4 REBAR SHALL BE INCIDENTAL TO THE COST OF THE RUBBLE RIPRAP.

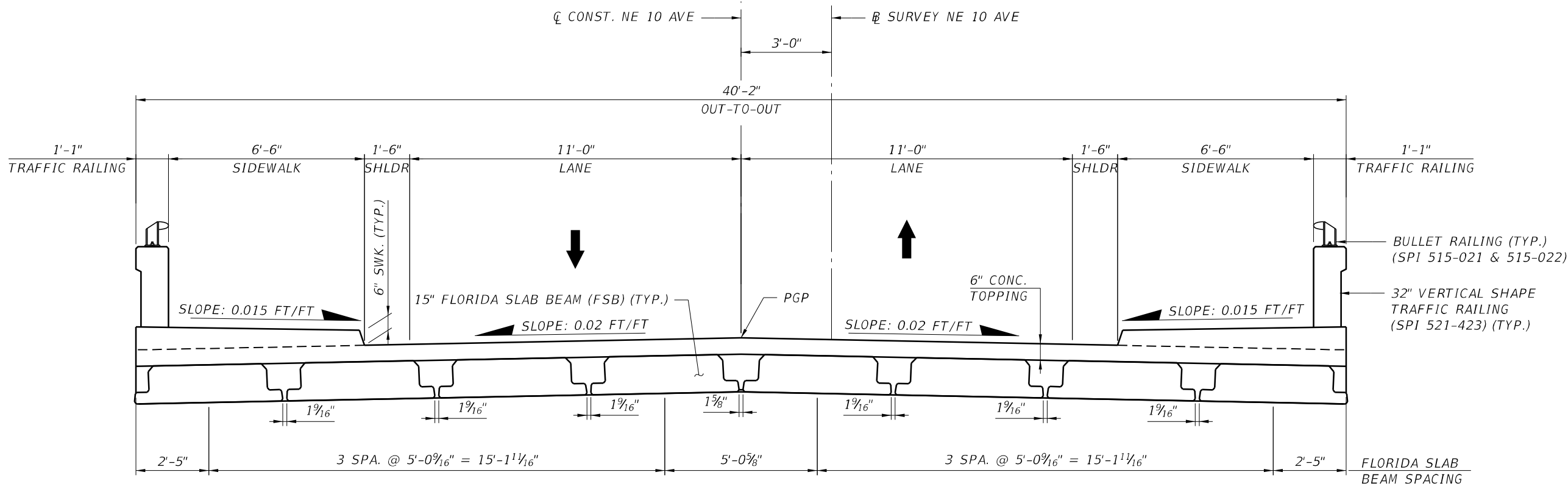
BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION						SLOPE PROTECTION DETAILS		
								ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	SHEET NO.	
								NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET	B1-5	



REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: PLAN AND ELEVATION		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20						
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
							CHECKED BY: RA 07-20	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET		B1-6

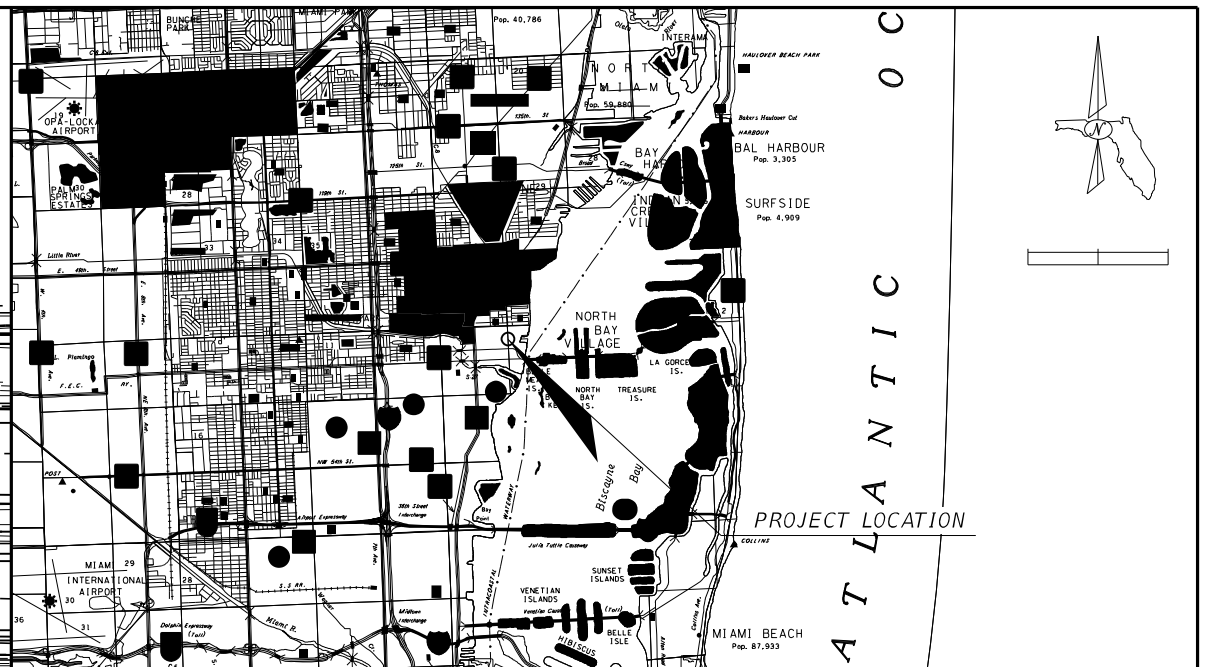
THE OFFICIAL RECORD OF THIS SHEET IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, F.A.C.



TYPICAL SECTION

BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20				TYPICAL SECTION		
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	SHEET NO.	
							CHECKED BY: RA 07-20	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET	B1-7	



NOTE:
 The hydraulic data is shown for informational purposes only to indicate the flood discharges and water surface elevations which may be anticipated in any given year. This data was generated using highly variable factors determined by a study of the watershed. Many judgements and assumptions are required to establish these factors. The resultant hydraulic data is sensitive to changes, particularly antecedent conditions, urbanization, channelization and land use. Users of this data are cautioned against the assumption of precision which cannot be obtained.

TERMS:
Design Flood: Utilized to assure a desired level of hydraulic performance.
Base Flood: Has a 1% chance of being exceeded in any given year (100 year frequency)
Overtopping Flood: Causes flow over the highway, over a watershed divide, or thru emergency relief structures.
Greatest Flood: The most severe that can be predicted where overtopping is not practicable.

WATER SURFACE ELEVATIONS: N.H.W. (Non-Tidal) _____ M.H.W. (Tidal) 0.20 NAVD (1.74 NGVD)
 CONTROL (Non-Tidal) _____ M.L.W. (Tidal) -1.98 NAVD (-0.44 NGVD)

FLOOD DATA: MAX. EVENT OF RECORD DESIGN FLOOD BASE FLOOD
 STAGE ELEV. NAVD (ft) _____ REM #1 _____ REM #1 _____
 DISCHARGE (cfs) _____ N/A _____ N/A _____
 AVERAGE VELOCITY (f/s) _____ N/A _____ N/A _____
 EXCEEDANCE PROB. (%) _____ N/A _____ N/A _____
 FREQUENCY (yr.) _____ N/A _____ N/A _____

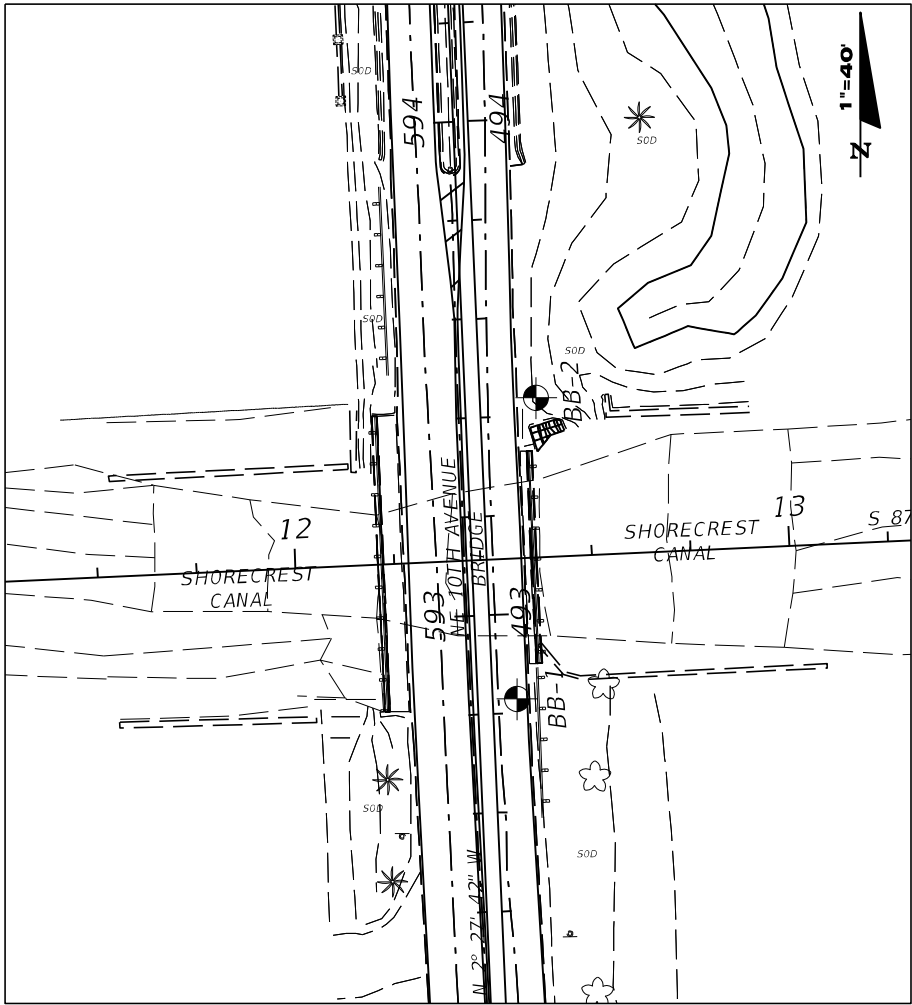
☐ OVERTOPPING or
☐ GREATEST FLOOD
 REM #1 _____

SCOUR PREDICTIONS FOR PROPOSED STRUCTURE DESCRIBED ABOVE:

PIER INFORMATION		TOTAL SCOUR ELEVATION		
NUMBERS	SIZE AND TYPE	LONG TERM SCOUR ELEV. REM #2	WORST CASE < 100 yr. FREQ. (yr.)	WORST CASE < 500 yr. FREQ. (yr.)
N/A	N/A		N/A	N/A

REMARKS:	(1) THE BRIDGE IS AT THE END OF A DEAD-END CANAL THAT DOES NOT DRAIN A SIZABLE WATERSHED. FLOWS ARE LIMITED TO THE EBB OF THE TIDE.
	(2) THE PROPOSED BRIDGE DOES NOT CONSTRICT THE EXISTING CANAL. BOTTOM OF THE CANAL UNDER THE BRIDGE WILL BE EXCAVATED IN NATURAL LIMESTONE LAYER. NO SIGNIFICANT SCOUR IS ANTICIPATED.

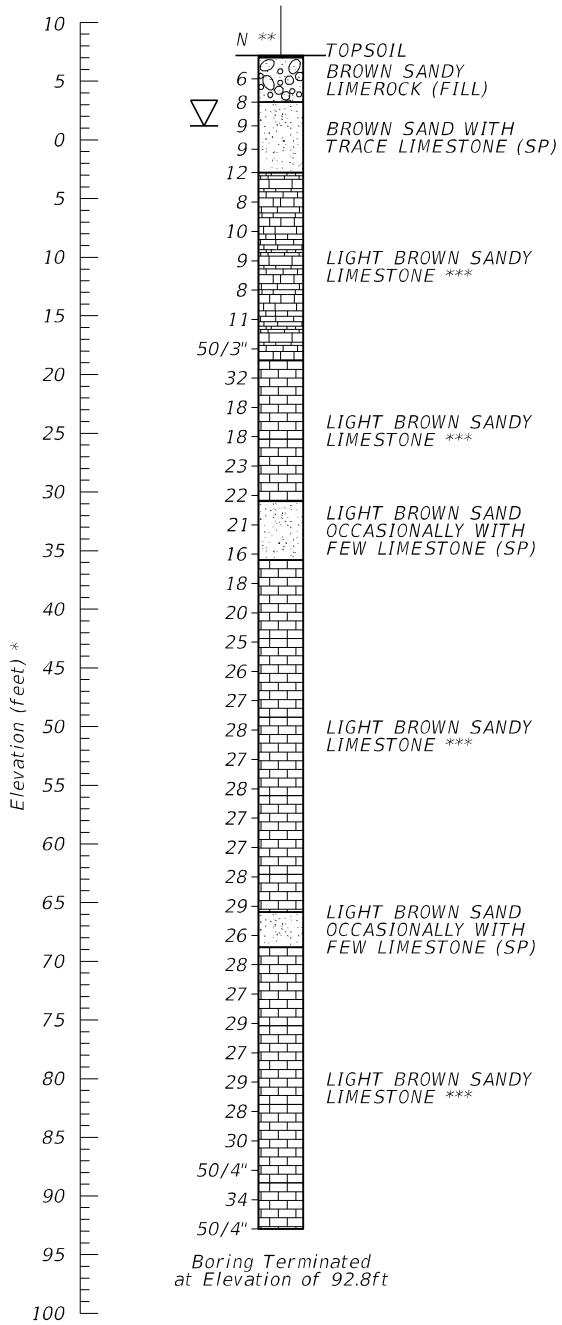
REVISIONS				RIBBECK ENGINEERING, INC. 14335 SW 120 STREET, SUITE 205 MIAMI, FLORIDA 33186 TEL. (305) 383-5909 FBPE C.A. NO. 27592 CARLOS F. RIBBECK, P.E. LICENSE NO. 50543	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			BRIDGE HYDRAULIC RECOMMENDATIONS	SHEET NO.
DATE	DESCRIPTION	DATE	DESCRIPTION		ROAD NO.	COUNTY	PROJECT NUMBER		B1-8
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190196		



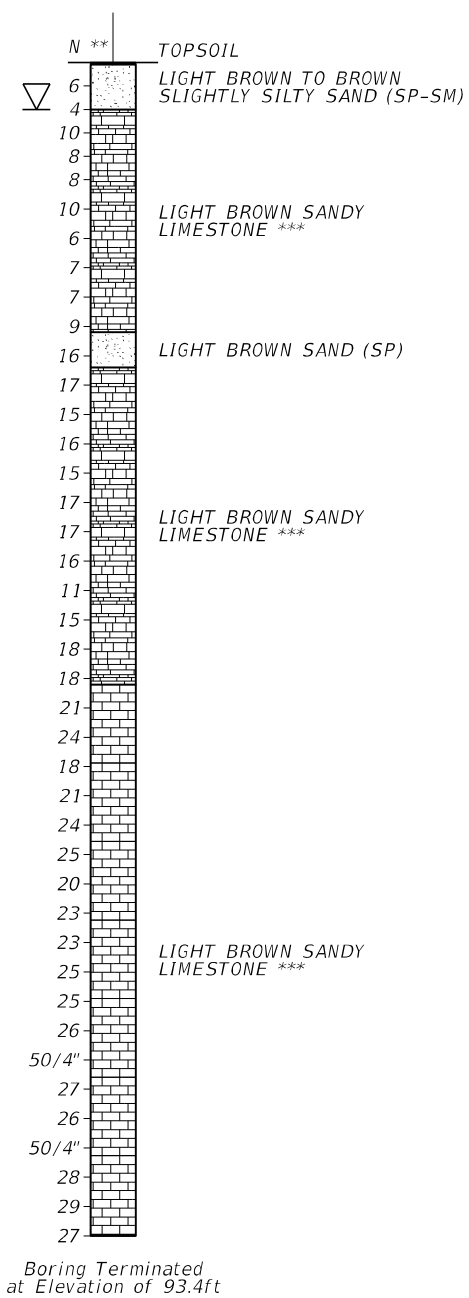
BORINGS LOCATION PLAN

Approximate Location of SPT Borings

BOR # BB-1
STA. 492+82.82 NE10AVE
OFF. 5.0' RT
*ELEV. 7.2
DATE 5/22/2020
HAMMER Auto
RIG CME-45
LATITUDE 25.852415
LONGITUDE -80.176586



BOR # BB-2
STA. 493+43.51 NE10AVE
OFF. 11.0' RT
*ELEV. 6.6
DATE 5/19/2020
HAMMER Auto
RIG CME-45
LATITUDE 25.852583
LONGITUDE -80.176574



LEGEND

Asphalt	Limestone Soft
Sandy Gravel	Limestone Hard
Sand	Sandy Silt

NOTES:

ENCOUNTERED WATER TABLE DURING DRILLING

GNE GROUNDWATER NOT ENCOUNTERED IN UPPER 10 FEET

APPROXIMATE LOCATION OF SPT BORING

* ELEVATION BASED ON TIN FILE

** NUMBERS TO THE LEFT OF BORINGS INDICATE SPT VALUE FOR 12" PENETRATION. (UNLESS OTHERWISE NOTED.) GENERALLY, DESIGN CORRELATION AND PROGRAMS USE SAFETY HAMMER N-VALUES. HENCE THE ABOVE N-VALUES NEED TO BE MULTIPLIED BY 1.23 TO OBTAIN EQUIVALENT SAFETY HAMMER N-VALUES FOR DESIGN PURPOSE.

*** THE LIMESTONE STRATA ENCOUNTERED WITHIN THE PROJECT SITE CORRESPOND TO ROCK FORMATION THAT TYPICALLY OFFER HIGH RESISTANCE TO EXCAVATION AND DRILLING. SPECIAL EQUIPMENT AND BREAKING TOOLS ARE TYPICALLY REQUIRED TO EXCAVATE AND DRILL WITHIN THESE LIMESTONE LAYERS. THESE LIMESTONE LAYERS ARE ALSO DIFFICULT TO DEWATER DUE TO ITS HIGH POROSITY AND PERMEABILITY.

STRATA BOUNDARIES ARE APPROXIMATE AND MAY VARY BETWEEN OR AWAY FROM BORING LOCATIONS.

STANDARD PENETRATION TEST DATA

SPOON INSIDE DIA.	1.375 inches
SPOON OUTSIDE DIA.	2.0 inches
AVG. HAMMER DROP	30.0 inches
HAMMER WEIGHT	140.0 pounds

SPT CONSISTENCY CHART

CONSISTENCY	SILTS AND CLAYS	
	SAFETY HAMMER SPT N-VALUE (BLOW/FOOT)	AUTOMATIC HAMMER SPT N-VALUE (BLOW/FOOT)
VERY SOFT	LESS THAN 2	LESS THAN 1
SOFT	2 - 4	1 - 3
FIRM	4 - 8	3 - 6
STIFF	8 - 15	6 - 12
VERY STIFF	15 - 30	12 - 24
HARD	GREATER THAN 30	GREATER THAN 24

SPT DENSITY CHART

GRANULAR MATERIALS	SPT DENSITY CHART	
	SAFETY HAMMER SPT N-VALUE (BLOW/FOOT)	AUTOMATIC HAMMER SPT N-VALUE (BLOW/FOOT)
VERY LOOSE	LESS THAN 4	LESS THAN 3
LOOSE	4 - 10	3 - 8
MEDIUM	10 - 30	8 - 24
DENSE	30 - 50	24 - 40
VERY DENSE	GREATER THAN 50	GREATER THAN 40

ENVIRONMENTAL CLASSIFICATION

(BASED ON WATER SAMPLE)

SUPERSTRUCTURE: EXTREMELY AGGRESSIVE

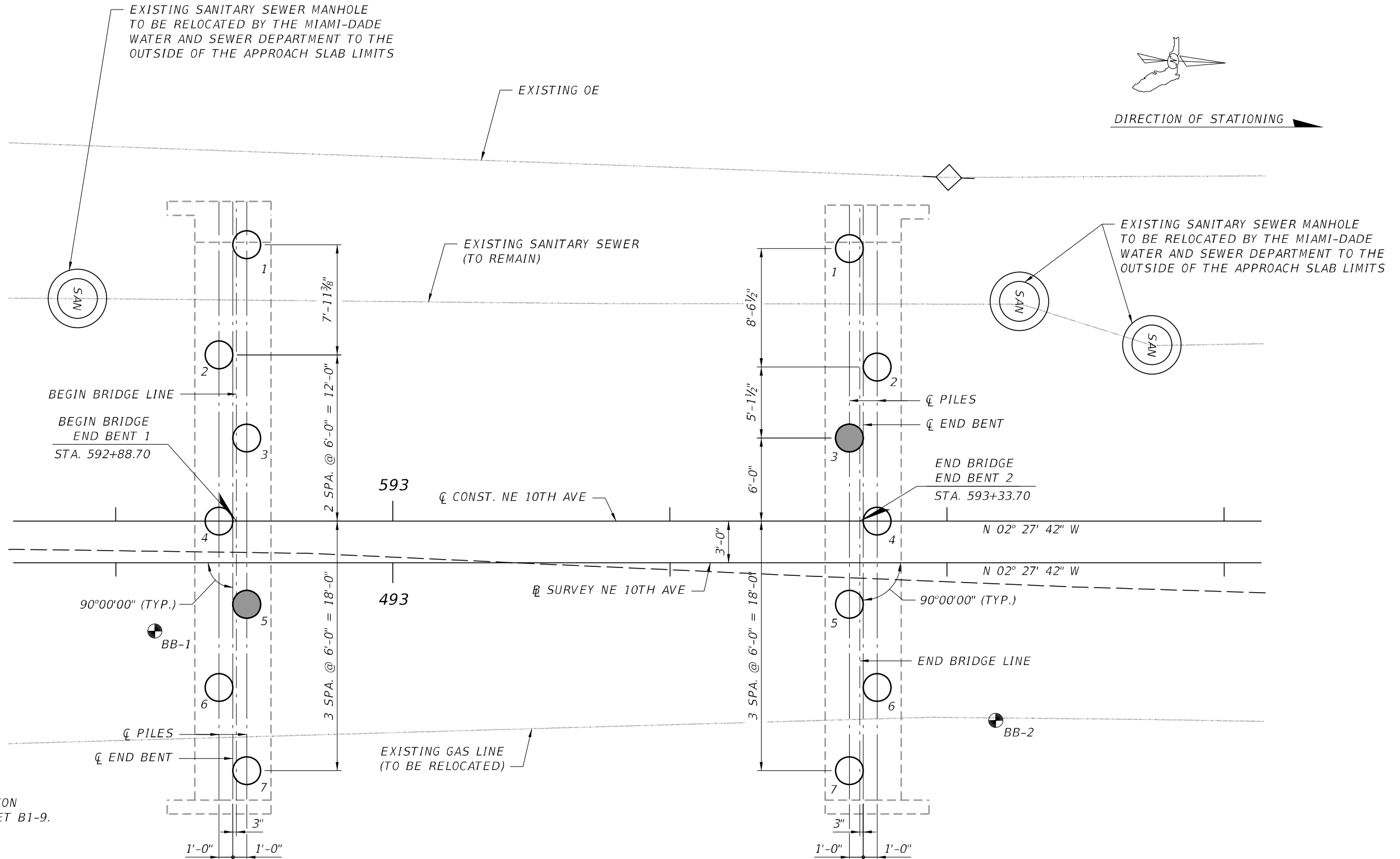
SUBSTRUCTURE:

STEEL: EXTREMELY AGGRESSIVE

CONCRETE: EXTREMELY AGGRESSIVE

(RESISTIVITY=80 ohm-CM)

REVISIONS				RAMAKUMAR VEDULA, P.E. P.E. LICENSE NUMBER 54873 TIERRA SOUTH FLORIDA, INC. 2765 VISTA PARKWAY, S-10 WEST PALM BEACH, FL 33411	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			REPORT OF CORE BORINGS NE 10TH AVENUE BRIDGE	SHEET NO.
DATE	DESCRIPTION	DATE	DESCRIPTION		ROAD NO.	COUNTY	PROJECT NUMBER		
					NE 10 AVE	MIAMI-DADE	EDP-MT-20190196		B1-9



- LEGEND**
- APPROXIMATE BORING LOCATION FOR BORING DATA, SEE SHEET B1-9.
 - 24" \odot ACIP PILE, TYP.
 - 24" \odot ACIP DEMONSTRATION PILE

- NOTES:**
- PILE SPACING IS DIMENSIONED ALONG ϕ PILE.
 - FOR PILE DATA TABLE, TABLE OF PILE CUT-OFF ELEVATIONS, AND NOTES, SEE SHEET B1-11.
 - THE EXISTING PILES TO BE CUT TWO FEET BELOW THE MUDLINE/GROUND LINE.

BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: FOUNDATION LAYOUT		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION			ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
								NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET		B1-10

ACIP PILE DATA TABLE														
INSTALLATION CRITERIA						DESIGN CRITERIA						TESTING	TOP OF ACIP PILE ELEVATION (Ft.)	EXITING GRADE
END BENT OR PIER NO.	SIZE (In.)	TIP ELEV. ⁵ (Ft.)	MIN. TIP ELEV. ⁶ (Ft.)	MIN. PILE EMBEDMENT LENGTH ⁷ (Ft.)	MIN. TOP OF BEARING LAYER ELEV. ⁷ (Ft.)	FACTORED DESIGN LOAD (tons)	FACTORED DESIGN UPLIFT LOAD (tons)	DOWN DRAG (tons)	100-YEAR SCOUR ELEV. (Ft.)	Ø COMPRESSION	Ø UPLIFT	CONSIDER REDUNDANT		
END BENT 1	24	-41	-41	44	-20	100	N/A	N/A	N/A	0.5	0.4	YES	SEE TABLE	+7.47' ±
END BENT 2	24	-46	-46	49	-20	100	N/A	N/A	N/A	0.5	0.4	YES	SEE TABLE	+7.35' ±

ACIP = AUGER CAST-IN-PLACE PILE

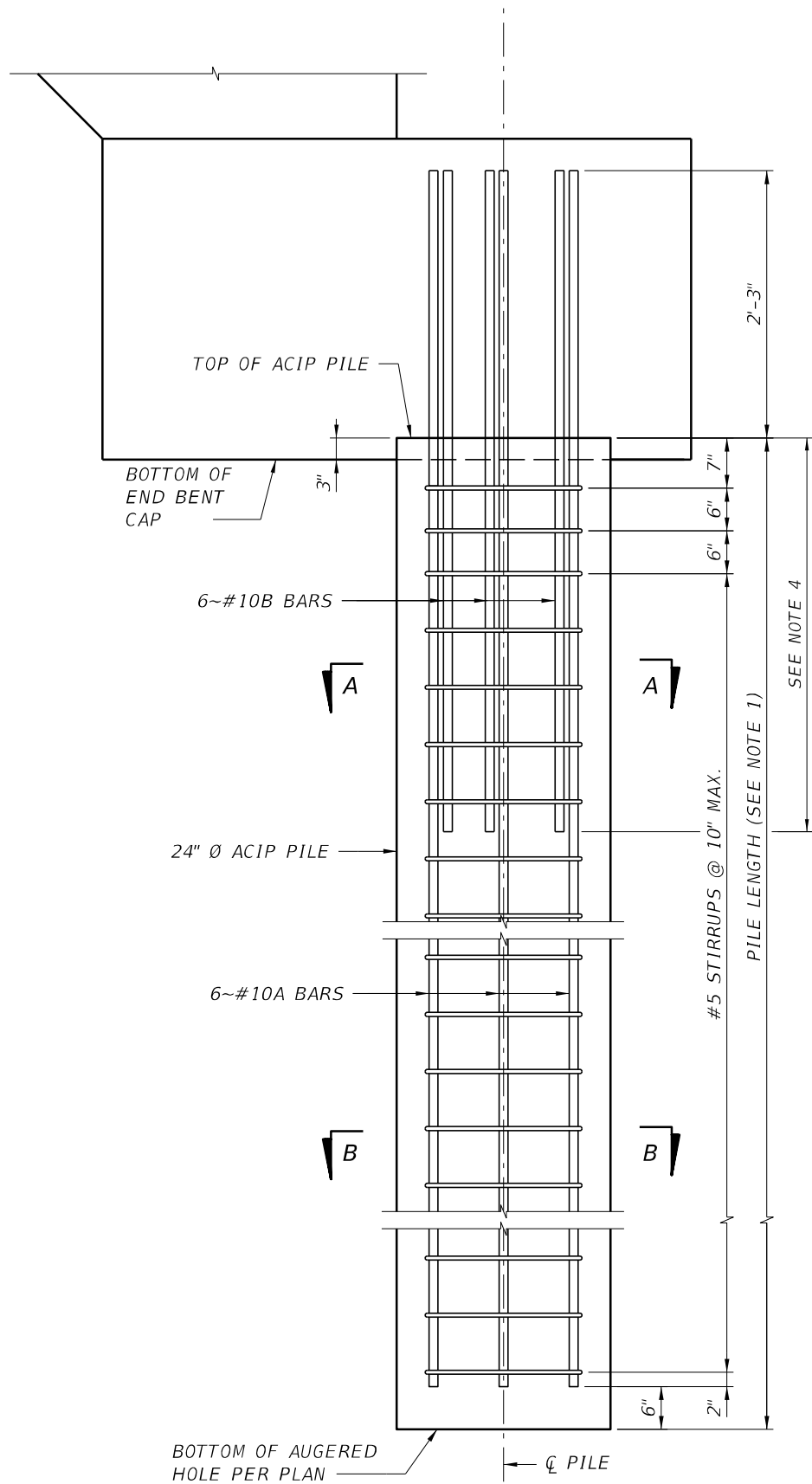
TOP OF ACIP PILE ELEVATION							
LOCATION	PILE 1	PILE 2	PILE 3	PILE 4	PILE 5	PILE 6	PILE 7
END BENT 1	+2.443	+2.602	+2.722	+2.842	+2.722	+2.602	+2.482
END BENT 2	+2.500	+2.671	+2.773	+2.893	+2.773	+2.653	+2.533

NOTES:

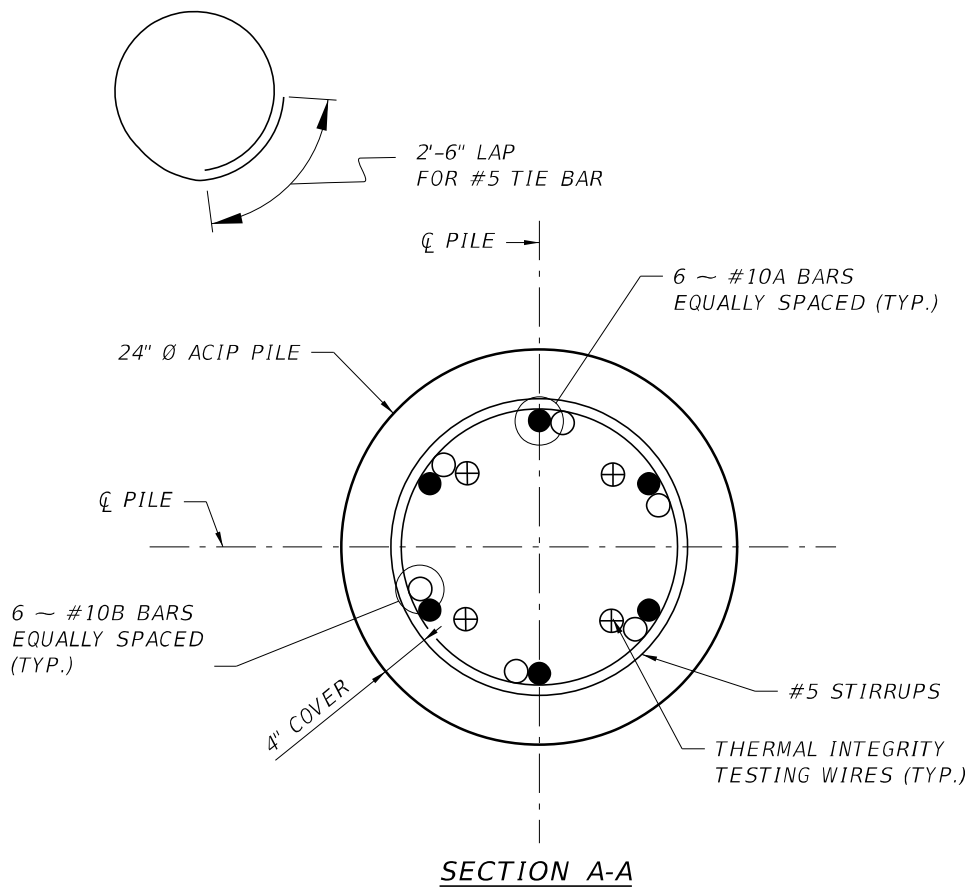
1. NON-PRODUCTION DEMONSTRATION PILE(S) SHALL BE INSTALLED AT THE LOCATION(S) INDICATED IN THE PLANS. THE DEMONSTRATION PILE(S) SHALL BE CONSTRUCTED WITH THE SAME DIAMETER, DEEPEST TIP ELEVATION, REINFORCEMENT, AND THERMAL INTEGRITY TESTING WIRES AS THE PRODUCTION PILES. A PILOT HOLE(S) SHALL BE PERFORMED AT THE LOCATION OF THE DEMONSTRATION PILE(S).
2. ALL ACIP PILES SHALL BE INSTALLED PLUMB IN ACCORDANCE WITH SECTION 455-44 OF THE MODIFIED SPECIAL PROVISIONS (MSP) FOR ACIP PILES.
3. THE CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY LOCATION OF ALL EXISTING ABOVE AND BELOW GROUND UTILITIES PRIOR TO INSTALLATION OF PILES. FOR DISPOSITION OF EXISTING UTILITIES. SEE ROADWAY PLANS.
4. FOLLOWING THE TEST PILE PROGRAM, A LETTER WILL BE ISSUED BY THE ENGINEER FOR PRODUCTION PILE LENGTHS AND TIP ELEVATIONS. PRODUCTION PILE INSTALLATION SHALL NOT COMMENCE UNTIL THE PRODUCTION PILE LENGTH AND TIP ELEVATION HAS BEEN ISSUED BY THE ENGINEER AND APPROVED BY THE DEPARTMENT.
5. ALL PILES SHALL BE INSTALLED TO THE TIP ELEVATION AS SHOWN ON THE PROJECT PLANS UNLESS LOAD TEST DATA, ROCK CORE TESTS, OR OTHER GEOTECHNICAL TEST DATA OBTAINED DURING PILOT HOLES ALLOWS THE ENGINEER TO AUTHORIZE A DIFFERENT TIP ELEVATION. ACTUAL TIP ELEVATIONS ARE TO BE RECORDED IN AS-BUILT PLANS.
6. THE MINIMUM TIP ELEVATION PROVIDED IN THE PLANS SATISFIES AXIAL (COMPRESSION AND UPLIFT), LATERAL, AND SETTLEMENT REQUIREMENTS.
7. THE MINIMUM PILE EMBEDMENT LENGTH IS REQUIRED BELOW THE MINIMUM TOP OF BEARING LAYER ELEVATION TO ACHIEVE THE REQUIRED NOMINAL BEARING RESISTANCE.
8. TEST ALL DEMONSTRATION PILES, TEST PILES, AND PRODUCTION PILES BY MEANS OF THERMAL INTEGRITY TESTING IN ACCORDANCE WITH THE MSP FOR ACIP PILES.
9. THE PROVISIONS IN THE SETTLEMENT AND VIBRATION MONITORING PLAN (SVMP), NOISE CONTROL MONITORING PLAN (NCMP), AND SECTION 108 OF THE FDOT STANDARD SPECIFICATIONS SHALL BE FOLLOWED FOR THE PROTECTION OF THE EXISTING STRUCTURES AND/OR NEARBY UTILITIES DURING PILE INSTALLATION OPERATIONS. ALL STRUCTURES AND/OR UTILITIES LOCATED ADJACENT TO THE PROPOSED PILE INSTALLATION OPERATIONS SHALL BE SURVEYED AS WELL AS MONITORED FOR VIBRATIONS AND SETTLEMENTS IN ACCORDANCE WITH THE APPROVED SVMP, NCMP, AND SECTION 108 OF THE FDOT STANDARD SPECIFICATIONS.
10. COMPRESSION LOAD TEST(S) SHALL BE PERFORMED AT THE LOCATIONS INDICATED IN THE PLANS IN ACCORDANCE WITH SECTION 455 OF THE MSP FOR ACIP PILES. A PILOT HOLE SHALL BE PERFORMED AT THE LOCATION OF EACH COMPRESSION LOAD TEST. COMPRESSION TEST LOAD SHALL BE AT LEAST 200 TONS FOR THE LOCATION SHOWN IN THE PLANS. THE TEST PILE SHALL BE INSTALLED TO THE TIP ELEVATION SHOWN IN THE ACIP PILE DATA TABLE, AND SHALL BE REINFORCED AS SHOWN IN THE PLANS.
11. A MINIMUM OF FIVE (5) PERCENT OR AT LEAST ONE (1) PER FOUNDATION UNIT OF PRODUCTION PILES SHALL BE PROOF TESTED TO THE FACTORED DESIGN RESISTANCE TO VERIFY THE LOAD-CARRYING CAPACITY OF THE ACIP PILES. THE FACTORED DESIGN RESISTANCE IS THE SUM OF THE FACTORED DESIGN LOAD, DOWNDRAG LOAD, AND NET SCOUR LOAD. THE PILES TO BE TESTED SHALL BE SELECTED BY THE ENGINEER, WITH CONCURRENCE BY THE ENGINEER, AND TESTING SHALL BE PERFORMED IN ACCORDANCE WITH THE MSP FOR ACIP PILES. PROOF TESTING CAN BE PERFORMED BY MEANS OF RAPID LOAD TEST (RLT), STATIC LOAD TEST (SLT) OR DYNAMIC LOAD TEST (DLT). THE ENGINEER SHALL REVIEW ALL PROOF LOAD TEST DATA FOR ACCEPTANCE. IF RLT OR DLT IS SELECTED, PROOF TESTING IS REQUIRED FOR ALL LOAD TEST PILES PRIOR TO STATIC LOAD TESTING.
12. THE BORINGS ENCOUNTERED VERY DENSE SANDS AND/OR A VERY HARD LIMESTONE FORMATION. DEWATERING, PENETRATING AND/OR EXCAVATING THROUGH THE VERY DENSE SANDS AND LIMESTONE FORMATION MAY REQUIRE SPECIAL EQUIPMENT DUE TO THE STRENGTH OF THE MATERIALS.
13. THE BORINGS AND ROCK CORING LOCATIONS REVEALED THE PRESENCE OF VERY LOOSE SOILS AND WEAK LIMESTONE. AS SUCH, THE ACIP PILE CONTRACTOR SHOULD BE FAMILIAR WITH MEANS AND METHODS TO INSTALL ACIP PILES IN VERY LOOSE SOILS. ALSO, DUE TO THE PRESENCE OF LOOSE SOIL ZONES AND POROUS LIMESTONE FORMATION, IT IS POSSIBLE THAT LOSS OF GROUT FLOW INTO THE SURROUNDING ROCK FORMATION WILL BE EXPERIENCED DURING EXCAVATION, AND IS TYPICAL IN THIS AREA. THE CONTRACTOR SHALL TAKE THIS INTO ACCOUNT IN HIS BID.
14. THE MINIMUM 28-DAY GROUT STRENGTH FOR ALL PILES SHALL BE 5,500 POUNDS PER SQUARE INCH (PSI).
15. AUTOMATED MONITORING EQUIPMENT (AME) DATA IS TO BE SUBMITTED TO FDOT IN ELECTRONIC FORM, TRANSFERABLE TO EXCEL, WITHIN 24 HOURS OF PILE INSTALLATION.
16. ELEVATIONS ARE WITH RESPECT TO NGVD29 DATUM.
17. ACIP PILES TO BE INSTALLED TO EXISTING GRADE ELEVATION AND CUT TO FINAL ELEVATIONS IN ACCORDANCE WITH ACIP INSTALLATION PLAN.
18. SIX (6) PAIR OF STRAIN GAUGES TO BE INSTALLED AT VARYING DEPTHS WITHIN THE COMPRESSION TEST PILES/DEMONSTARTION PILES.

BRIDGE NO. 874185

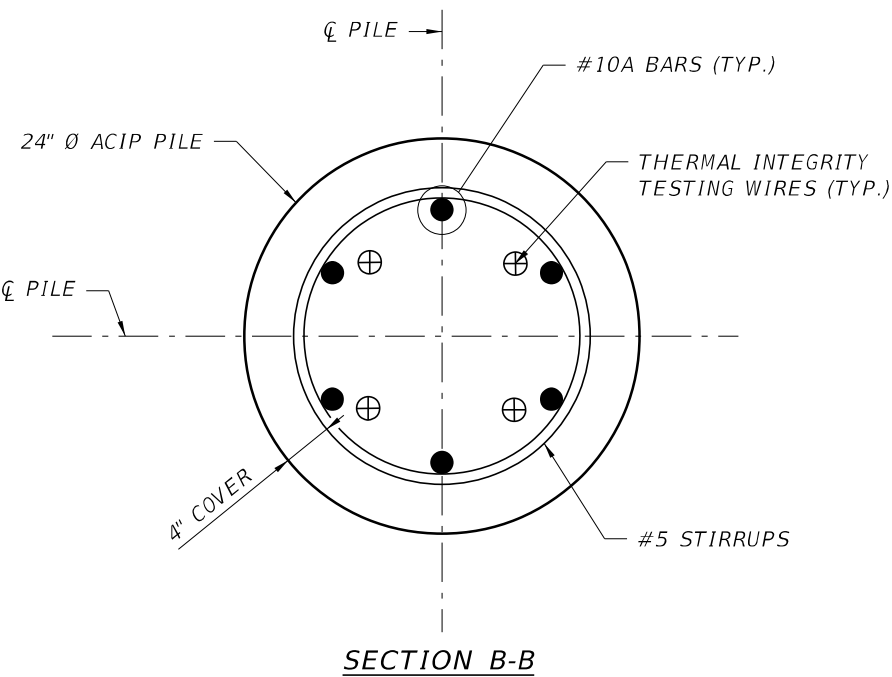
REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION			ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PILE DATA TABLE		
											PROJECT NAME:		SHEET NO.
								NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET		B1-11



ELEVATION



SECTION A-A



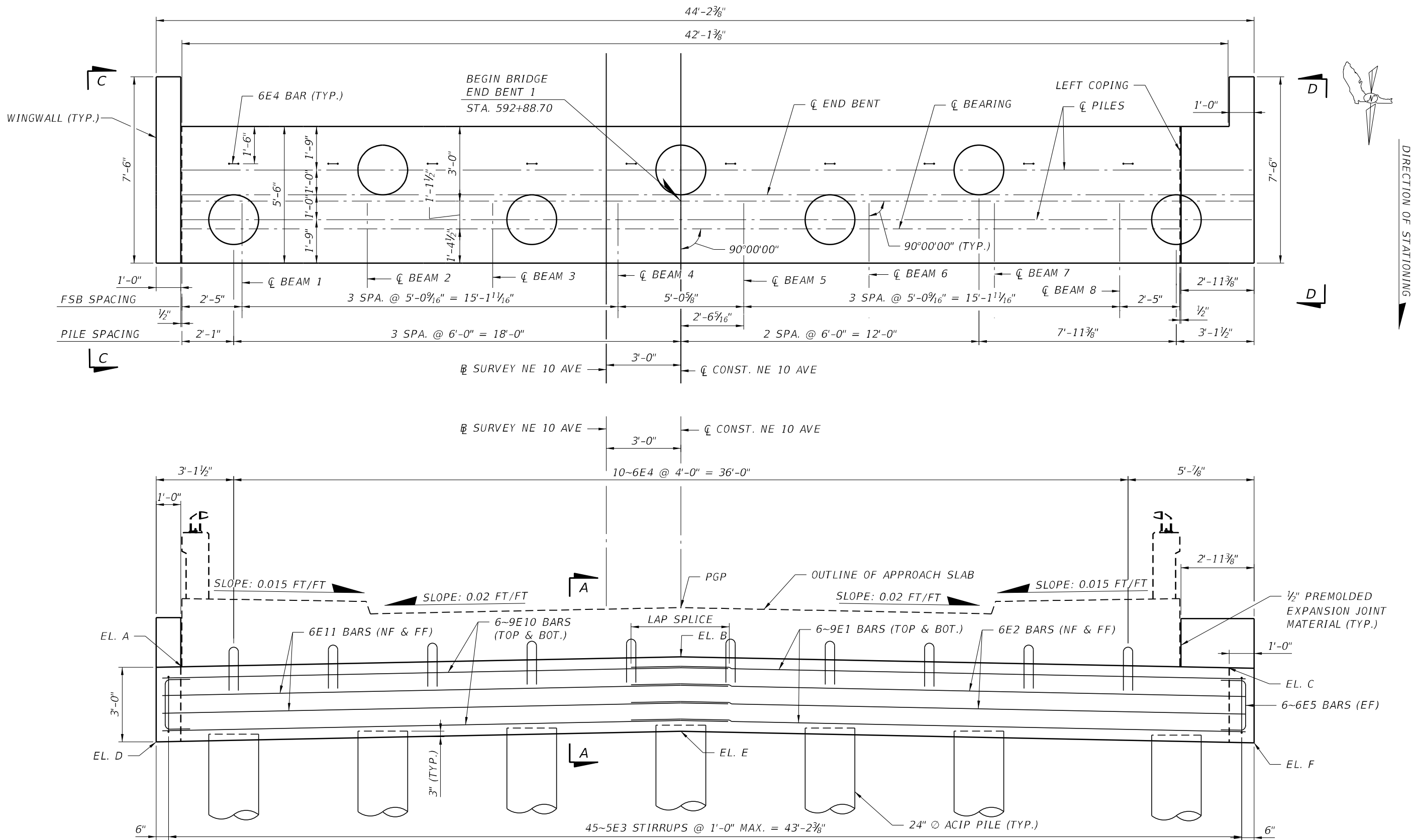
SECTION B-B

NOTES:

1. FOR PILE LENGTH TABLES, SEE PILE DATA TABLES.
2. FOR ELEVATIONS AT THE TOP AND TIP OF THE ACIP PILES, SEE PILE DATA TABLE.
3. USE GRADE 75, 2205 STAINLESS REINFORCING STEEL FOR AUGER CAST PILE REBARS.
4. CONTRACTOR HAS OPTION TO USE MECHANICAL SPLICES FROM FDOT APL FOR AUGER CAST PILE REBARS. LAB SPLICES OF VERTICAL REINFORCEMENT ARE NOT ALLOWED. SPLICING OF VERTICAL REINFORCEMENT SHALL BE ACCOMPLISHED USING MECHANICAL COUPLERS. MECHANICAL COUPLERS SHALL BE FROM FDOT APL LIST.

BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20				ACIP PILE DETAILS		
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	SHEET NO.	
							CHECKED BY: RA 07-20	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET	B1-12	

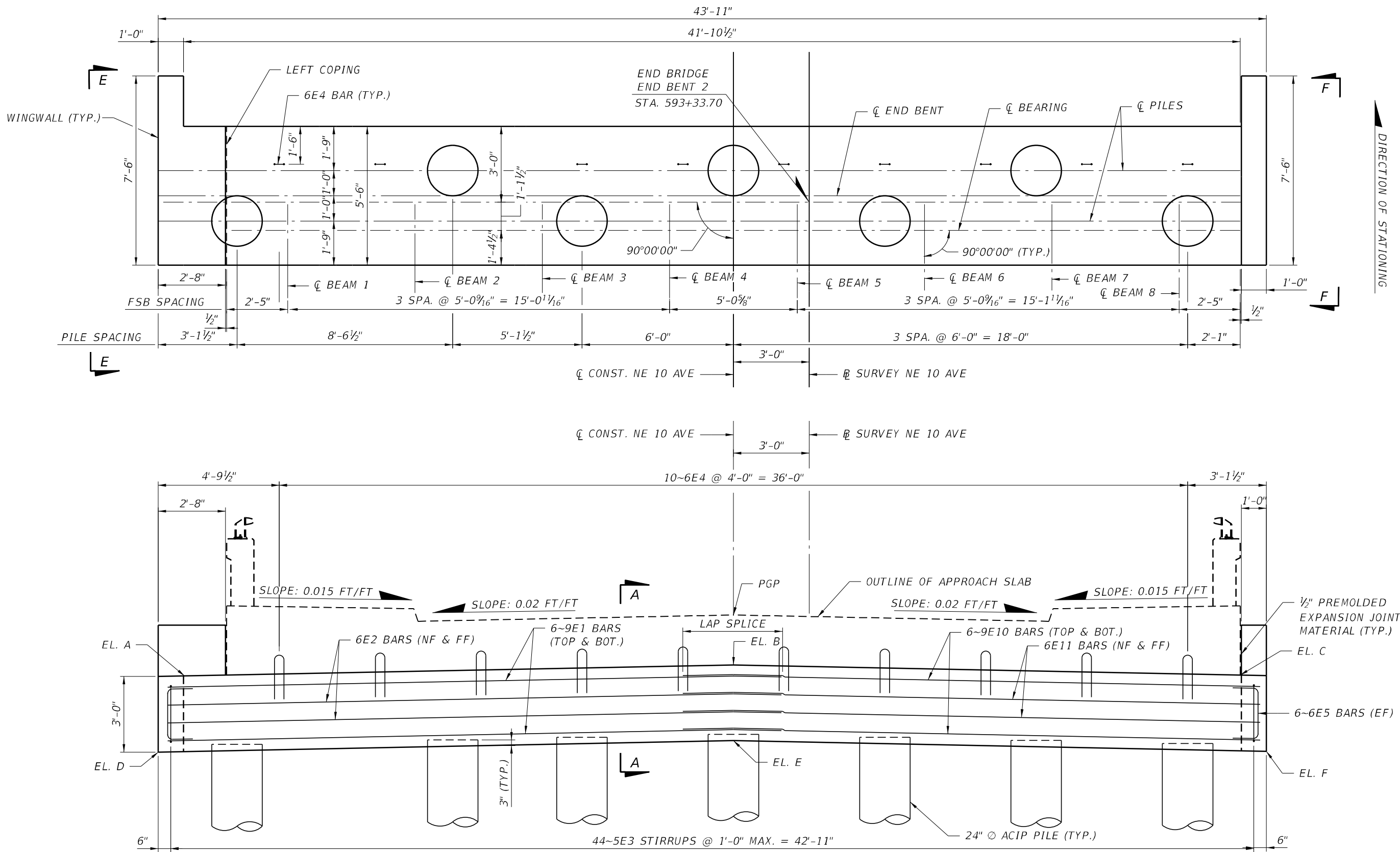


- NOTES:**
1. FOR SECTION A-A, VIEWS C-C & D-D, AND TABLE OF ELEVATIONS, SEE SHEET B1-15.
 2. FOR PILE LOADS AND PILE CUT OFF ELEVATIONS, SEE SHEET B1-11.
 3. FOR REINFORCING BAR LIST, SEE SHEET B1-22.
 4. MINIMUM LAP:
#6 = 2'-7"
#9 = 4'-0"

ELEVATION

BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130			DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20			MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: END BENT 1		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION												
												ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
												NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	FROM NE 83RD STREET TO NE 84TH STREET		B1-13



- NOTES:**
1. FOR SECTION A-A, VIEWS E-E & F-F, AND TABLE OF ELEVATIONS, SEE SHEET B1-15.
 2. FOR PILE LOADS AND PILE CUT OFF ELEVATIONS, SEE SHEET B1-11.
 3. FOR REINFORCING BAR LIST, SEE SHEET B1-22.
 4. MINIMUM LAP: #6 = 2'-7" #9 = 4'-0"

ELEVATION

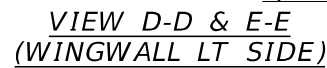
BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130			DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20			MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: END BENT 2		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION												
												ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
												NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	FROM NE 83RD STREET TO NE 84TH STREET		B1-14



TABLE OF END BENT ELEVATIONS		
LOCATION	END BENT 1	END BENT 2
EL. A	5.190	5.208
EL. B	5.592	5.643
EL. C	5.151	5.241
EL. D	2.170	2.188
EL. E	2.592	2.643
EL. F	2.131	2.221

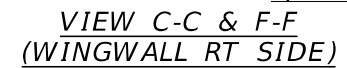
FOR LOCATION OF SECTION A-A, VIEWS C-C
AND D-D, AND ELEVATION POINTS, SEE
SHEETS 'B1-13' & 'B1-14'.



(VIEW E-E SHOWN, VIEW D-D SIMILAR OPPOSITE HAND)



(END BENT 2 SHOWN. END BENT 1 SIMILAR)
(BARS 5E3 SHOWN IN GRAY FOR CLARITY)



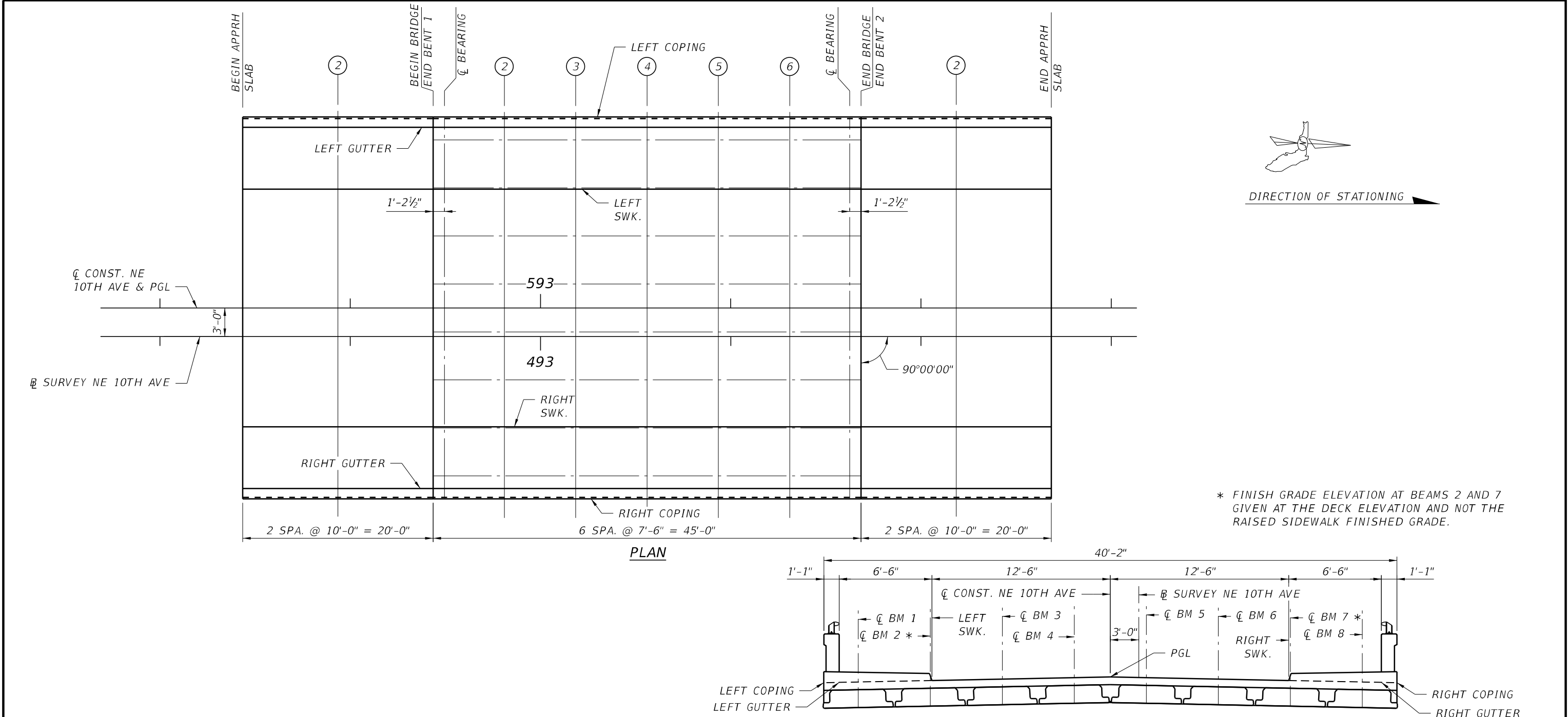
(VIEW C-C SHOWN, VIEW F-F SIMILAR OPPOSITE HAND)



(END BENT 2 SHOWN. END BENT 1 SIMILAR)
(BARS 5E3 SHOWN IN GRAY FOR CLARITY)

BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20				END BENT DETAILS		
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME: NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET	SHEET NO.	
							CHECKED BY: RA 07-20	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196		B1-15	



FINISH GRADE ELEVATIONS									
LOCATION	SPAN 1								
	BEGIN BRIDGE END BENT 1	CL BEARING	2	3	MID-SPAN 4	5	6	CL BEARING	END BRIDGE END BENT 2
LEFT COPING	7.013	7.018	7.042	7.063	7.077	7.082	7.079	7.071	7.069
LEFT GUTTER	7.034	7.040	7.064	7.085	7.098	7.104	7.101	7.093	7.091
BEAM 1	7.066	7.071	7.095	7.117	7.130	7.135	7.133	7.124	7.122
BEAM 2	7.167	7.172	7.196	7.217	7.231	7.236	7.234	7.225	7.223
BEAM 3	7.268	7.273	7.297	7.318	7.332	7.337	7.335	7.326	7.324
BEAM 4	7.369	7.374	7.398	7.419	7.433	7.438	7.436	7.427	7.425
PGL	7.419	7.425	7.449	7.470	7.483	7.489	7.486	7.478	7.476
BEAM 5	7.369	7.374	7.398	7.419	7.433	7.438	7.436	7.427	7.425
BEAM 6	7.268	7.273	7.297	7.318	7.332	7.337	7.335	7.326	7.324
BEAM 7	7.167	7.172	7.196	7.217	7.231	7.236	7.234	7.225	7.223
BEAM 8	7.066	7.071	7.095	7.117	7.130	7.135	7.133	7.124	7.122
RIGHT GUTTER	7.034	7.040	7.064	7.085	7.098	7.104	7.101	7.093	7.091
RIGHT COPING	7.013	7.018	7.042	7.063	7.077	7.082	7.079	7.071	7.069

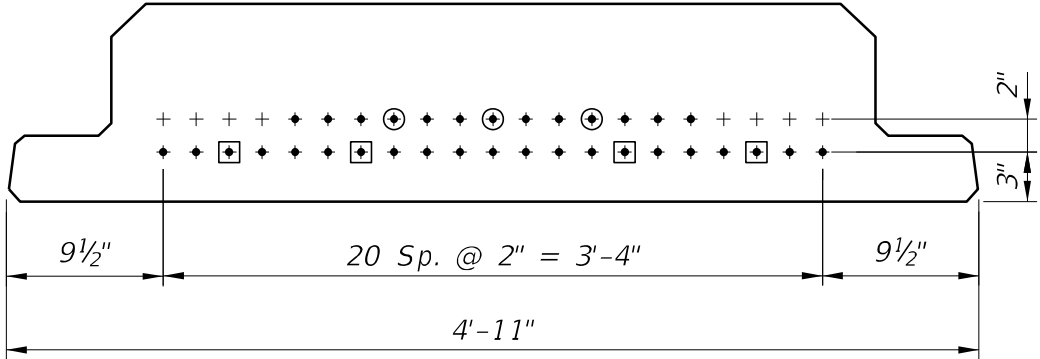
FINISH GRADE ELEVATIONS						
LOCATION	APPROACH SLAB 1			APPROACH SLAB 2		
	BEGIN APPR. SLAB 1	2	BEGIN BRIDG END BENT 1	END BRIDGE END BENT 4	2	END APPR. SLAB 1
LEFT COPING	6.896	6.961	7.013	7.069	7.042	7.002
LEFT GUTTER	6.917	6.983	7.034	7.091	7.064	7.023
PGL	7.302	7.368	7.419	7.476	7.449	7.408
RIGHT GUTTER	6.917	6.983	7.034	7.091	7.064	7.023
RIGHT COPING	6.896	6.961	7.013	7.069	7.042	7.002

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: FINISH GRADE ELEVATIONS			REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20							
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME: NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET			SHEET NO.
							CHECKED BY: RA 07-20	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196				

THE OFFICIAL RECORD OF THIS SHEET IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, F.A.C.

FLORIDA SLAB BEAM - TABLE OF VARIABLES																												Table Date 01-01-16			
LOCATION		BEAM TYPE	CONCRETE PROPERTIES			STND. PTRN.	PLAN VIEW CASE		ANGLE Ø		BEAM DIMENSIONS*			REINFORCING STEEL																	
SPAN NO.	BEAM NO.		CLASS	STRENGTHS (psi)			END 1	END 2	END 1	END 2	DIM W	DIM L	DIM R	3C		4D1	4D2	4D3		5E1		5E2		6Y1	6Y2	4K	NO. OF BAR SPACES		BAR SPACING*		
				28 Day	Release	NO.								DIM C	DIM D	DIM D	NO.	DIM E	NO.	DIM E	DIM Y	DIM Y	NO.	S1	S2	V1	V2				
1	1-8	FSB 15X59	VI	8500	6000	1	1	1	90°	90°	4'-11"	43'-8½"	½"	45	4'-6½"	2'-5½"	2'-5½"	36	2'-5½"	87	4'-9"	87	4'-7½"	3'-6½"	3'-6½"	148	6	11	9"	1'-6"	

NOTE: WORK THIS SHEET WITH STANDARD PLAN INDEX
450-450 THRU 450-453.



TYPE ① 34 STRANDS

STRAND DESCRIPTION: USE 0.60" DIAMETER, GRADE 270, LOW RELAXATION CARBON STEEL STRANDS STRESSED AT 44.0 KIPS EACH. AREA PER STRAND EQUALS 0.217 SQ. IN.

STRAND PATTERNS

STRAND DEBONDING LEGEND

- FULLY BONDED STRANDS.
- ⊕ - STRANDS DEBONDED 4'-0" FROM END OF BEAM.
- ⊞ - STRANDS DEBONDED 6'-0" FROM END OF BEAM.

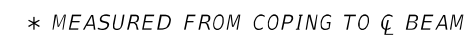
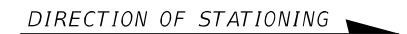
NOTE: ON BEAMS WITH SKEWED ENDS THE DEBONDED LENGTH SHALL BE MEASURED ALONG THE DEBONDED STRAND.

DIMENSION NOTES

* ALL LONGITUDINAL BEAM DIMENSIONS SHOWN ON THIS SHEET WITH A SINGLE ASTERISK (*) ARE MEASURED ALONG THE TOP OF BEAM AT THE CENTERLINE. DIMENSION "R" IS CALCULATED AT MID-HEIGHT OF THE BEAM.

BRIDGE NO. 874185

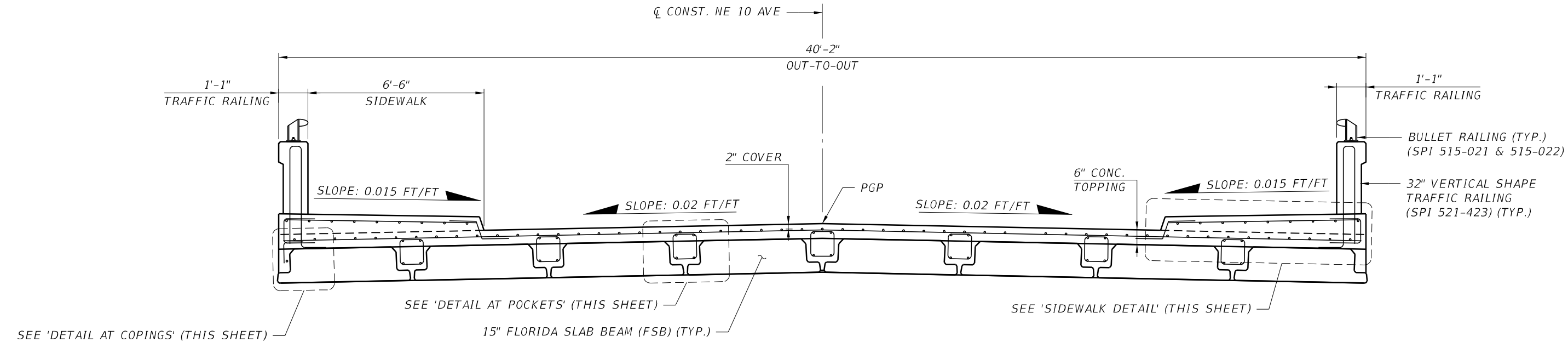
REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130		DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20			MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:				REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION													
														PROJECT NAME:				SHEET NO.
														NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET				B1 - 17



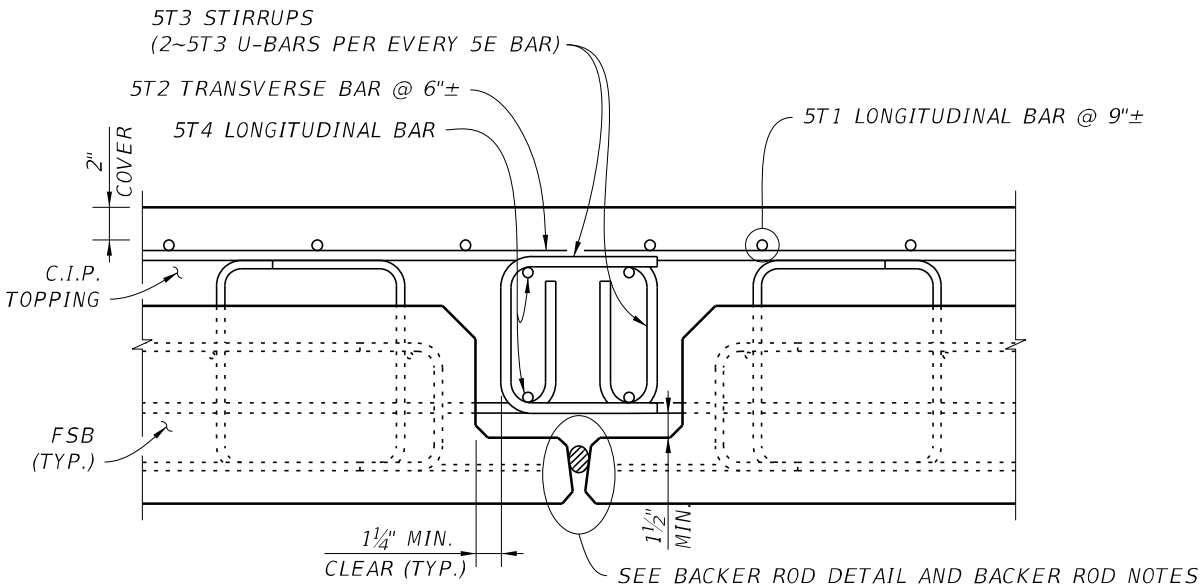
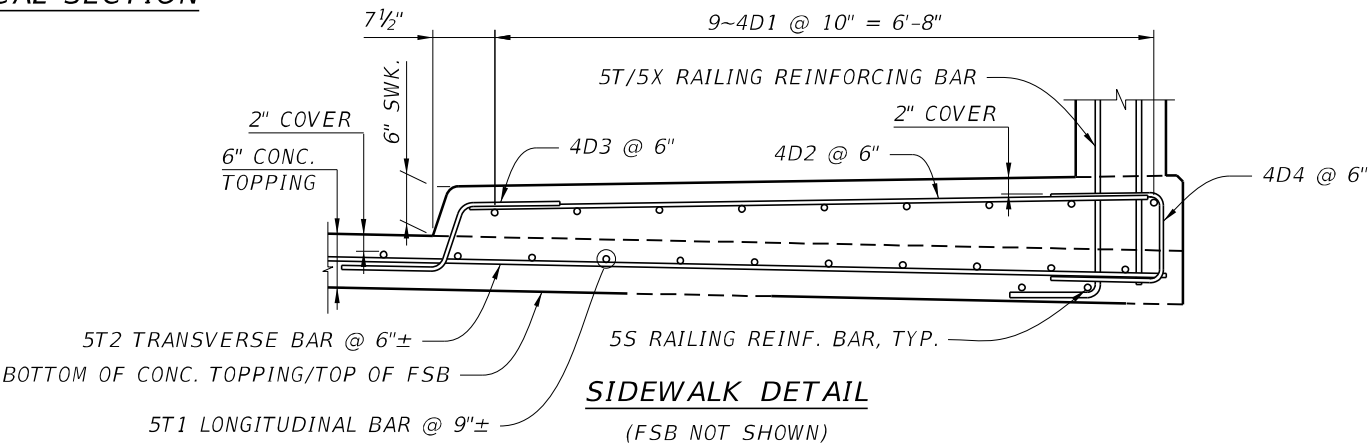
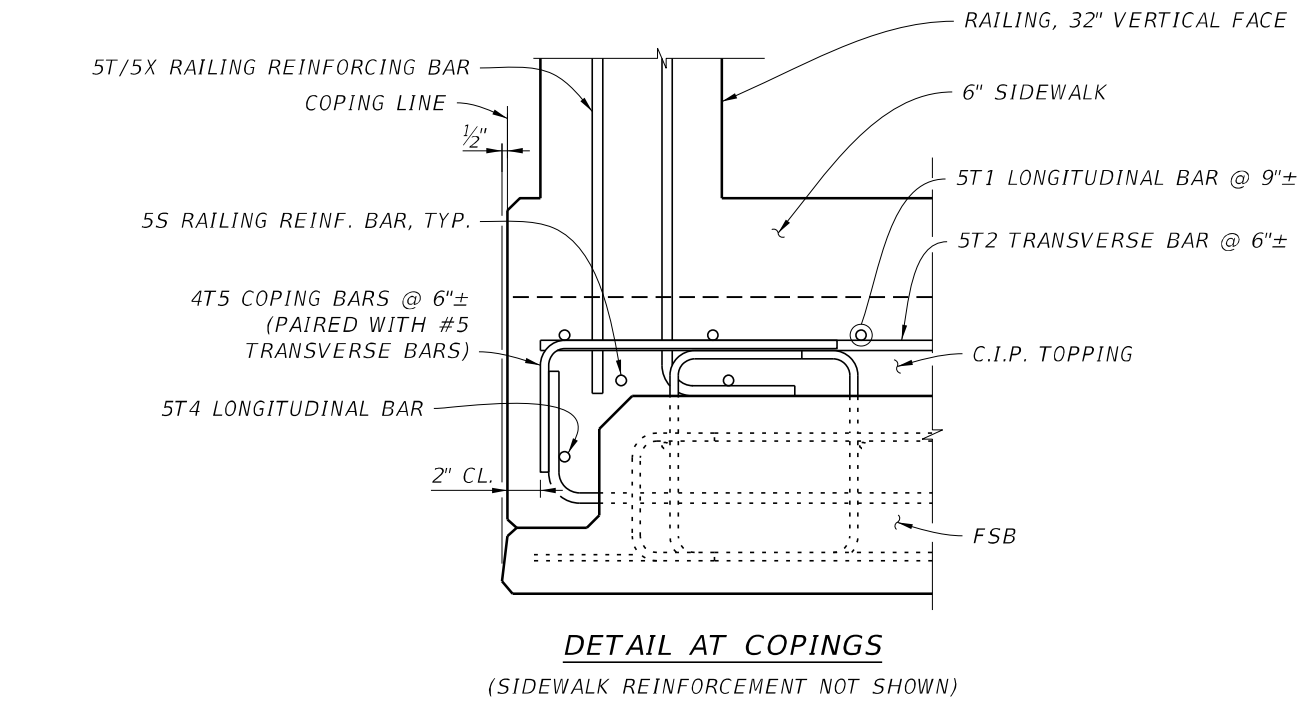
(BARS 5T3 AND 5T4 AT POCKETS BETWEEN BEAMS NOT SHOWN)

THE OFFICIAL RECORD OF THIS SHEET IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, F.A.C.

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: BA 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION						SUPERSTRUCTURE PLAN		
								ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
								NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET		B1-18



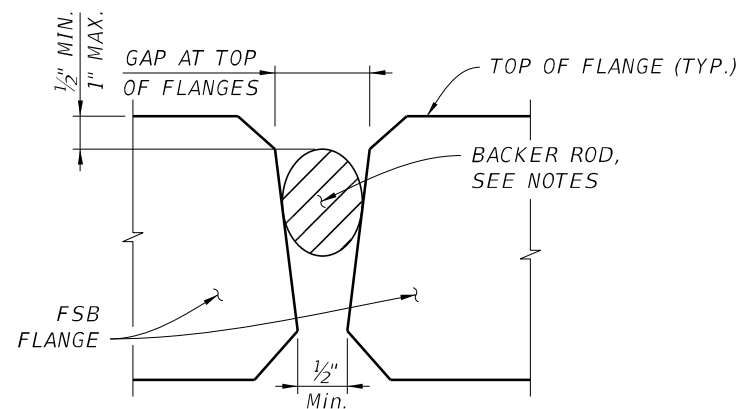
TYPICAL SECTION



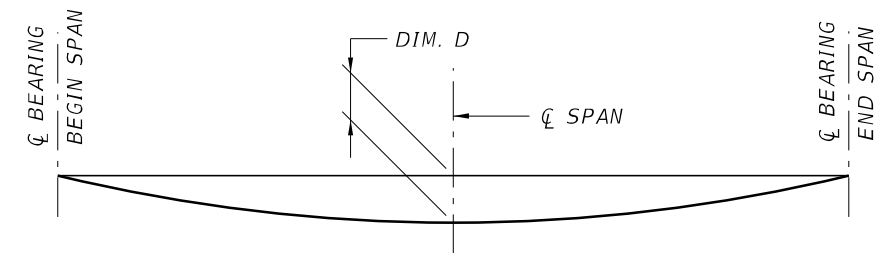
NOTE:
USE STAINLESS STEEL REBAR
FOR BARS 5T3, 3C, & 5E

BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: SUPERSTRUCTURE SECTION	REF. DWG. NO.			
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20					ROAD NO. NE 10 AVE	COUNTY MIAMI-DADE	FINANCIAL PROJECT ID EDP-MT-20190196	
							DESIGNED BY: SS 07-20								SHEET NO.
							CHECKED BY: RA 07-20								
							PROJECT NAME: NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET								



CAMBER NOTE:
THE VALUES GIVEN IN THE TABLE ARE BASED ON THEORETICAL BEAM CAMBERS. THE CONTRACTOR SHALL MONITOR BEAM CAMBERS FOR THE PURPOSE OF PREDICTING CAMBER VALUES AT THE TIME OF THE TOPPING CASTING. IF THE PREDICTED CAMBERS BASED ON FIELD MEASUREMENTS DIFFER MORE THAN $\pm 1/2$ " FROM THE THEORETICAL "NET BEAM CAMBER @ 120 DAYS" SHOWN IN THE TABLE, PROPOSE MODIFIED DIMENSIONS AS REQUIRED AND SUBMIT TO THE ENGINEER FOR APPROVAL A MINIMUM OF 21 DAYS PRIOR TO CASTING TOPPING CONCRETE.



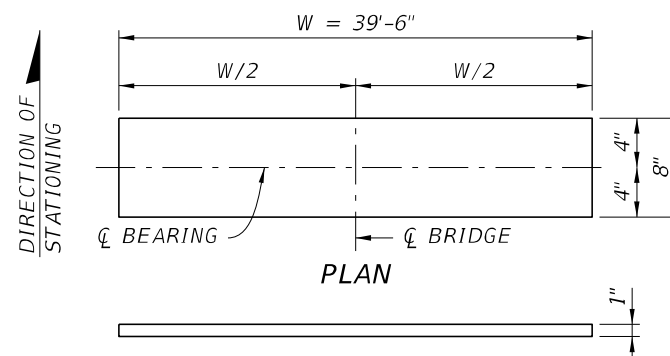
(REINFORCING WITHIN FSB NOT SHOWN FOR CLARITY)

* SEE POURED EXPANSION JOINT
DATA TABLE (THIS SHEET) &
INDEX NO. 458-110.

** MEASURED PERPENDICULAR TO BEAM END.

1. USE A BACKER ROD TO FORM THE BOTTOM OF THE CAST IN PLACE TOPPING AT THE GAP BETWEEN ADJACENT FSBS. USE A BACKER ROD MEETING THE REQUIREMENTS OF ASTM C1330 OR ASTM D5249, TYPES 1 OR 3, WITH A MINIMUM UNCOMPRESSED DIAMETER 50% LARGER THAN THE FIELD VERIFIED MAXIMUM WIDTH OF THE GAP BETWEEN ADJACENT FSBS. MEASURE GAP AT THE TOP OF THE FLANGES AS SHOWN IN THE BACKER ROD DETAIL.
2. INSTALL THE BACKER ROD FROM THE TOP DOWN TO THE POSITION SHOWN IN THE BACKER ROD DETAIL.
3. SECURE THE BACKER ROD TO PREVENT DISPLACEMENT DURING TOPPING CONCRETE PLACEMENT AND TO BE MORTAR TIGHT USING A COMPATIBLE CONSTRUCTION ADHESIVE.
4. THE BACKER ROD MAY REMAIN IN PLACE AFTER TOPPING CONCRETE PLACEMENT.

CAMBER AND DEFLECTION DATA TABLE FOR FLORIDA SLAB BEAMS			Table Date 1-01-16
LOCATION		NET BEAM CAMBER (PRESTRESS - DEAD LOAD OF BEAM) @ 120 DAYS (IN.)	DIM. D DEAD LOAD DEFLECTION DUE TO TOPPING CASTING @ 120 DAYS (IN.)
SPAN NO.	BEAM NO.		
1	1 & 8	2 $\frac{3}{8}$ "	1 $\frac{1}{2}$ "
1	2 THRU 7	2 $\frac{3}{8}$ "	9 $\frac{1}{16}$ "



CONTINUOUS BEARING PAD DETAIL

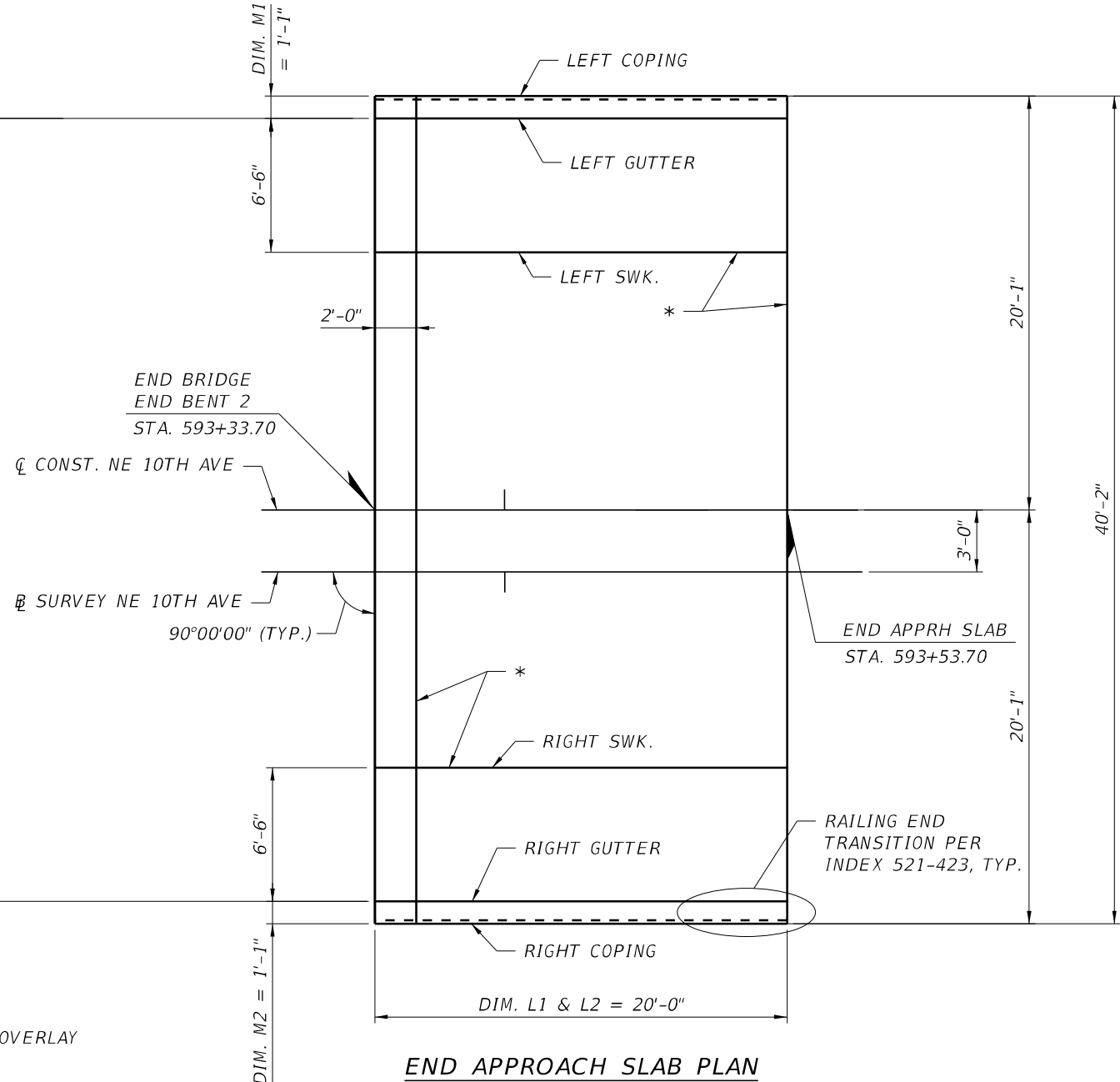
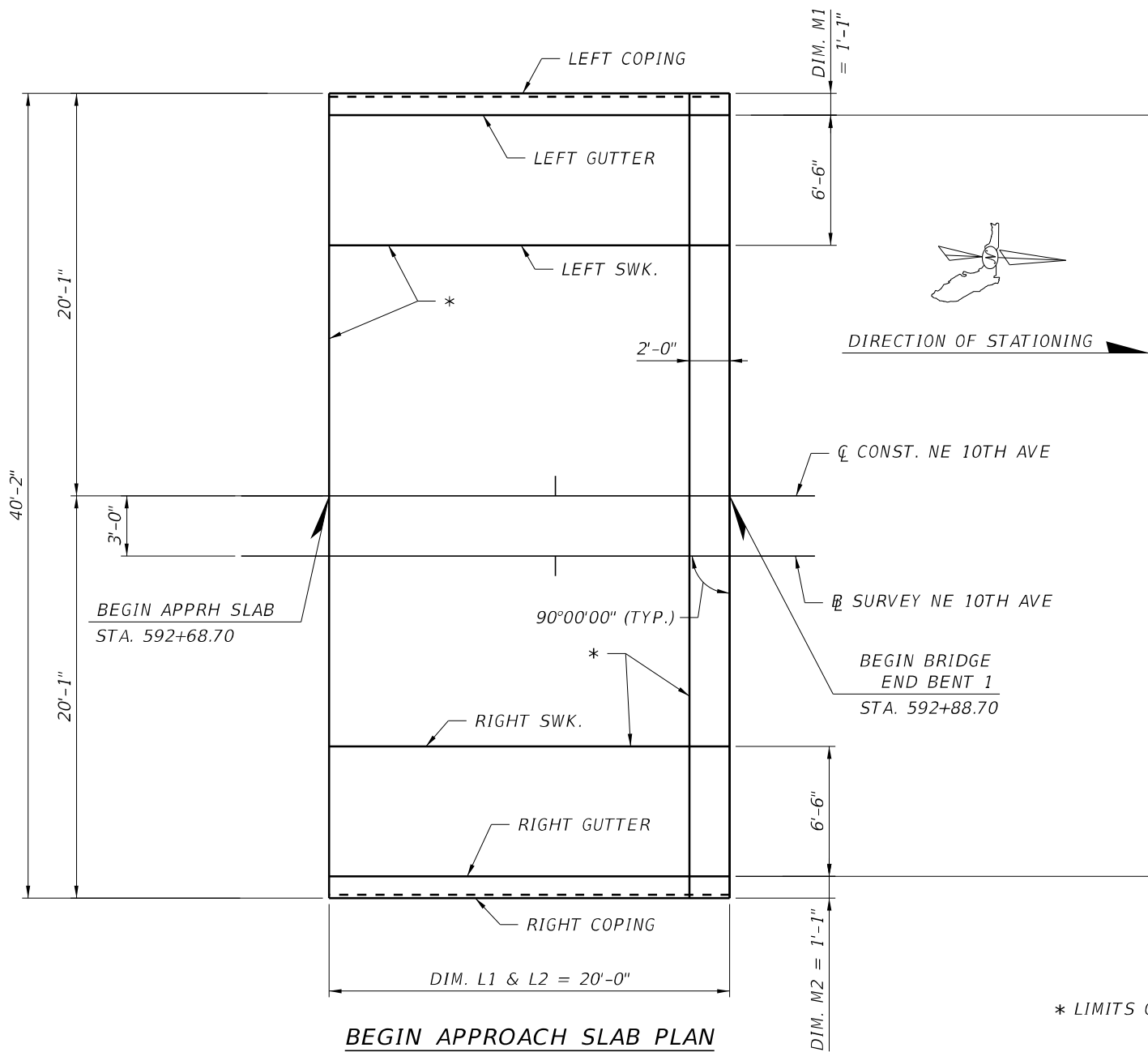
PROVIDE PLAIN ELASTOMERIC BEARING PADS WITH A SHEAR MODULUS $G = 110$ PSI AND IN ACCORDANCE WITH SPECIFICATION 932.

POURED EXPANSION JOINT DATA TABLE INDEX 458-110			Table Date 1-01-09
LOCATION	DIM. "A" @ 70°F	TOTAL DESIGN MOVEMENT	DIM. "A" ADJUSTMENT PER 10°F
END BENT 1	1"	$\frac{3}{8}$ "	$\frac{1}{32}$ "
END BENT 2	1"	$\frac{3}{8}$ "	$\frac{1}{32}$ "

NOTE:
 DIM. "A" ADJUSTMENT PER 10°F SHOWN IS MEASURED PERPENDICULAR TO \perp
 EXPANSION JOINT. WORK THIS TABLE WITH STANDARD PLANS INDEX 458-110.

BRIDGE NO. 874185

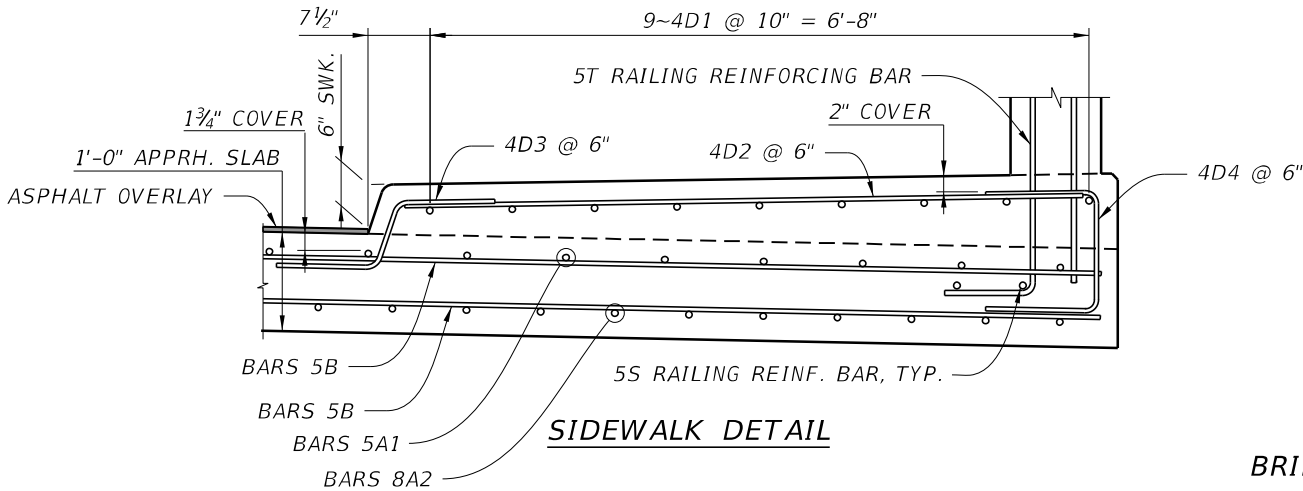
REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.		
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20				PROJECT NAME:				
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET	SHEET NO.
							CHECKED BY: BA 07-20								B1-20



* LIMITS OF ASPHALT OVERLAY

NOTE: WORK THIS DATA TABLE WITH DEVELOPMENTAL STANDARD PLANS INDEX D20920

APPROACH SLAB TABLE OF DIMENSIONS						Table Date 11-01-16
LOCATION	DIMENSIONS					ANGLE Ø
	L1	L2	M1	M2	N	
BEGIN APPROACH SLAB	20'-0"	20'-0"	1'-1"	1'-1"	38'-0"	90°
END APPROACH SLAB	20'-0"	20'-0"	1'-1"	1'-1"	38'-0"	90°
DIMENSION NOTES: DIMENSIONS L1 & L2 ARE MEASURED ALONG GUTTER LINE, INSIDE FACE OF PARAPET OR INSIDE FACE OF RAILING ON RAISED SIDEWALKS.						



BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20 CHECKED BY: SS 07-20 DESIGNED BY: SS 07-20 CHECKED BY: RA 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: APPROACH SLAB DETAILS		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION			ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
								NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	FROM NE 83RD STREET TO NE 84TH STREET		B1-21

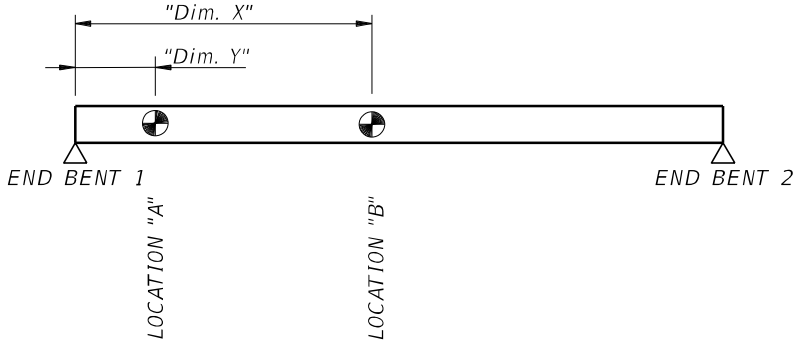
Load Rating Summary Details for Prestressed Concrete Bridges (Flat Slab and Deck/Girder)																	Table Date 07-01-15
Table 2 - LRFR																	
Level	Limit State	Vehicle	Weight (tons)	Load Factors			Moment (Strength) or Stress (Service)					Shear (Strength)					Comments:
				LL	DC	DW	Distribution Factor (DF)	Rating Factor	Tons	Location	Dimension	Distribution Factor (DF)	Rating Factor	Tons	Location	Dimension	
Design Load Rating	Strength I (Inv)	HL-93	N/A	1.75	1.25	1.50	0.41	1.85	N/A	B	22.52	0.69	5.02	N/A	A	4.25	EXTERIOR FSB 1 & 8 (M)
	Strength I (Op)	HL-93	N/A	1.35	1.25	1.50	0.41	2.40	N/A	B	22.52	0.69	6.51	N/A	A	4.25	EXTERIOR FSB 1 & 8 (M)
	Service III (Inv)	HL-93	N/A	0.80	1.00	1.00	0.41	2.66	N/A	B	22.52	N/A	N/A	N/A	N/A	N/A	EXTERIOR FSB 1 & 8 (M)
Permit Load Rating	Strength II	FL120	60.0	1.35	1.25	1.50	0.41	1.75	93.0	B	22.52	0.69	4.55	254.7	A	4.25	EXTERIOR FSB 1 & 8 (M)

GENERAL NOTES:

1. THIS TABLE IS BASED ON THE REQUIREMENTS ESTABLISHED IN THE JANUARY 2021 "STRUCTURES MANUAL".

TABLE 2 NOTES [NOTES DATE 07-01-15]:

1. PERMIT CAPACITY IS DETERMINED BY USING THE PERMIT VEHICLE IN ALL LANES.
2. SERVICE III DESIGN INVENTORY TENSILE STRESS LIMITS = $3\sqrt{f'_c}$ OR $6\sqrt{f'_c}$.
3. HAS THE AASHTO LRFD SPECIFICATIONS ARTICLE 5.8.3.5 LONGITUDINAL REINFORCEMENT BEEN SATISFIED? ☒ YES ☐ NO
4. BRIDGE RATING PERFORMED USING FDOT LRFD PRESTRESSED BEAM PROGRAM v.5.2



RATING LOCATIONS

Abbreviations:

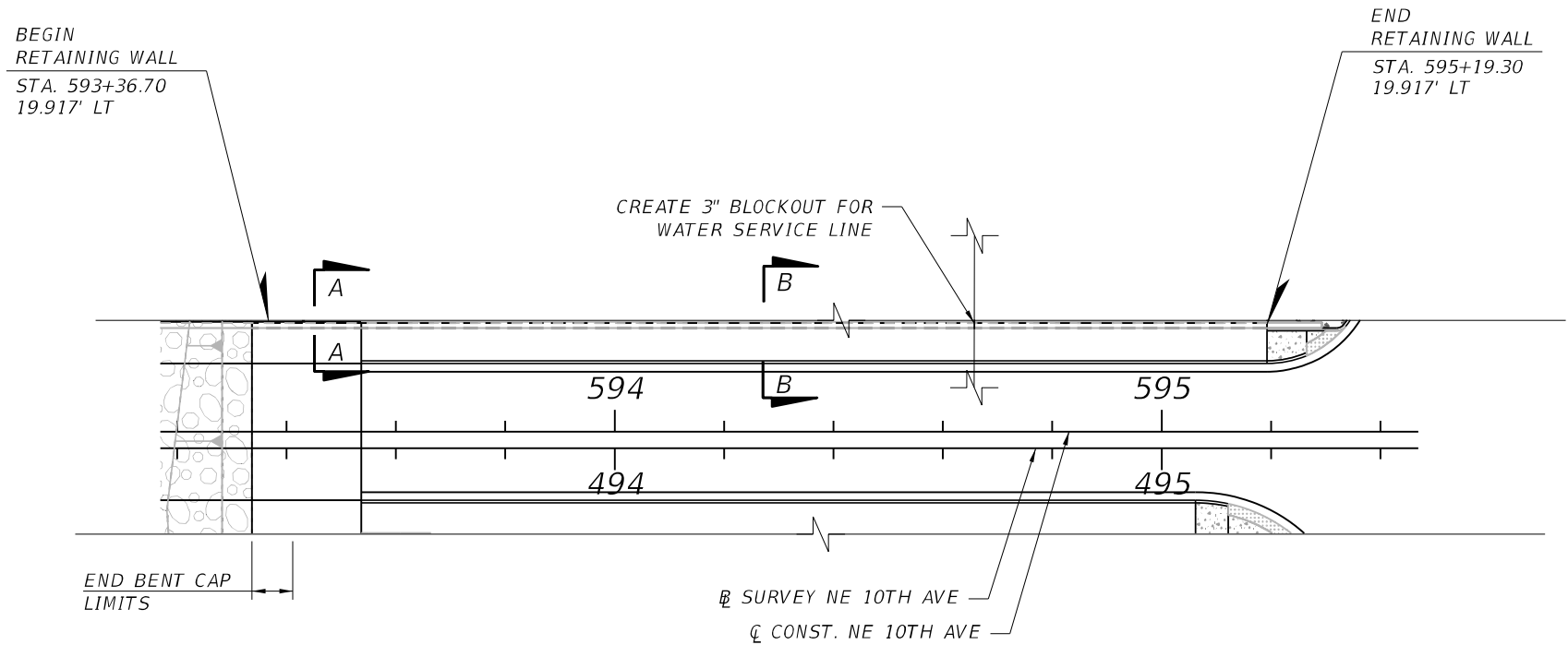
Inv - Inventory

Op - Operating

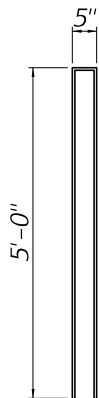
BRIDGE NO. 874185

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE:		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20				LOAD RATING SUMMARY SHEET		
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	SHEET NO.	
							CHECKED BY: RA 07-20	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET	B1-23	

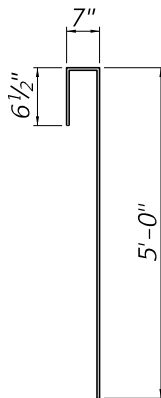
*REFER TO APPROACH SLAB DRAWINGS
FOR REINFORCING DETAILS



PLAN VIEW



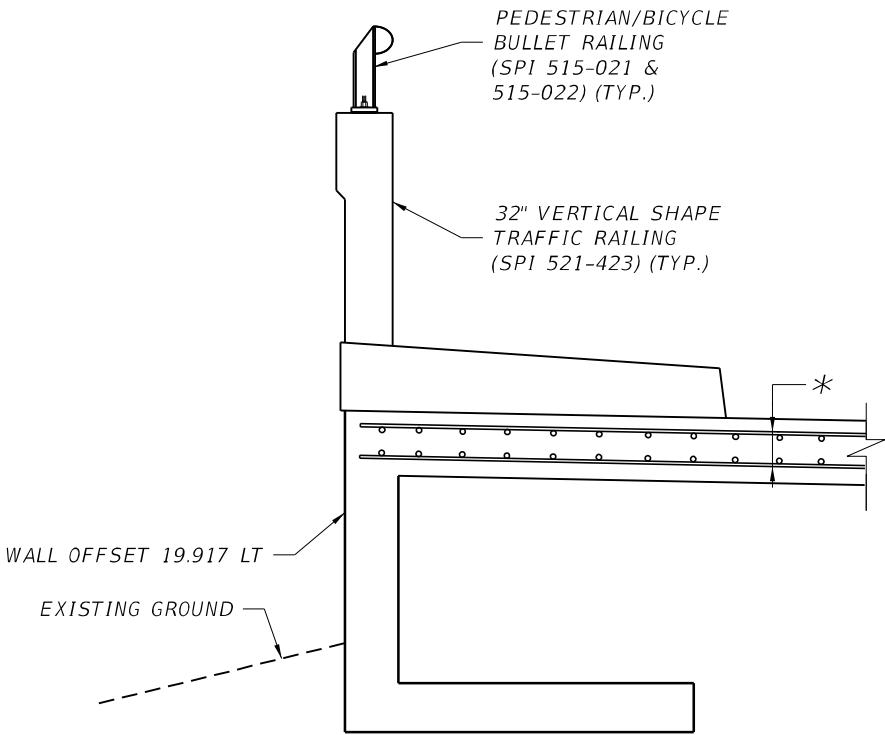
MODIFICATIONS TO BARS 5T



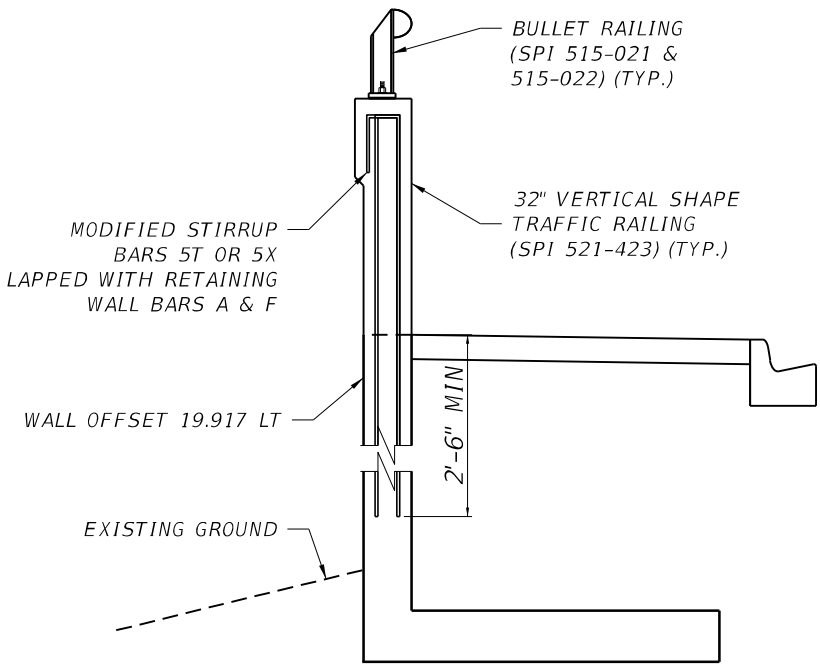
MODIFICATIONS TO BARS 5X

TRAFFIC RAILING MOUNTED ON RETAINING WALL (Sta. 593+53.70 to Sta. 595+19.30)

- NOTES:
- WORK BAR BENDING DIAGRAM WITH STANDARD INDEX 415-001.
 - FOR 32" VERTICAL SHAPE TRAFFIC RAILING, WORK WITH STANDARD INDEX 521-423.



SECTION A-A
FROM STA. 593+36.70 TO STA. 593+53.70
(RETAINING WALL REINFORCING NOT SHOWN FOR CLARITY)



SECTION B-B
FROM STA. 593+53.70 TO STA. 595+19.30
(RETAINING WALL REINFORCING NOT SHOWN FOR CLARITY)

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: RETAINING WALL DETAILS (1 OF 2)		REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20						
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	SHEET NO.	
							CHECKED BY: RA 07-20	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196	NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET	B1-24	

THE OFFICIAL RECORD OF THIS SHEET IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, F.A.C.

C-I-P CANTILEVER RETAINING WALLS DATA TABLES

WALL DIMENSIONS																									
WALL NO.	BEGIN		END		HEIGHT				WALL LENGTH		D	W	L FOOT		L TOE		SLOPE BACKWALL	D SOIL*		L KEY	D KEY	V STEP	WALL COVER	FTG COV (TYP.)	FTG COV (BOT.)
	STATION	OFFSET	STATION	OFFSET	BEGIN		END		FT	IN	IN	IN	FT	IN	FT	IN		FT	IN	IN	IN	IN	IN	IN	IN
					FT	IN	FT	IN																	
1	593+36.70	19.917	593+53.70	19.917	5	6	4	0	17	0	18	11	6	6	0	0	0	1	0	0	0	0	3	3	4.5
2	593+53.70	19.917	595+19.30	19.917	4	0	3	0	165	7	18	11	4	0	0	0	0	1	0	0	0	0	3	3	4.5

* MNIMUM SOIL COVER

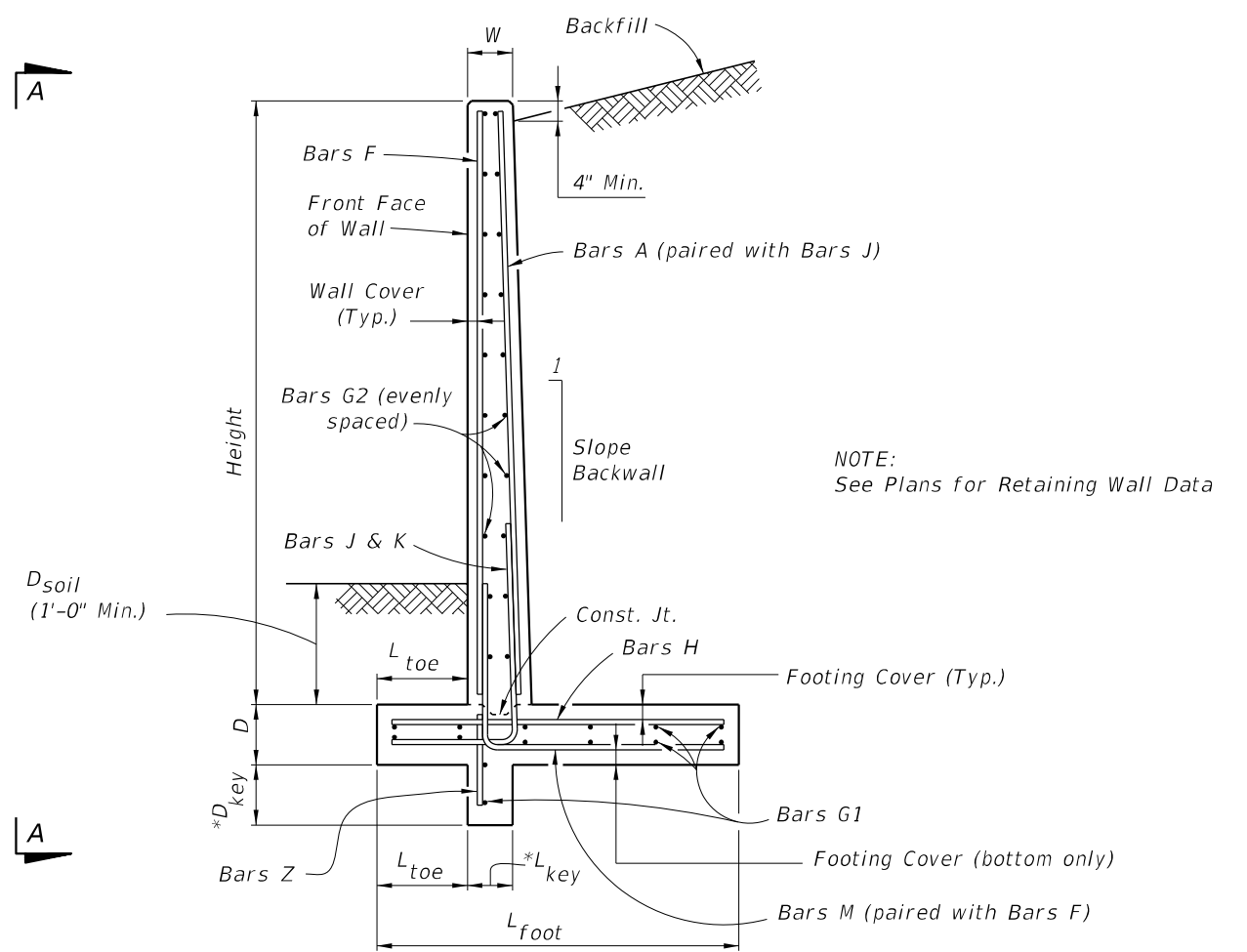
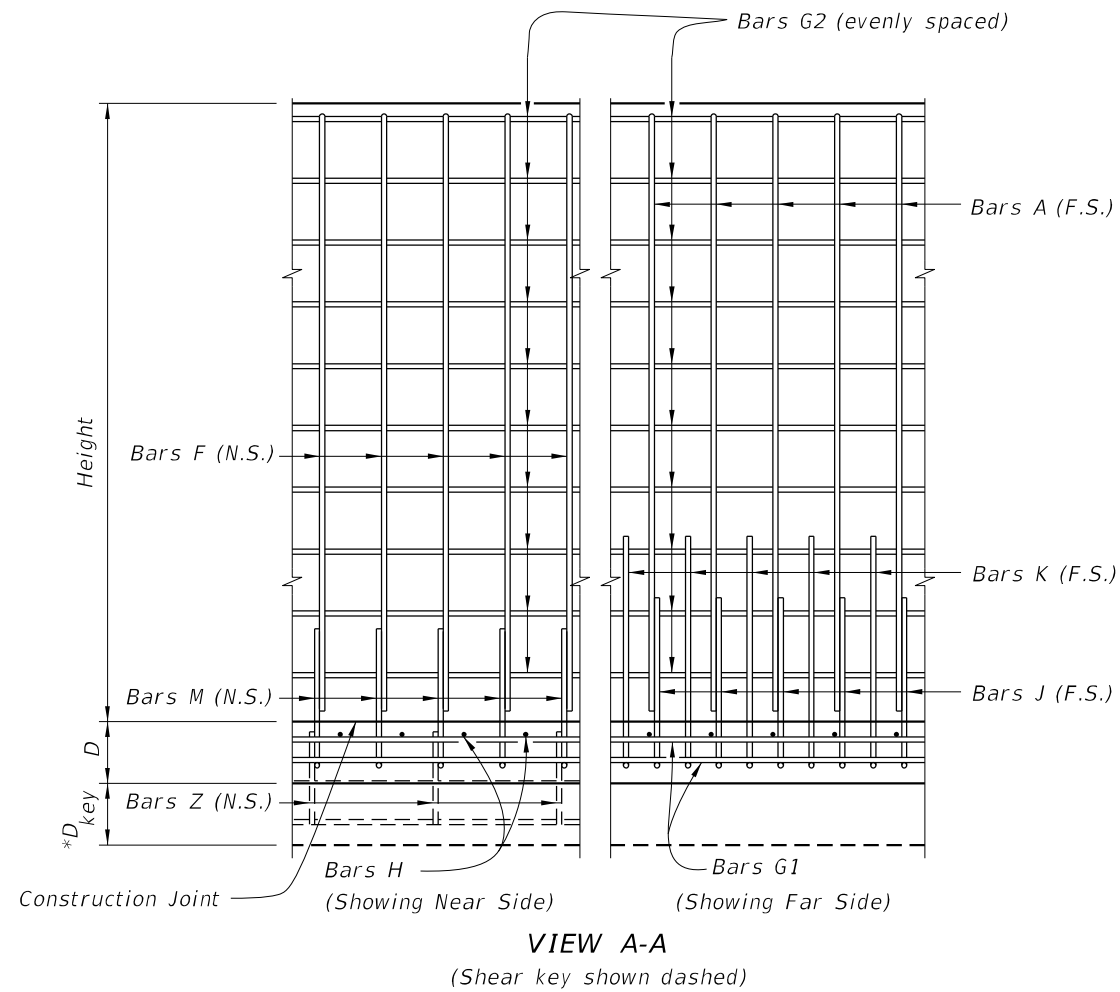
BILL OF REINFORCING STEEL																																			
WALL NO.	BARS J											BARS K										BARS M													
	SIZE	NO.	SPACING	A				B		AVERAGE		SIZE	NO.	SPACING	A				B		AVERAGE		SIZE	NO.	A		B		TOTAL LENGTH						
				BEGIN		END				TOTAL LENGTH					BEGIN		END				TOTAL LENGTH				A		B		TOTAL LENGTH						
				IN	FT	IN	FT			IN	FT				IN	FT	IN	FT			IN	FT			IN	FT	IN	FT	IN	FT	IN	FT	IN	FT	IN
				IN	FT	IN	FT	IN	FT	IN	FT				IN	FT	IN	FT	IN	FT	IN	FT			IN	FT	IN	FT	IN	FT	IN	FT	IN	FT	IN
1	#5	18	12	3	6	3	6		8	4	2	#5	18	12	4	6	4	6		8	5	2	#4	18	3	6	6	0	9	6					
2	#5	166	12	3	6	2	6		8	3	2	#5	166	12	4	6	3	9		8	4	2	#4	166	3	6	3	6	7	0					

BILL OF REINFORCING STEEL																														
WALL NO.	BARS H					BARS G1						BARS R				BARS Z				BARS A										
	SIZE	NO.	SPACING	LENGTH		SIZE	NO.	SPACING	NO. OF LAP SPLICES	TOTAL LENGTH		SIZE	NO.	LENGTH		SIZE	NO.	SPACING	LENGTH		SIZE	NO.	LENGTH				AVERAGE LENGTH			
																							BEGIN						END	
																							FT	IN	FT	IN			FT	IN
1	#5	18	12	6	0	#5	14	12	0	16	6	#5	14	2	0						#4	18	5	3	3	9	4	6		
2	#5	166	12	3	6	#5	8	12	2	170	0	#5	8	2	0						#4	166	3	9	2	9	3	3		

BILL OF REINFORCING STEEL																	
WALL NO.	BARS F									BARS G2				BARS D			
	SIZE	NO.	SPACING	LENGTH		LENGTH		AVERAGE LENGTH		SIZE	NO.	LENGTH		SIZE	NO.	LENGTH	
				BEGIN		BEGIN											
				IN	FT	IN	FT	IN	FT			IN	FT			IN	
1	#4	18	12	5	3	3	9	4	6	#4	12	16	6	#5	6	1	6
2	#4	166	12	3	9	2	9	3	3	#4	8	170	0	#5	28	1	6

- NOTES [Notes Date 07-01-14]:
- Work these Data Tables with Index 400-010.
 - Concrete Class IV (f'c = 5500 psi) *with* silica fume, metakaolin or ultrafine fly ash.
 - Wall exposed face surface texture shall be SMOOTH FINISH.
 - Environmental Classification is EXTREMELY AGGRESSIVE.
 - Minimum Soil Nominal Bearing Resistance = 3500 psf.
 - A value of '0' for Slope Backwall indicates front and back of wall are parallel.
 - D_{soil} is typical depth of soil and is used for design purposes only. See Control Drawings for actual ground line.
 - Non-zero values for L_{key} and D_{key} indicate the existence of a shear key.
 - A non-zero value for V_{step} indicates the existence of a footing step, see Control Drawings for location.
 - Bars J, K, A and F vary uniformly between begin and end wall heights as indicated by begin and end dimensions.
 - The number of G1 Bars includes 2 additional bars when a shear key is specified.
 - For walls with variable begin/end height, Bars G2 shall be fanned such that they are evenly spaced throughout length of wall.

REVISIONS						ASA CONSULTANTS, INC. 510 SHOTGUN ROAD, SUITE 402 SUNRISE, FL 33326 (754)-216-2027 SOHEILA SADOUGH, PE; LIC. NO. 44130	DRAWN BY: EZ 07-20	MIAMI-DADE COUNTY PUBLIC WORKS DEPARTMENT			SHEET TITLE: RETAINING WALL DETAILS (2 OF 2)			REF. DWG. NO.
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		CHECKED BY: SS 07-20							
							DESIGNED BY: SS 07-20	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME: NE 10TH AVENUE FROM NE 83RD STREET TO NE 84TH STREET			SHEET NO.
							CHECKED BY: RA 07-20	NE 10 AVE	MIAMI-DADE	EDP-MT-20190196				B1-25

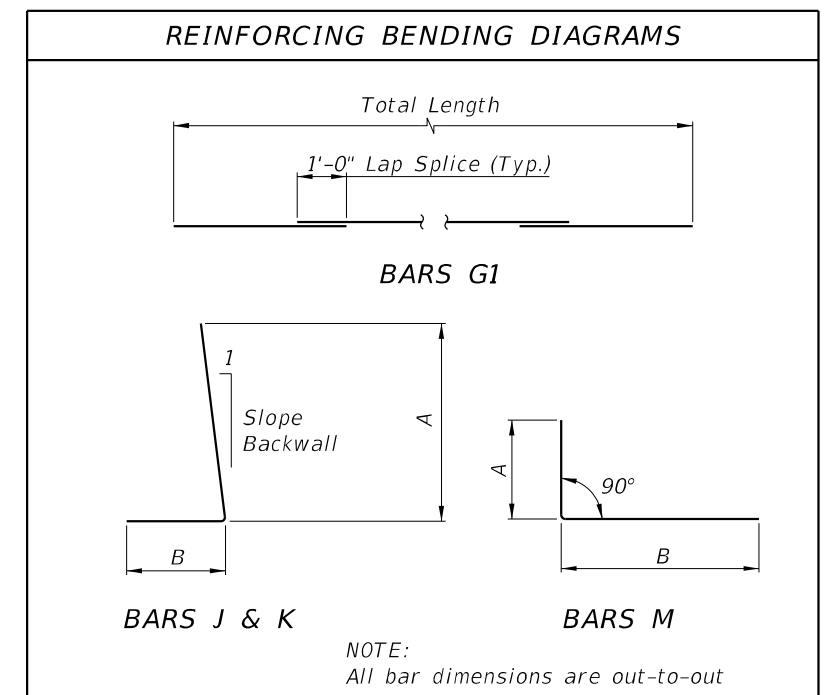


* Shear Key is required only when specified in the Plans.


NOTES

TRAFFIC RAILINGS OR PARAPETS:
If there is a Traffic Railing or Parapet on the wall, align Wall Joints with V-Grooves, and Wall Expansion Joints with Barrier Open Joints.

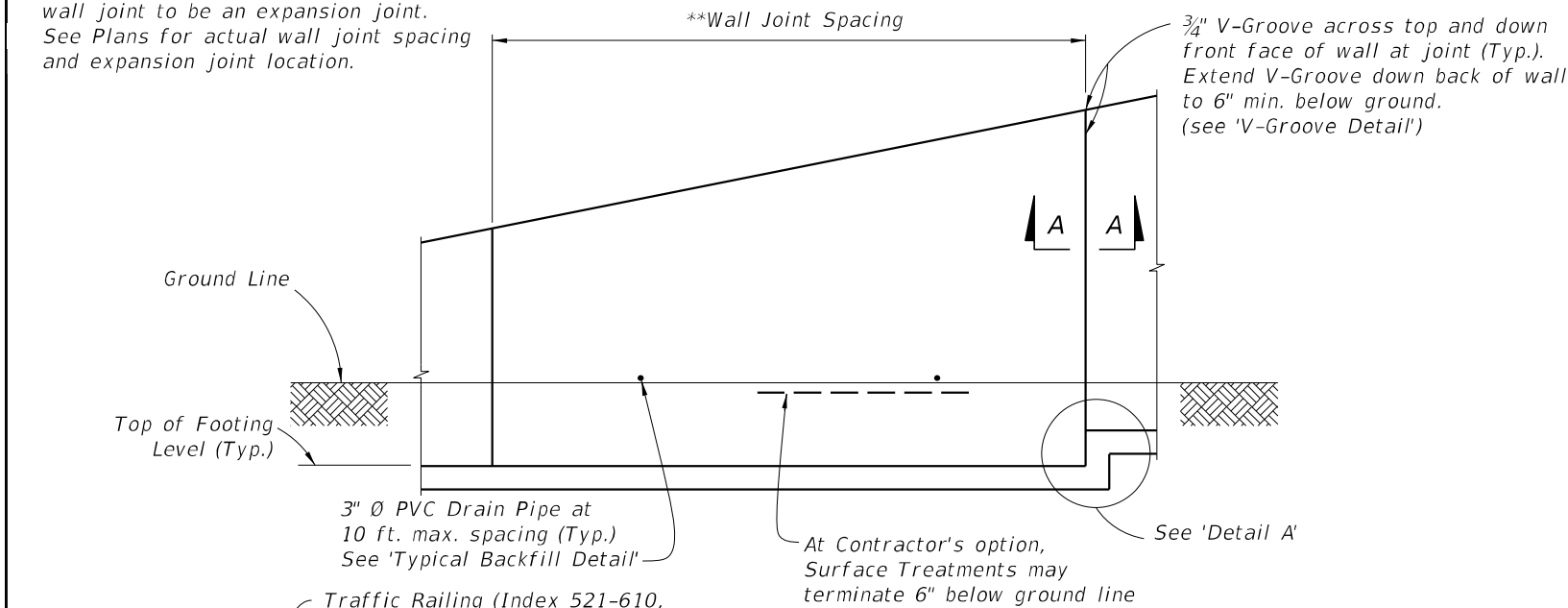
FOUNDATION:
Prepare the soil below the footing in accordance with the requirements for spread footings in Specification Section 455.



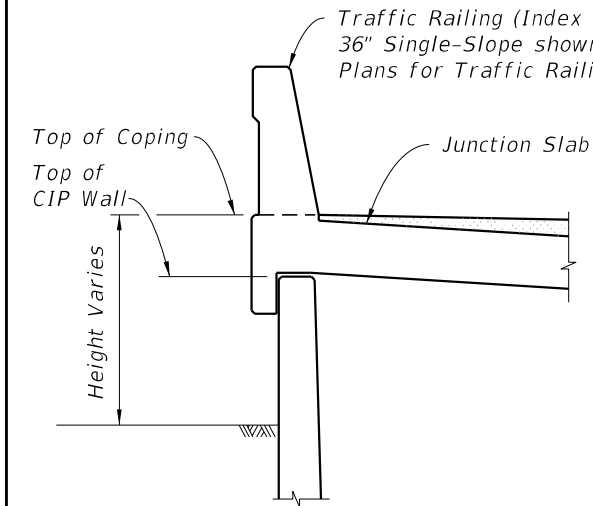
9/10/2024 11:26:51 AM

LAST REVISION 11/01/20	REVISION	DESCRIPTION:	 FY 2025-26 STANDARD PLANS	CANTILEVER RETAINING WALL (C-I-P)	INDEX 400-010	SHEET 1 of 2
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**Wall joint spacing 25 ft. maximum and 5' minimum. At minimum, every fourth wall joint to be an expansion joint. See Plans for actual wall joint spacing and expansion joint location.

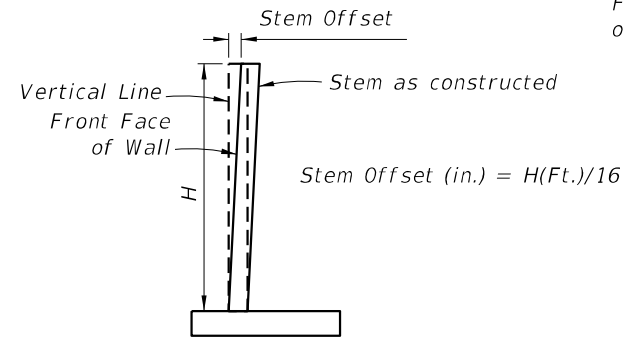


FRONT ELEVATION



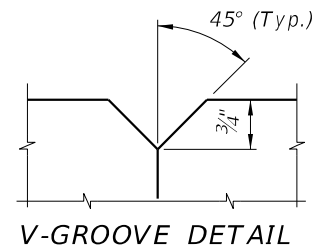
TRAFFIC RAILING/JUNCTION SLAB DETAIL

(36" Single-Slope shown, other Traffic Railings similar)

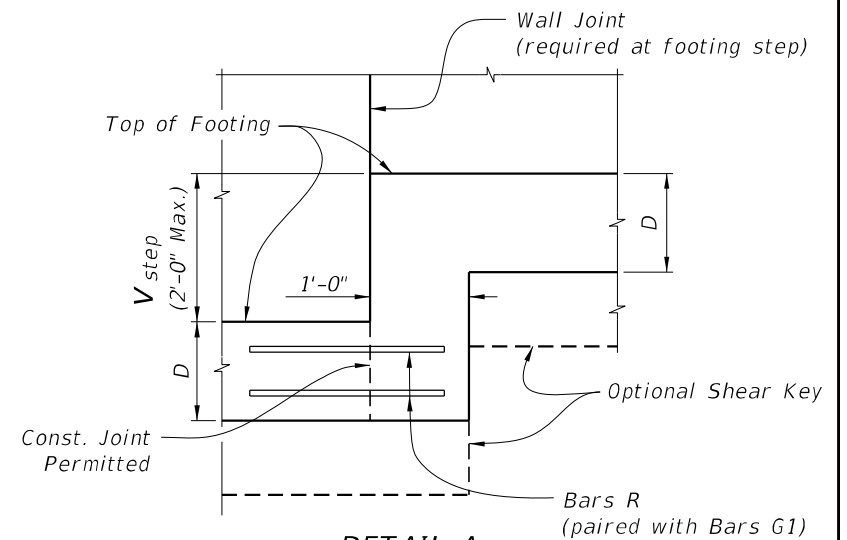


STEM OFFSET VALUES

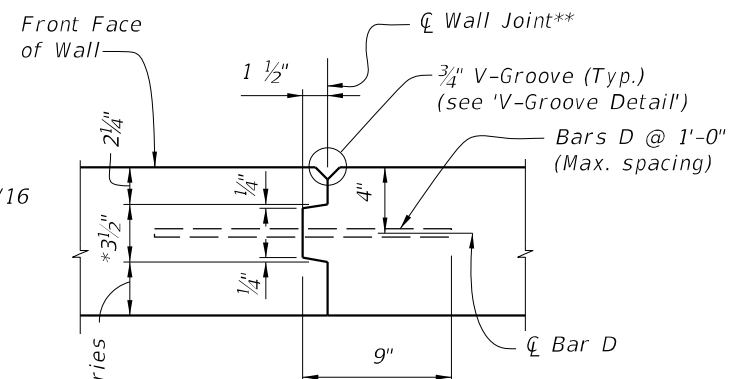
(for H < 20 Ft.)



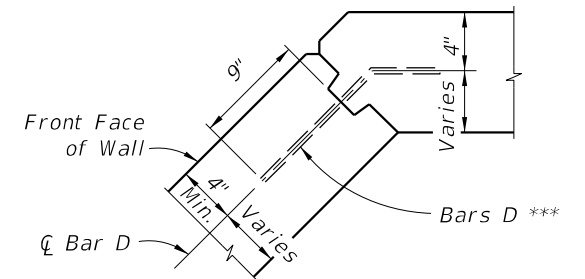
V-GROOVE DETAIL



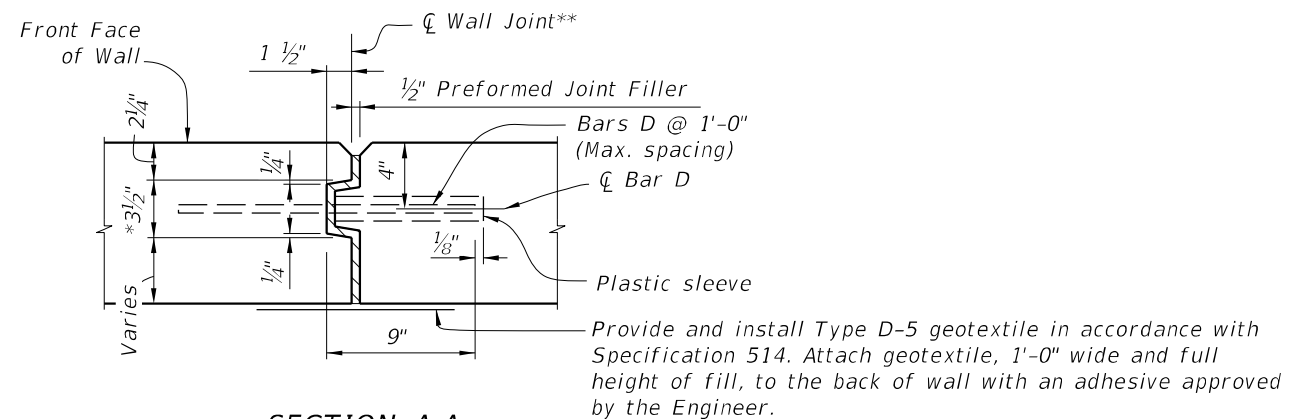
DETAIL A



SECTION A-A
WALL JOINT DETAIL



TYPICAL CORNER JOINT DETAIL

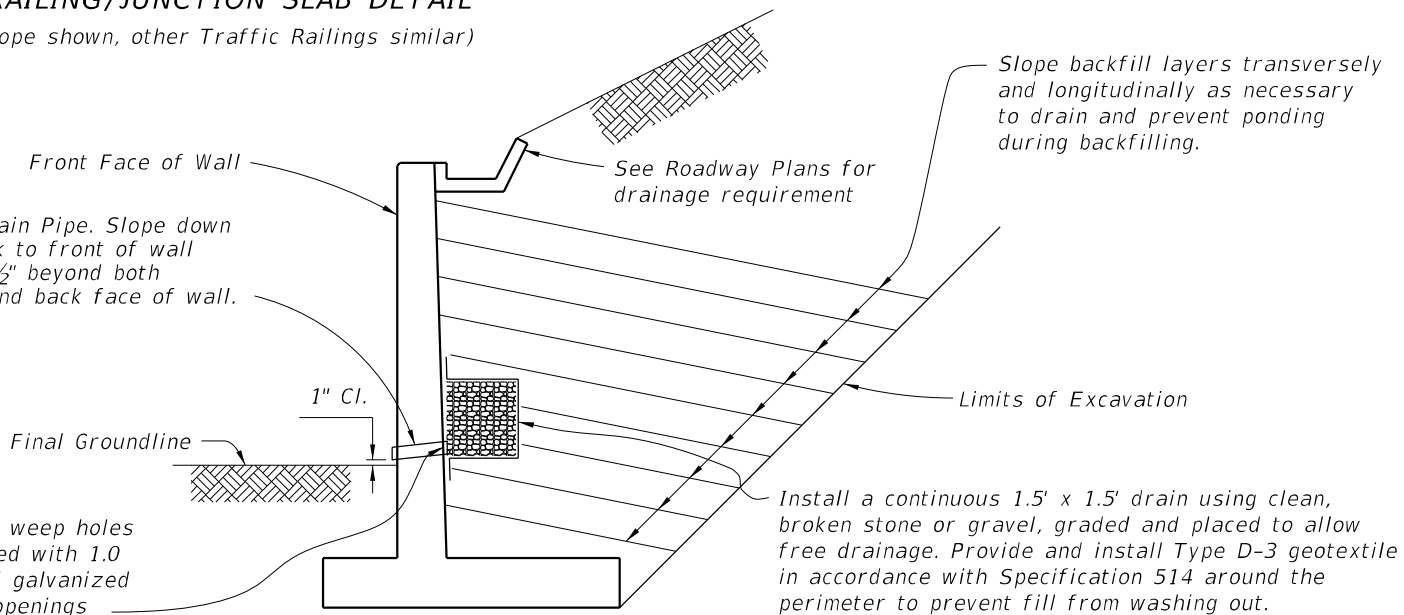


SECTION A-A
EXPANSION JOINT DETAIL

* Key to stop at top of footing and 6" from top of wall. Joint across footing and top of wall to be a straight line.

** Stay-In-Place Plastic Preformed Bond Beakers are permitted to form joints.

*** Bars D: Where a bend is required to maintain clearances, submit bend angle to GFRP supplier; steel reinforcing may be field bent.

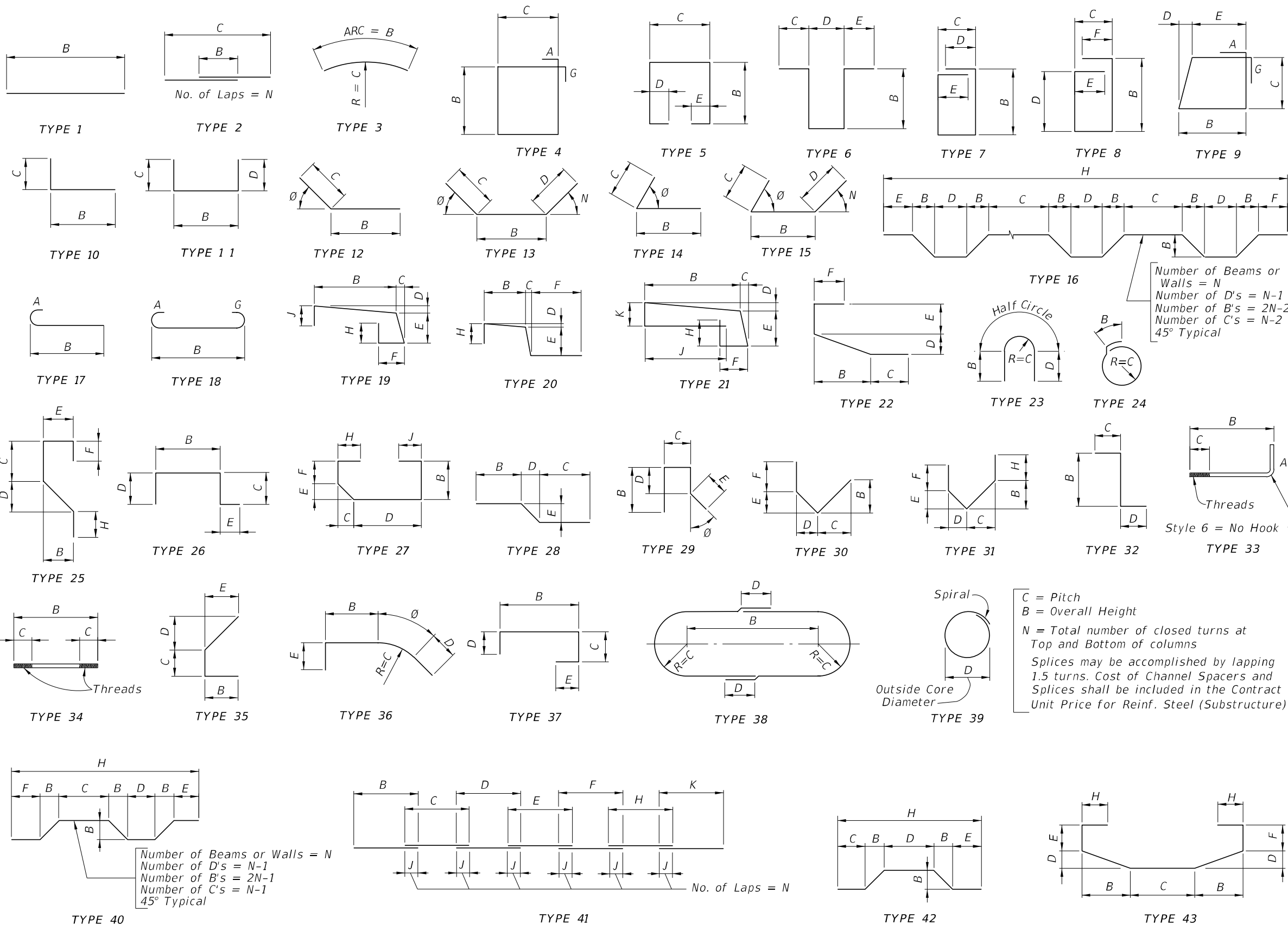


TYPICAL BACK-FILL DETAIL

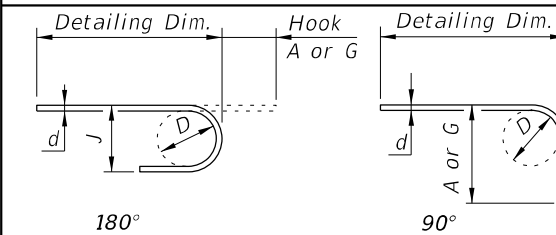
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LAST REVISION	DESCRIPTION:	FDOT	FY 2025-26 STANDARD PLANS	CANTILEVER RETAINING WALL (C-I-P)	INDEX	SHEET
11/01/23					400-010	2 of 2

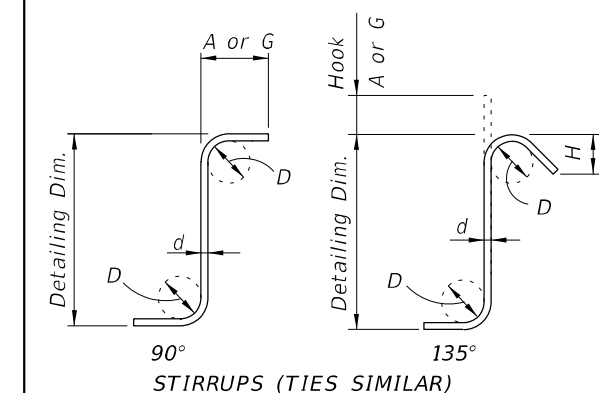
10/17/2024 8:17:12 AM



HOOK DETAILS



BAR SIZE	D	180° HOOKS		90° HOOKS
		A OR G	J	A OR G
#3	2 1/4"	5"	3"	6"
#4	3"	6"	4"	8"
#5	3 3/4"	7"	5"	10"
#6	4 1/2"	8"	6"	1'-0"
#7	5 1/4"	10"	7"	1'-2"
#8	6"	11"	8"	1'-4"
#9	9 1/2"	1'-3"	11 3/4"	1'-7"
#10	10 3/4"	1'-5"	1'-1 1/4"	1'-10"
#11	12"	1'-7"	1'-2 3/4"	2'-0"
#14	18 1/4"	2'-3"	1'-9 3/4"	2'-7"
#18	24"	3'-0"	2'-4 1/2"	3'-5"
STYLE		1		3



STIRRUP & TIE HOOK DIMENSIONS

BAR SIZE	D	90° HOOKS	135° HOOKS	
		A or G	A or G	H *
#3	1 1/2"	4"	4"	2 1/2"
#4	2"	4 1/2"	4 1/2"	3"
#5	2 1/2"	6"	5 1/2"	3 3/4"
#6	4 1/2"	1'-0"	8"	4 1/2"
#7	5 1/4"	1'-2"	9"	5 1/4"
#8	6"	1'-4"	10 1/2"	6"
STYLE		4		5

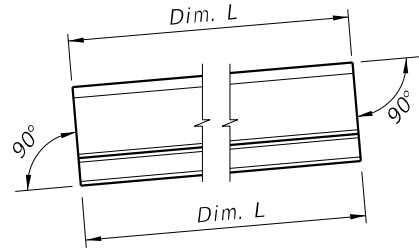
STYLE 6 = NO HOOK

* Dimension is approximate.
Hook Styles Detailed on this sheet are for Illustration Only.
Actual Hook Style for any particular bar will be shown under A or G Heading on REINFORCING BAR LIST sheet(s) in Structures Plans.
All Dimensions are out-to-out.

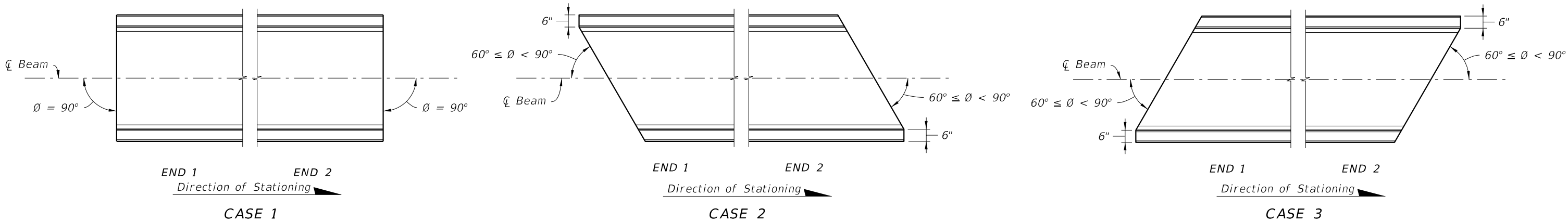
NOTE: For Bar Dimensions See REINFORCING BAR LIST Sheet(s) in Structures Plans.

LAST REVISION	DESCRIPTION:	FY 2025-26 STANDARD PLANS		BAR BENDING DETAILS (STEEL)		INDEX	SHEET
11/01/24		FDOT				415-001	1 of 1


- FABRICATION NOTES
- 1. The abbreviated FSB designation for depth and width is FSB "depth" x "width", e.g. FSB 12 x 48.
 - 2. All bar dimensions are out-to-out.
 - 3. Strands N shall be ASTM A416, Grade 250 or 270, $\frac{3}{8}$ " \varnothing or larger strands, stressed to 10,000 lbs. each.
 - 4. Unless otherwise noted, the minimum concrete cover for reinforcing steel shall be 2".
 - 5. For referenced Dimensions, Angles and Case Numbers, see Florida Slab Beam - Table of Variables in Structures Plans.
 - 6. Bars 4D1 & 6Y1 correspond to END 1, and 4D2 & 6Y2 correspond to END 2.
 - 7. Bars 5E1 correspond to interior FSBs, and 5E2 correspond to exterior FSBs.
 - 8. Rake the top surface of the Slab Beams transversely to provide a roughened surface with $\frac{1}{4}$ " amplitude.
 - 9. Embedment of Safety Line Anchorage Devices are permitted to accommodate full protection systems. See shop drawings for details and spacings.

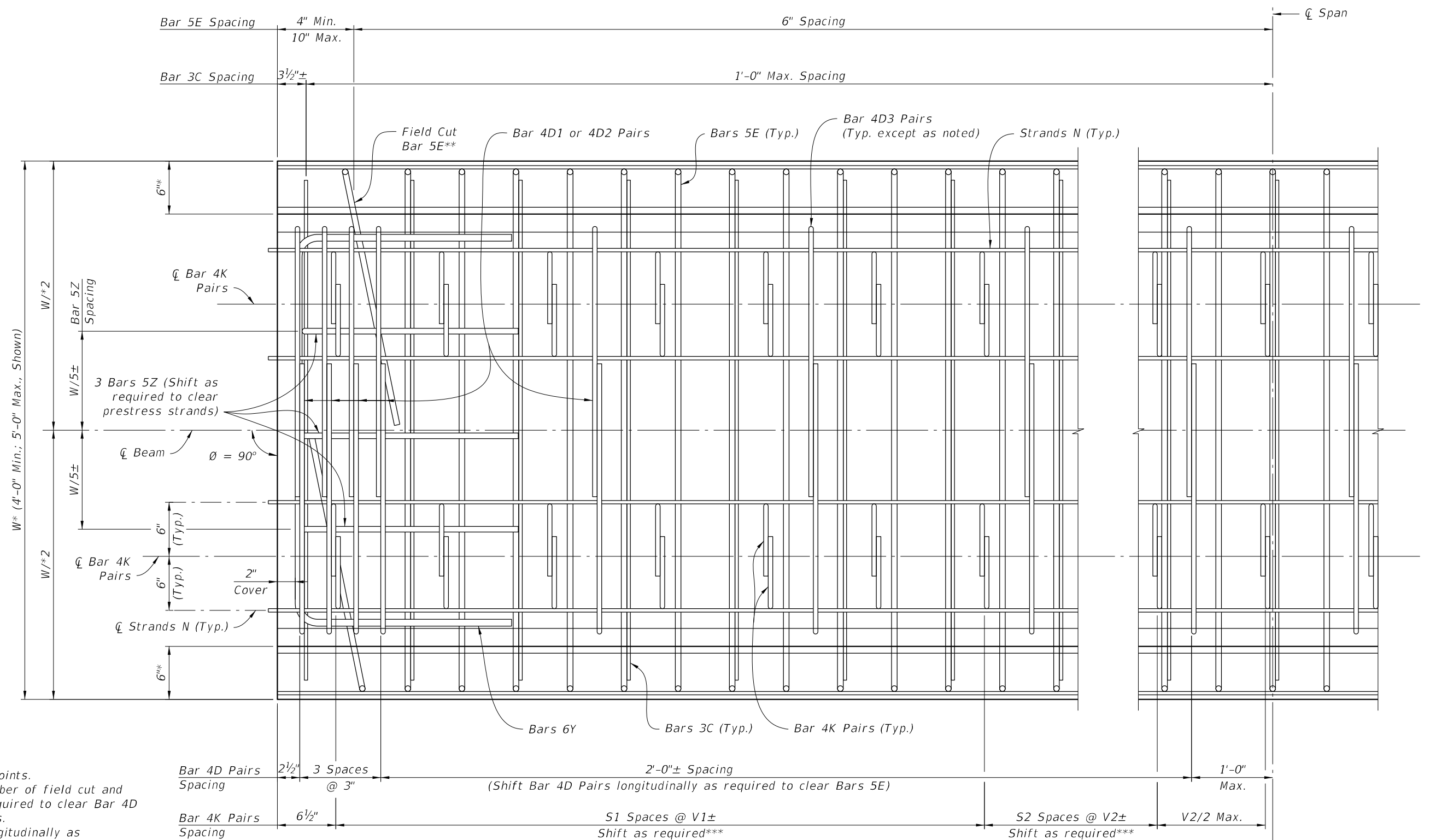


SCHEMATIC SIDE ELEVATION OF BEAM
(Beam on a Positive Grade shown; Beam on a Negative Grade or Horizontal Grade similar.)



SCHEMATIC PLAN VIEWS AT BEAM ENDS

LAST REVISION 04/01/23	REVISION	DESCRIPTION:	 FY 2025-26 STANDARD PLANS	FLORIDA SLAB BEAM TYPICAL DETAILS AND NOTES	INDEX 450-450	SHEET 1 of 3
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* Measured to Working Points.
 ** At Beam ends, use number of field cut and rotated Bars 5E as required to clear Bar 4D Pairs and Bar 4K Pairs.
 *** Shift Bar 4K Pairs longitudinally as required to clear Bar 4D Pairs and Bars 3C and 5E.

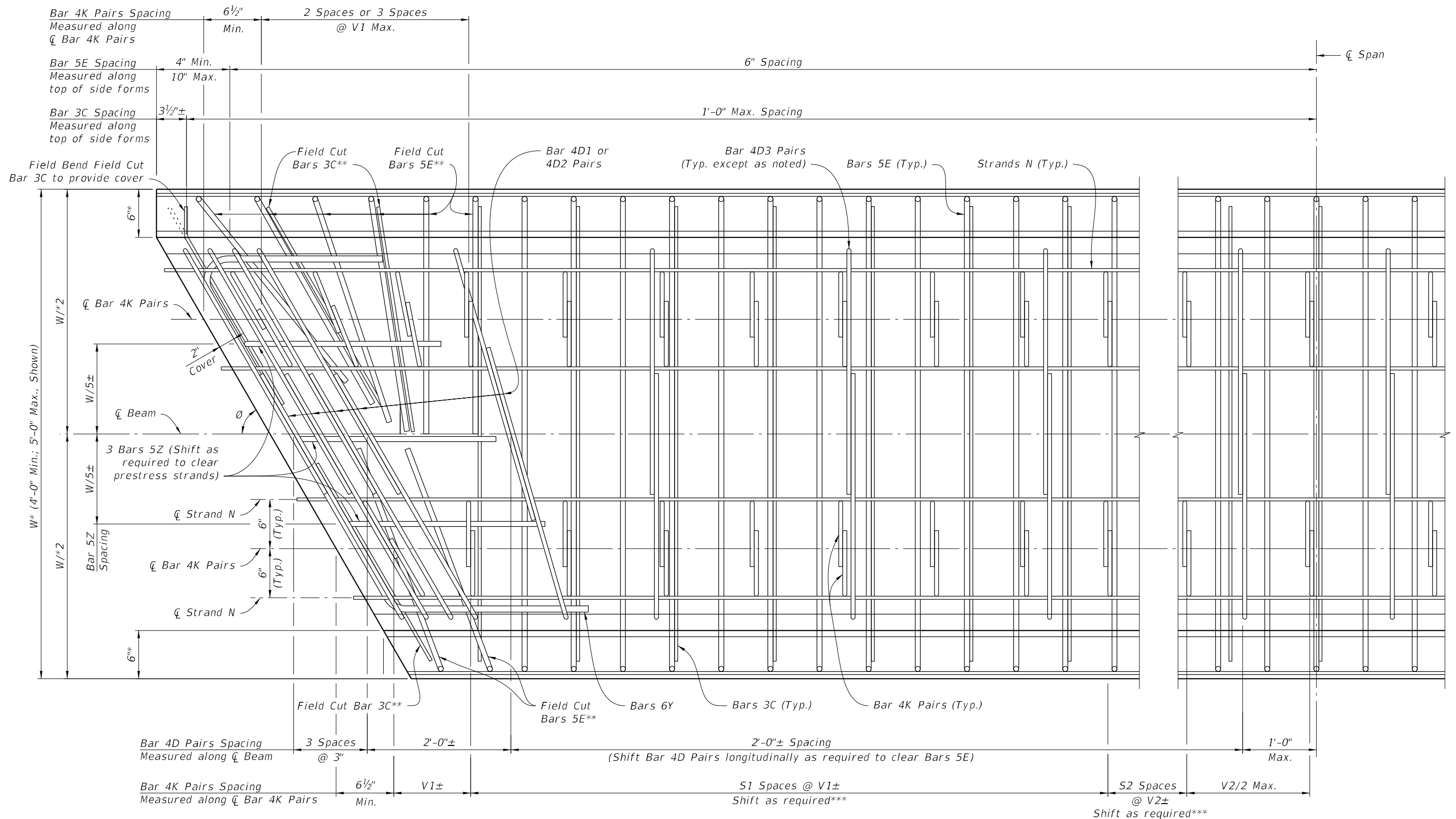
CROSS REFERENCE:
 For Dimensions V1, V2 & W and number of spaces S1 & S2, see Florida Slab Beam - Table of Variables in Structures Plans.
 See Indexes 450-451, 450-452 and 450-453 for Bars 5E Details.

PARTIAL PLAN VIEW $\theta = 90^\circ$
 (END 1 SHOWN, END 2 SIMILAR)
 (INTERIOR BEAM SHOWN, EXTERIOR BEAM SIMILAR, SEE BARS 5E DETAILS)

10/17/2024 9:49:29 AM

LAST REVISION 04/01/23	REVISION DESCRIPTION:	 FY 2025-26 STANDARD PLANS	FLORIDA SLAB BEAM TYPICAL DETAILS AND NOTES	INDEX 450-450	SHEET 2 of 3
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10/17/2024 9:50:10 AM



PARTIAL PLAN VIEW $60^\circ \leq \theta < 90^\circ$
(END 1 SHOWN, END 2 SIMILAR)
(INTERIOR BEAM SHOWN, EXTERIOR BEAM SIMILAR, SEE BARS 5E DETAILS)

CROSS REFERENCE:
For Dimensions V1, V2 & W, Angle θ and number of spaces S1 & S2,
see Florida Slab Beam - Table of Variables in Structures Plans.
See Indexes 450-451, 450-452 and 450-453 for Bars 5E Details.

* Measured to Working Points.

** At Beam ends, use number of field cut and rotated Bars 3C and 5E as required to clear Bar 4D Pairs and Bar 4K Pairs.

*** Shift Bar 4K Pairs longitudinally as required to clear Bar 4D Pairs and Bars 3C and 5E.

LAST
REVISION
04/01/23

REVISION

DESCRIPTION:



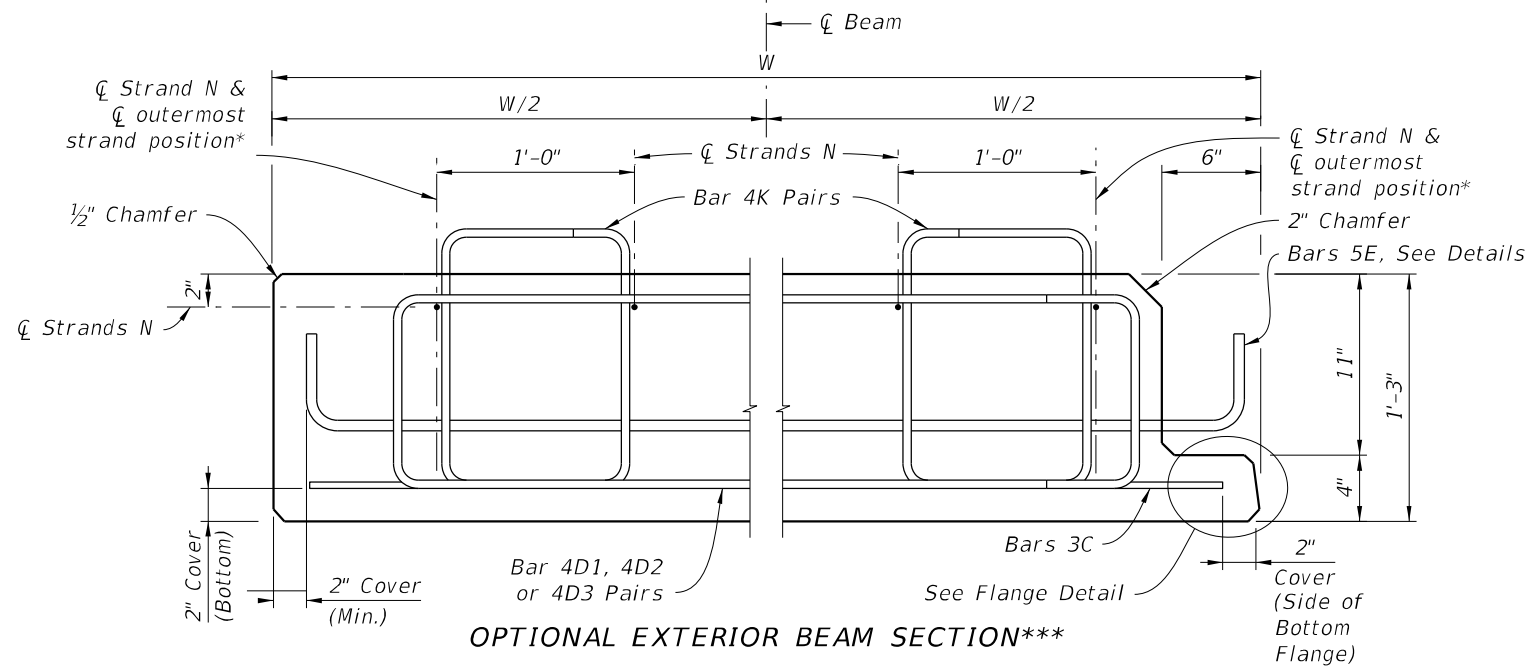
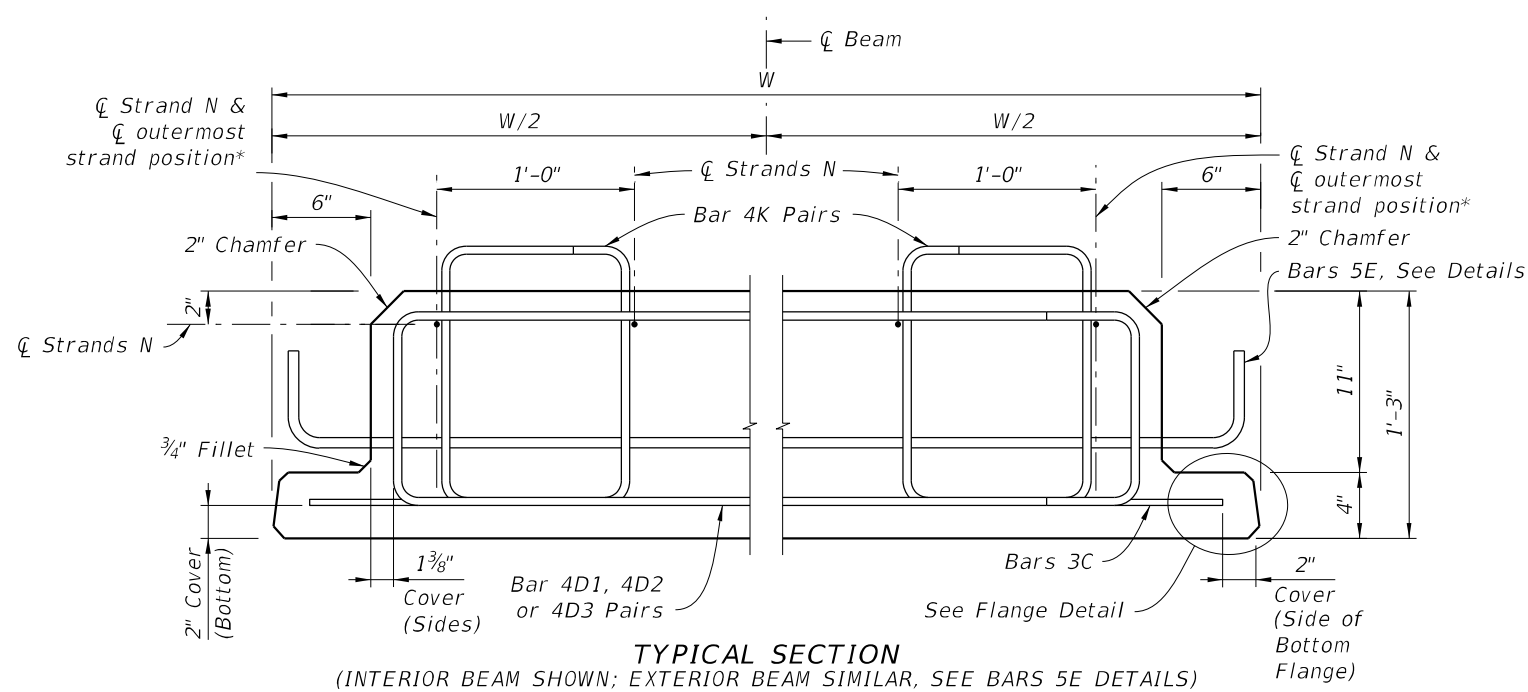
FY 2025-26
STANDARD PLANS

FLORIDA SLAB BEAM TYPICAL
DETAILS AND NOTES

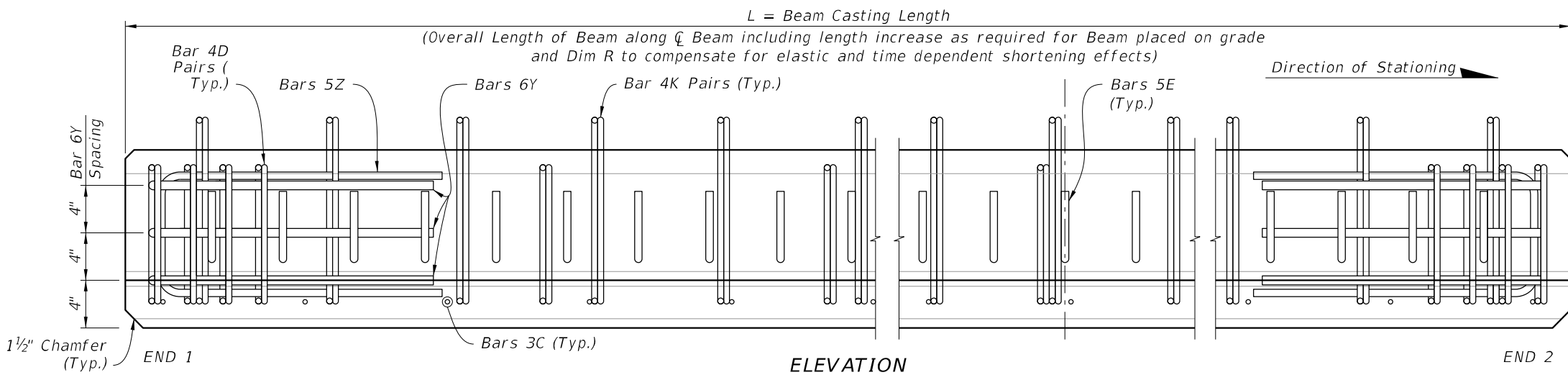
INDEX
450-450

SHEET
3 of 3

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*** At the Contractor's option, the Optional Exterior Beam Section may be used.



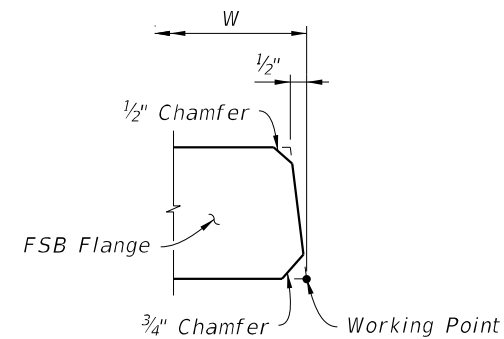
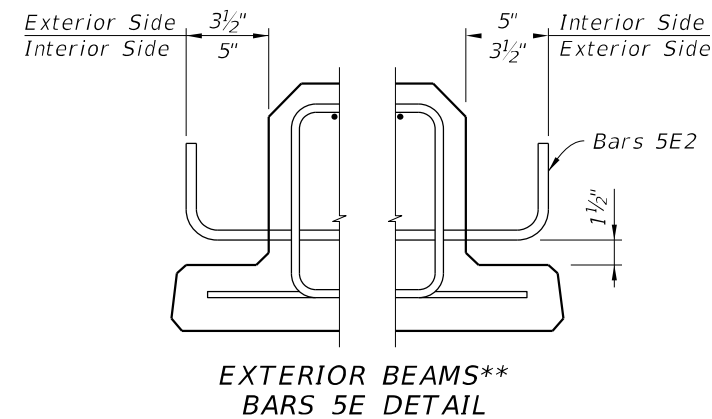
NOTES:
Work this Index with Index 450-450 and Florida Slab Beams - Table of Variables in Structures Plans.

For Dimensions C, D, E, L, R, W & Y and Angle θ , see Florida Slab Beam - Table of Variables in Structures Plans.

For referenced notes, see Index 450-450, Sheet 1.

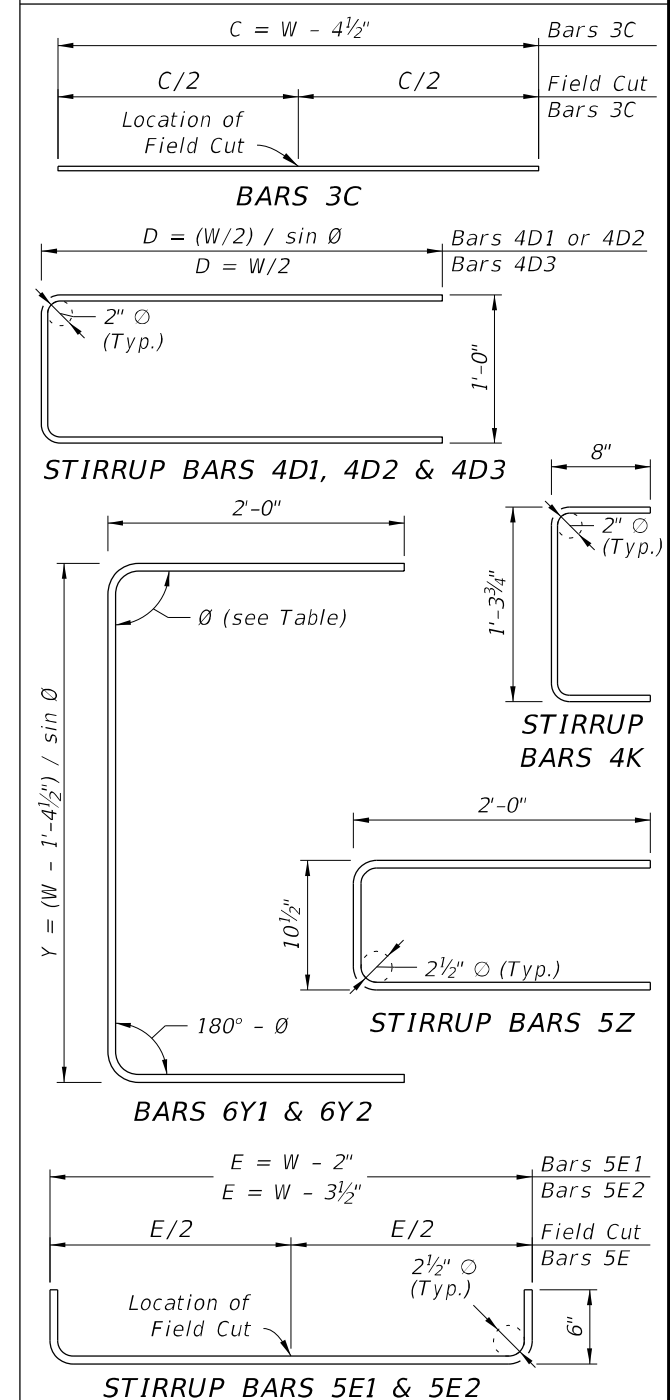
* For ϕ of outermost strand positions, see corresponding Strand Pattern on Florida Slab Beams - Table of Variables in Structures Plans.

** At the Contractor's option, the Detail as shown for Interior Beams may be used for Exterior Beams and the Bars 5E field bent on the exterior side of the Beam to provide the specified cover to the coping line.



BILL OF REINFORCING STEEL FOR ONE BEAM ONLY				
MARK	SIZE	NOTE NUMBERS	NUMBER REQUIRED	LENGTH (NOTE 1)
C	3		See Table	Varies
D1	4	6	10 (End 1)	Varies
D2	4	6	10 (End 2)	Varies
D3	4		See Table	Varies
E1	5	7	See Table	Varies
E2	5	7	See Table	Varies
K	4		See Table	2'-8"
N	See Note 3	3	4	Varies
Y1	6	6	3 (End 1)	Varies
Y2	6	6	3 (End 2)	Varies
Z	5		6	4'-10 1/2"

BENDING DIAGRAMS (See Note 1)



LAST
REVISION
11/01/20

REVISION
DESCRIPTION:

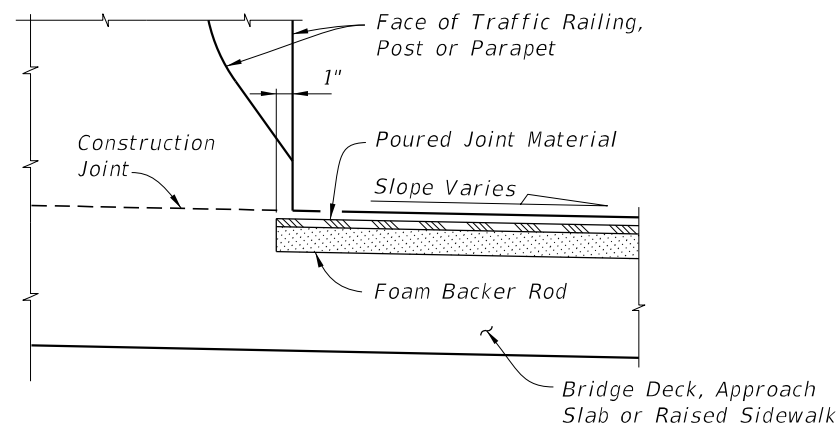


FY 2025-26
STANDARD PLANS

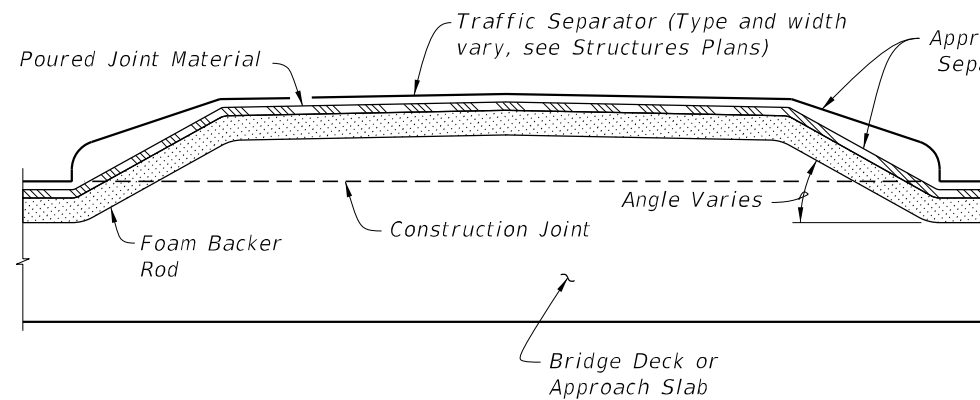
15" FLORIDA SLAB BEAM

INDEX
450-452

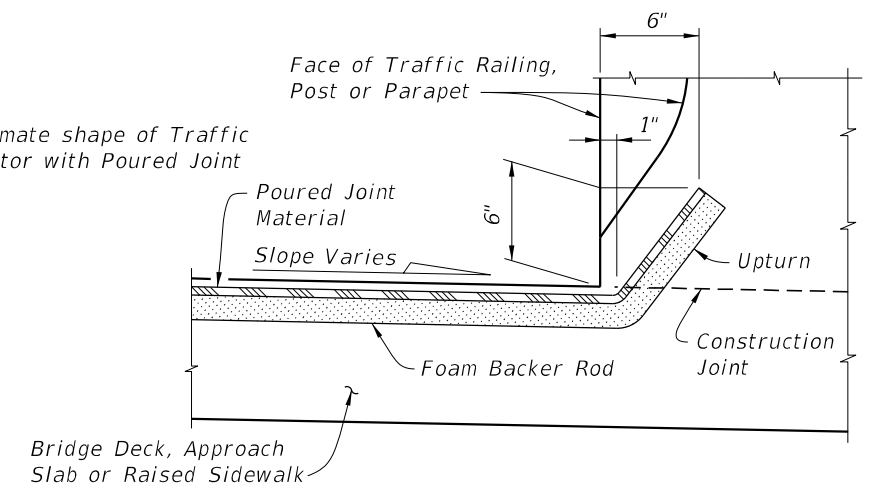
SHEET
1 of 1



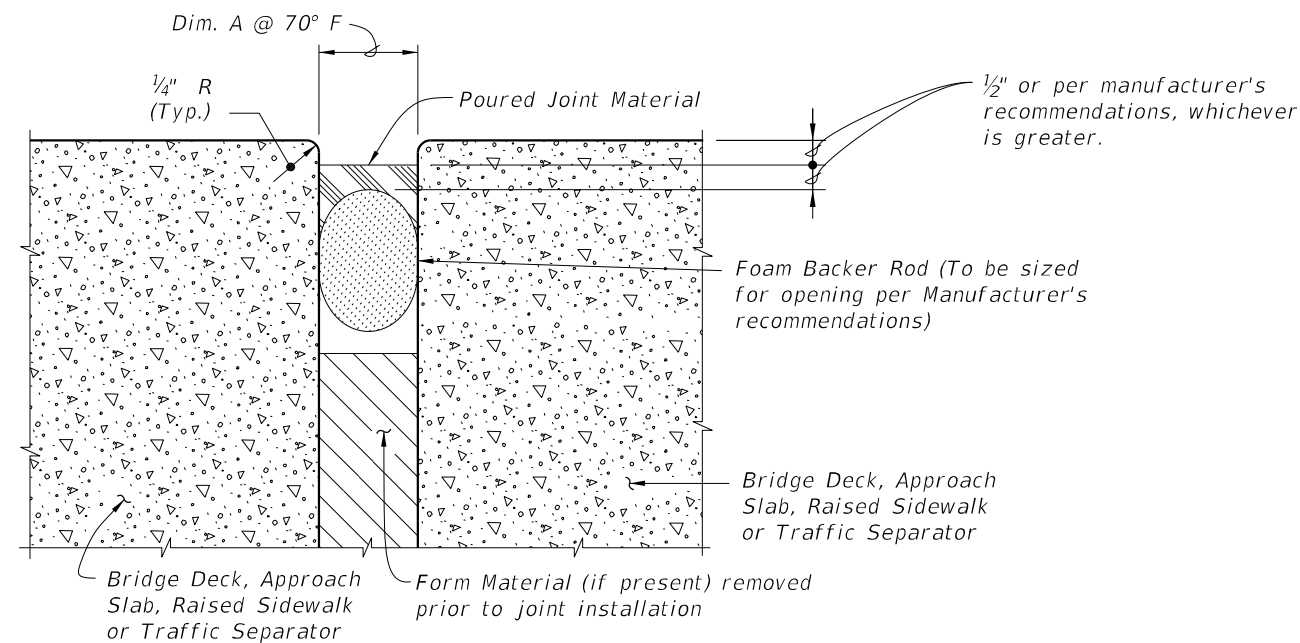
PARTIAL SECTION ALONG Q JOINT
JOINT TREATMENT AT HIGH SIDE OF
DECK WITH SLOPES 1% OR GREATER



PARTIAL SECTION ALONG Q JOINT,
JOINT TREATMENT AT TRAFFIC SEPARATOR



PARTIAL SECTION ALONG Q JOINT
JOINT TREATMENT AT LOW SIDE OF DECK OR
HIGH SIDE OF DECK WITH SLOPES < 1%



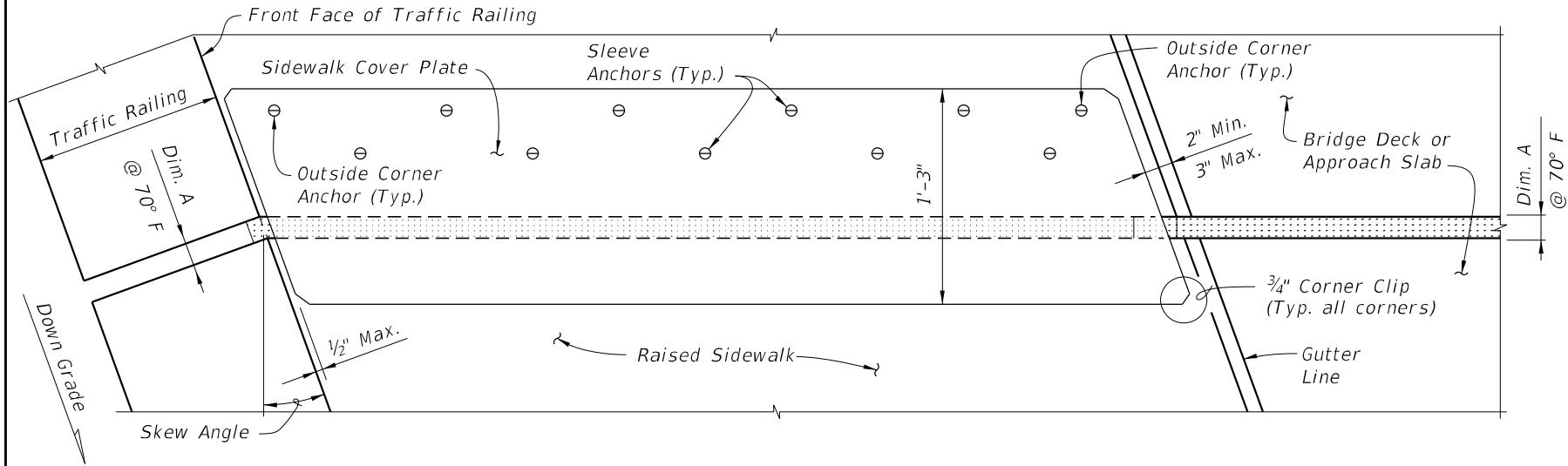
TYPICAL SECTION THRU JOINT

GENERAL NOTES:

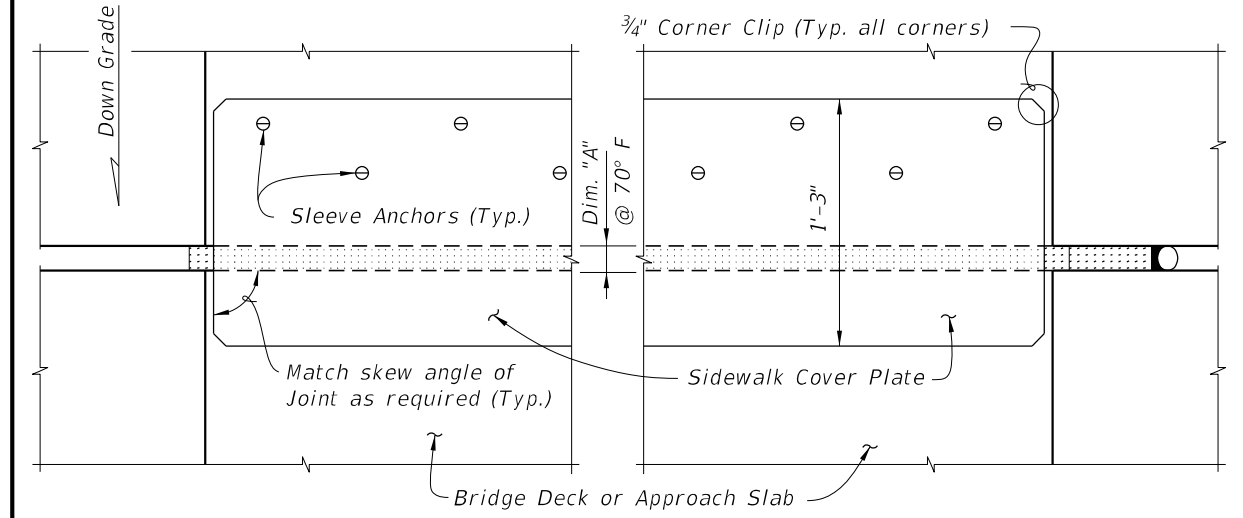
1. Furnish and install Poured Joint With Backer Rod Expansion Joint Systems in accordance with Specification Sections 458 and 932 using Type D silicone sealant material.
2. Refer to the Structures Plans, Poured Expansion Joint Data Table for Dim. A @ 70° F.

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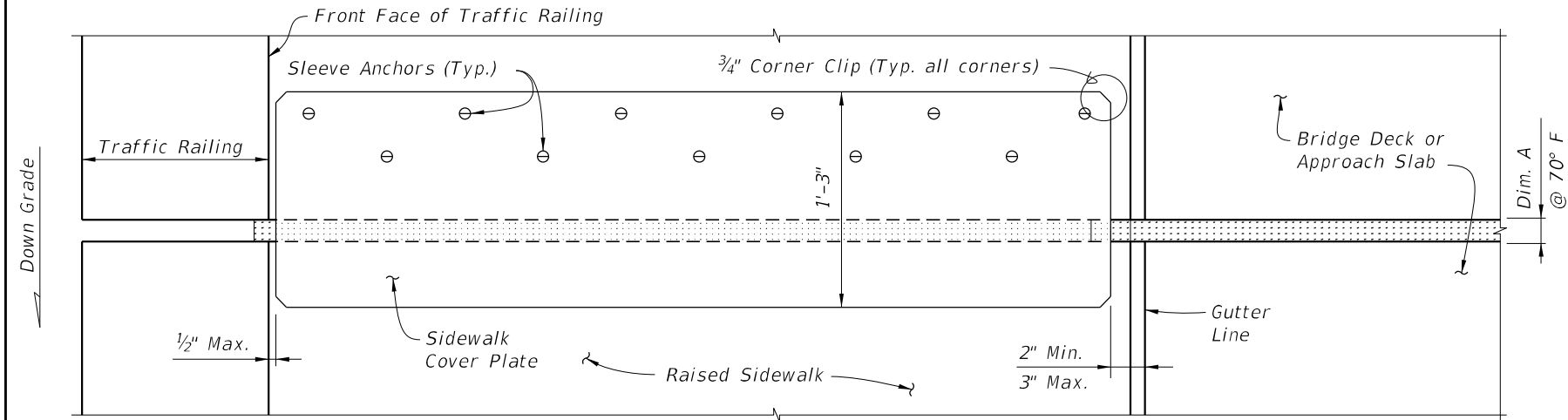
LAST REVISION 11/01/23	REVISION	DESCRIPTION:	FDOT	FY 2025-26 STANDARD PLANS	EXPANSION JOINT SYSTEM - POURED JOINT WITH BACKER ROD	INDEX 458-110	SHEET 1 of 2
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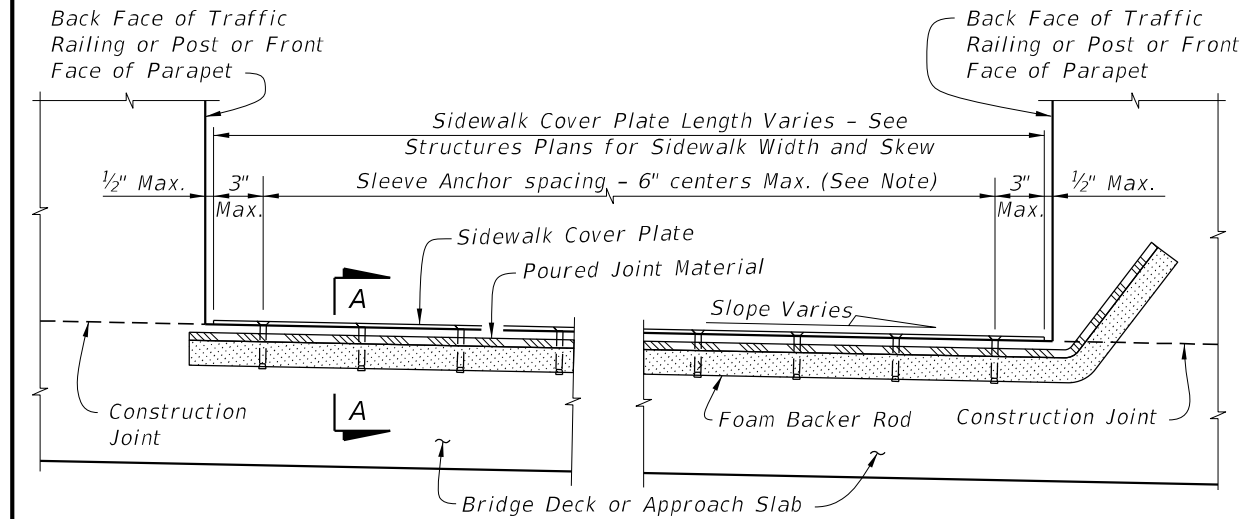
PARTIAL PLAN VIEW OF SKEWED JOINTS



PARTIAL PLAN VIEW

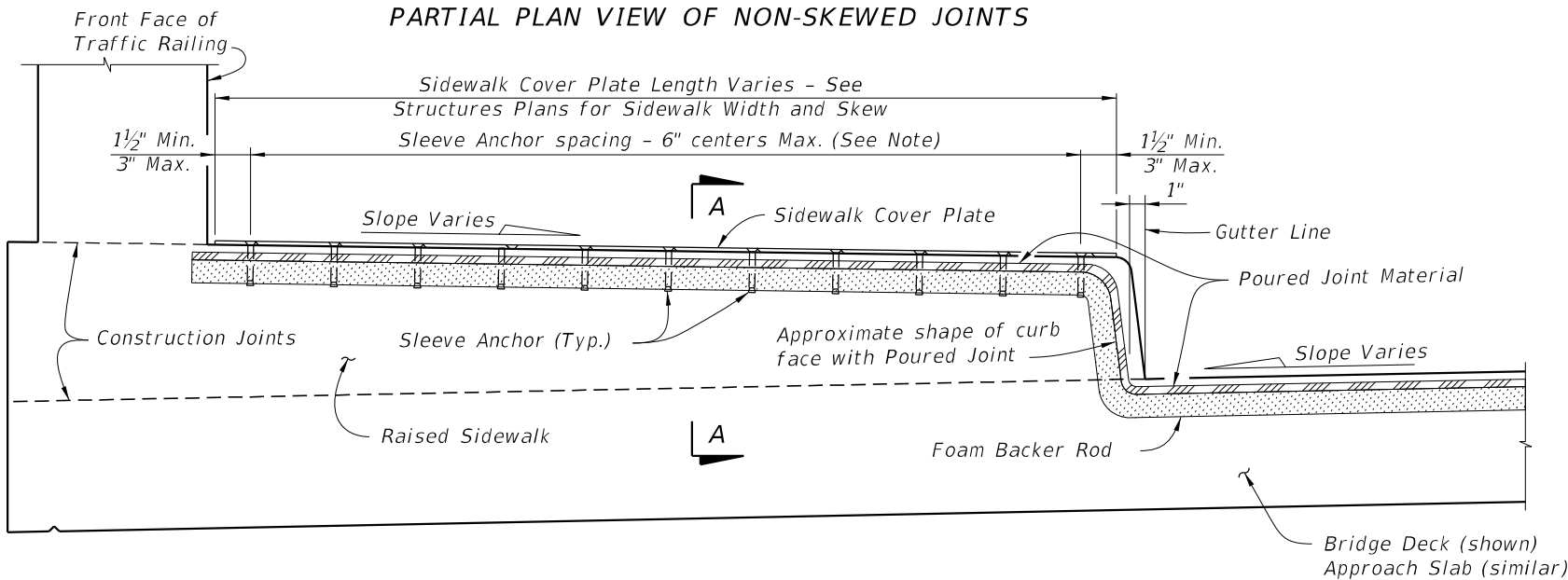


PARTIAL PLAN VIEW OF NON-SKEWED JOINTS



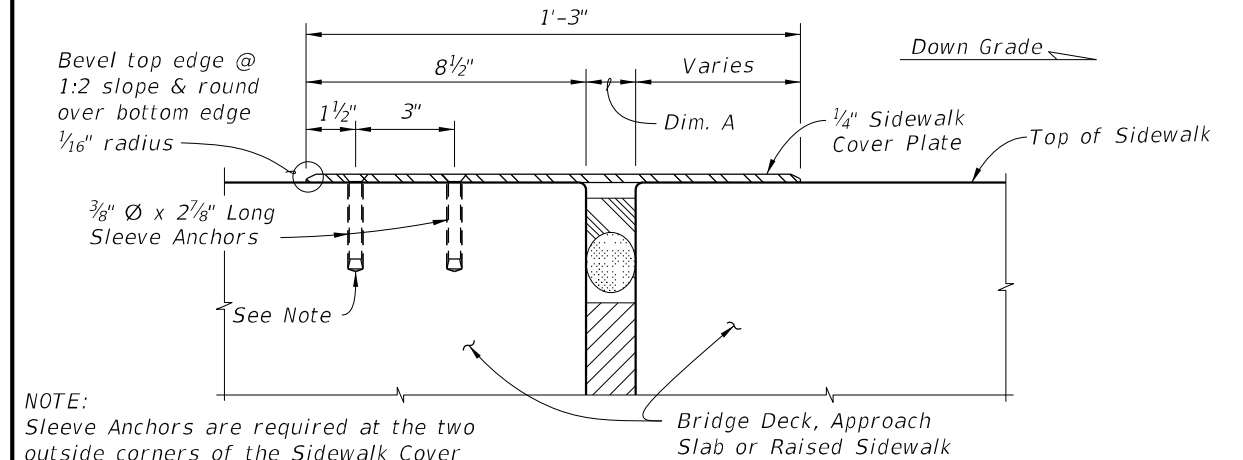
PARTIAL SECTION ALONG Q JOINT

FLUSH SIDEWALK DETAIL



PARTIAL SECTION ALONG Q JOINT


RAISED SIDEWALK DETAIL

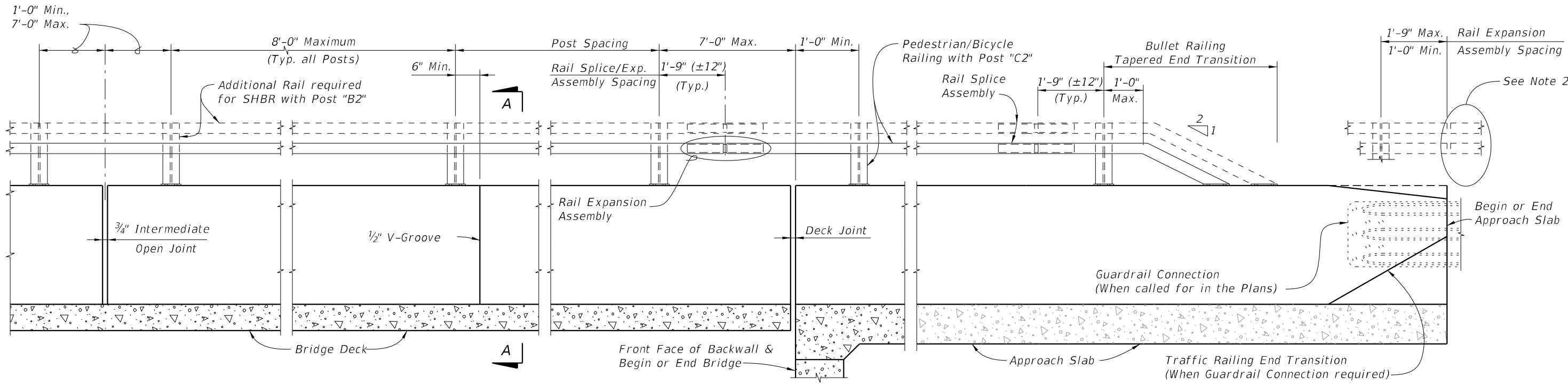


NOTE:
Sleeve Anchors are required at the two outside corners of the Sidewalk Cover Plate. Space Sleeve Anchors uniformly between the corner anchors.

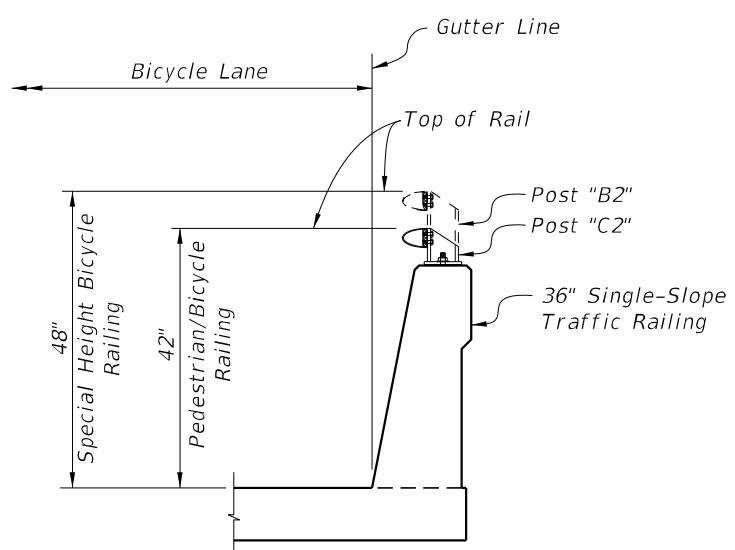
SECTION A-A

10/17/2024 10:11:06 AM

LAST REVISION 07/01/13	REVISION	DESCRIPTION:	 FY 2025-26 STANDARD PLANS	EXPANSION JOINT SYSTEM - POURED JOINT WITH BACKER ROD	INDEX 458-110	SHEET 2 of 2
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ELEVATION OF INSIDE FACE OF TRAFFIC RAILING WITH PEDESTRIAN/BICYCLE BULLET RAILING



SECTION A-A
TYPICAL SECTION THRU BRIDGE DECK
(APPROACH SLAB SIMILAR)


NOTES:

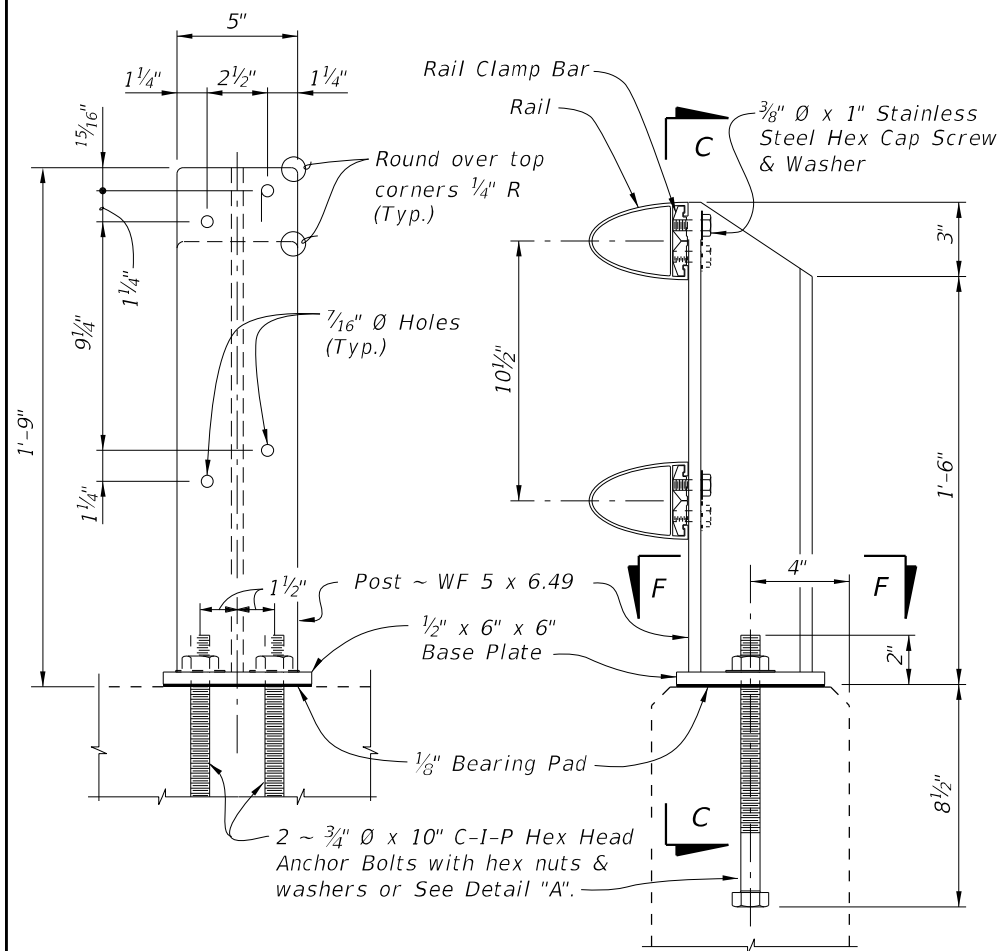
1. A Bullet Railing Tapered-End Transition is required for all approach ends of Bullet Railings on Traffic Railings. When Guardrail Connection is required terminate the Bullet Railing Tapered-End Transition at beginning of the Traffic Railing End Transition.
2. Where Bullet Railing continues on retaining wall mounted Traffic Railings or Barriers, provide a Bullet Railing Tapered End Transition at the terminus of the Bullet Railing.

CROSS REFERENCES:

Work in conjunction with Index 515-022.
For Traffic Railing Details, Reinforcement and Notes see Index 521-427.

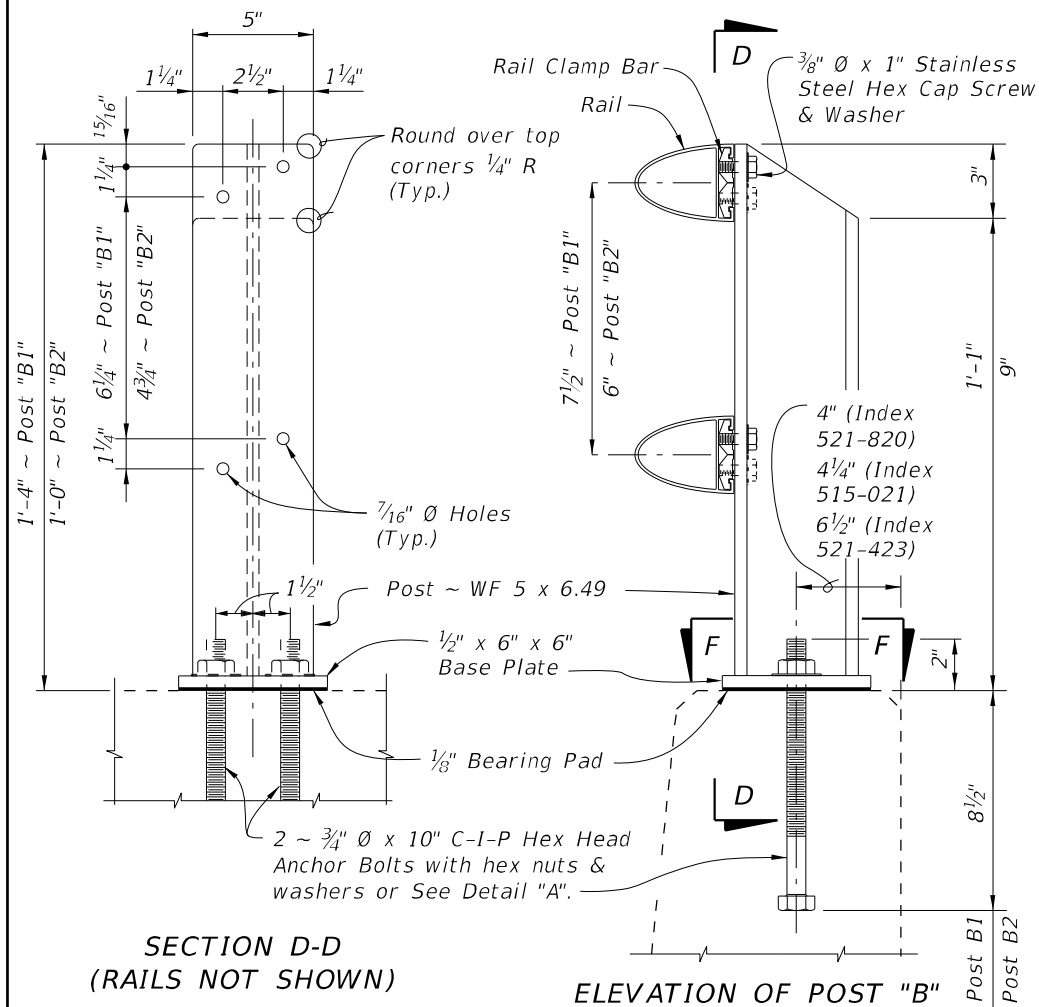
10/17/2024 10:23:20 AM

LAST REVISION 11/01/17	REVISION DESCRIPTION:	 <div>FY 2025-26 STANDARD PLANS</div>	<div>PEDESTRIAN/BICYCLE BULLET RAILING FOR TRAFFIC RAILING</div>	INDEX 515-021	SHEET 1 of 1
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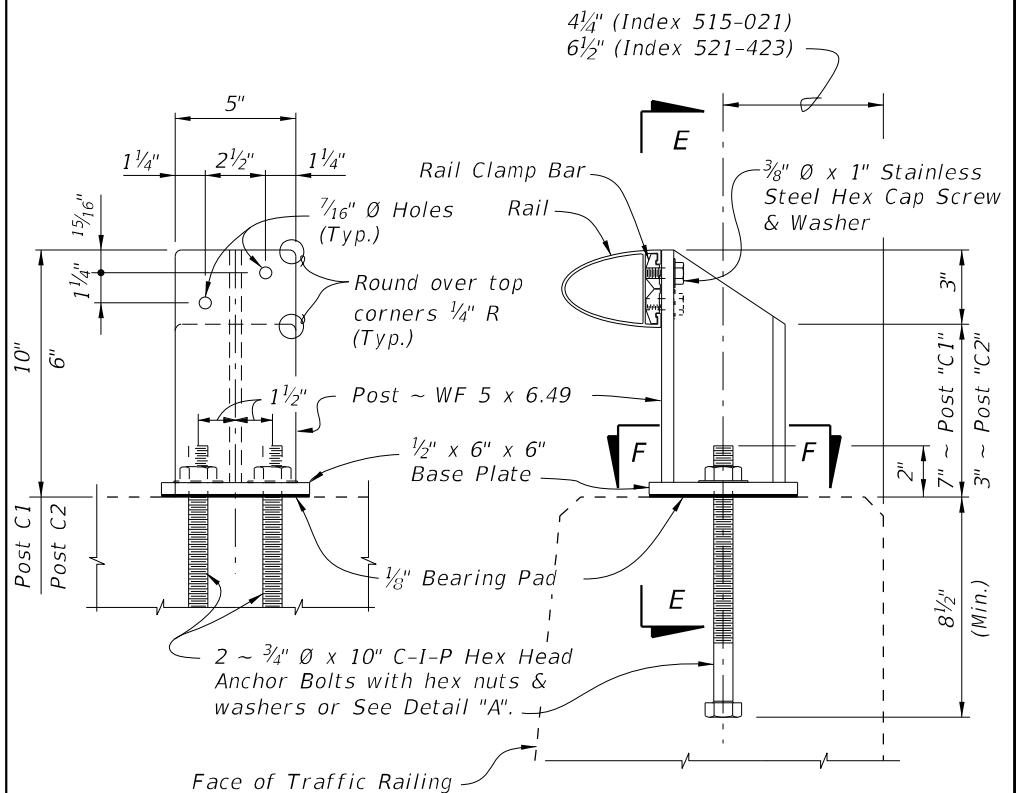
SECTION C-C
(RAILS NOT SHOWN)

POST "D" DETAILS FOR SPECIAL HEIGHT BICYCLE RAILING
(SHBR) ON CONCRETE PARAPET (INDEX 521-820)



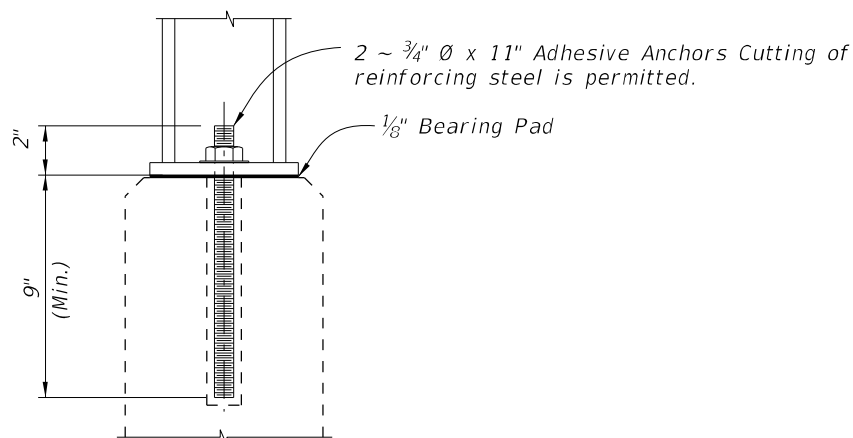
SECTION D-D
(RAILS NOT SHOWN)

POST "B1" DETAILS FOR SHBR ON TRAFFIC RAILING
(INDEX 521-423) AND FOR PEDESTRIAN/BICYCLE
RAILING (PBR) ON CONCRETE PARAPETS (INDEX 521-820)
POST "B2" DETAILS FOR SHBR ON TRAFFIC RAILING
(INDEX 521-427 AND 515-021)

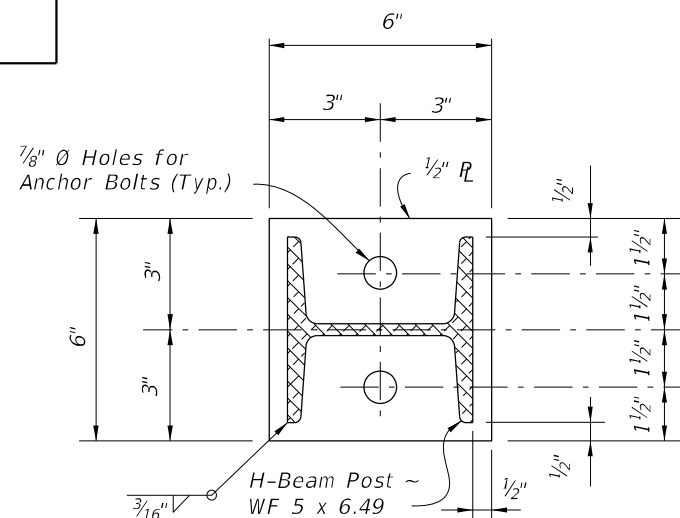


SECTION E-E
(RAIL NOT SHOWN)

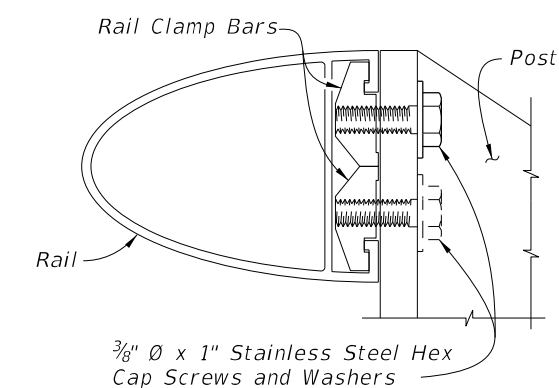
POST "C1" DETAILS FOR PEDESTRIAN/BICYCLE RAILING (PBR)
ON TRAFFIC RAILINGS (INDEX 521-423)
POST "C2" DETAILS FOR PBR ON
TRAFFIC RAILING (INDEX 521-427 & 515-021)



DETAIL "A"
ALTERNATE ANCHOR BOLT
(Concrete Parapet Shown,
Traffic Railings Similar)



SECTION F-F
BASE PLATE DETAIL



RAIL TO POST CONNECTION DETAIL

CROSS REFERENCES:

For post spacing on Concrete Parapets
see Index 521-820.

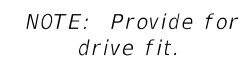
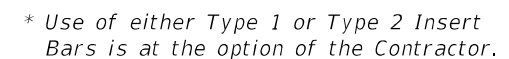
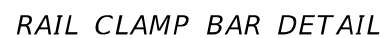
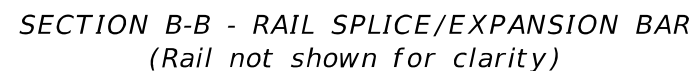
For post spacing on Traffic Railings
see Index 515-021.

For Rail Details see Sheet 2.

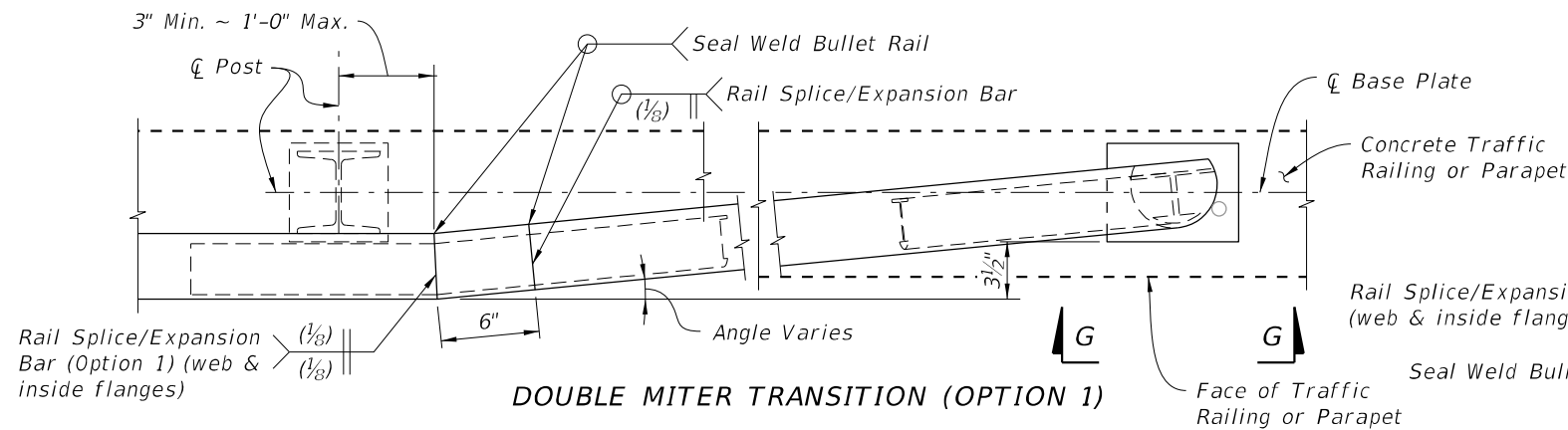
For Railing Notes and Tapered End
Transition Details see Sheet 3.

10/17/2024 10:23:41 AM

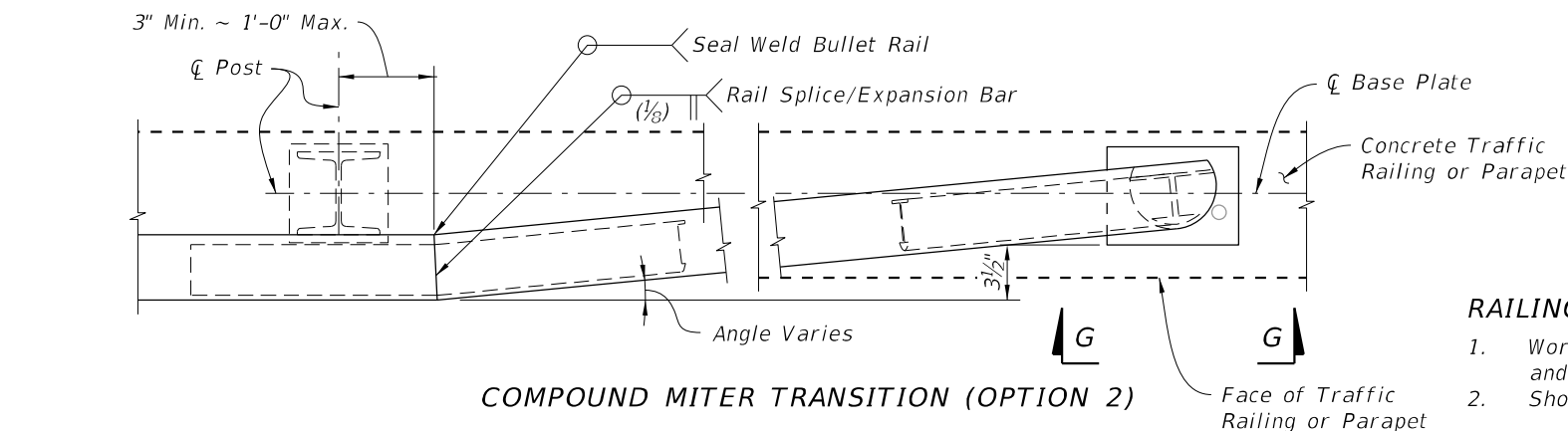
LAST REVISION	DESCRIPTION:	FDOT	FY 2025-26 STANDARD PLANS	PEDESTRIAN/BICYCLE BULLET RAILING DETAILS	INDEX 515-022	SHEET 1 of 3
11/01/17						



CROSS REFERENCE:
For Notes and Tapered End Transition Details,
See Sheet 3.

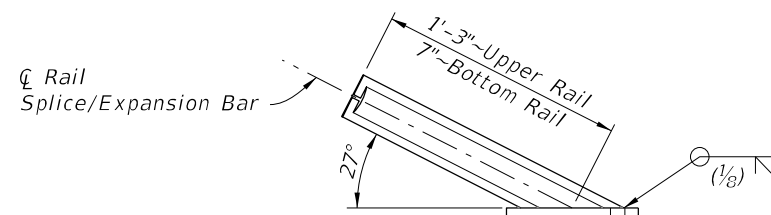


DOUBLE MITER TRANSITION (OPTION 1)

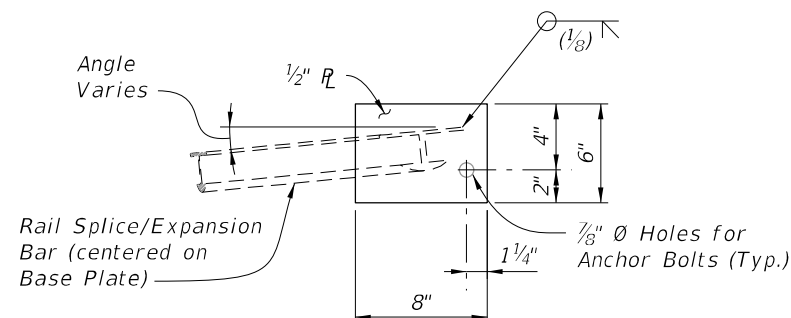


COMPOUND MITER TRANSITION (OPTION 2)

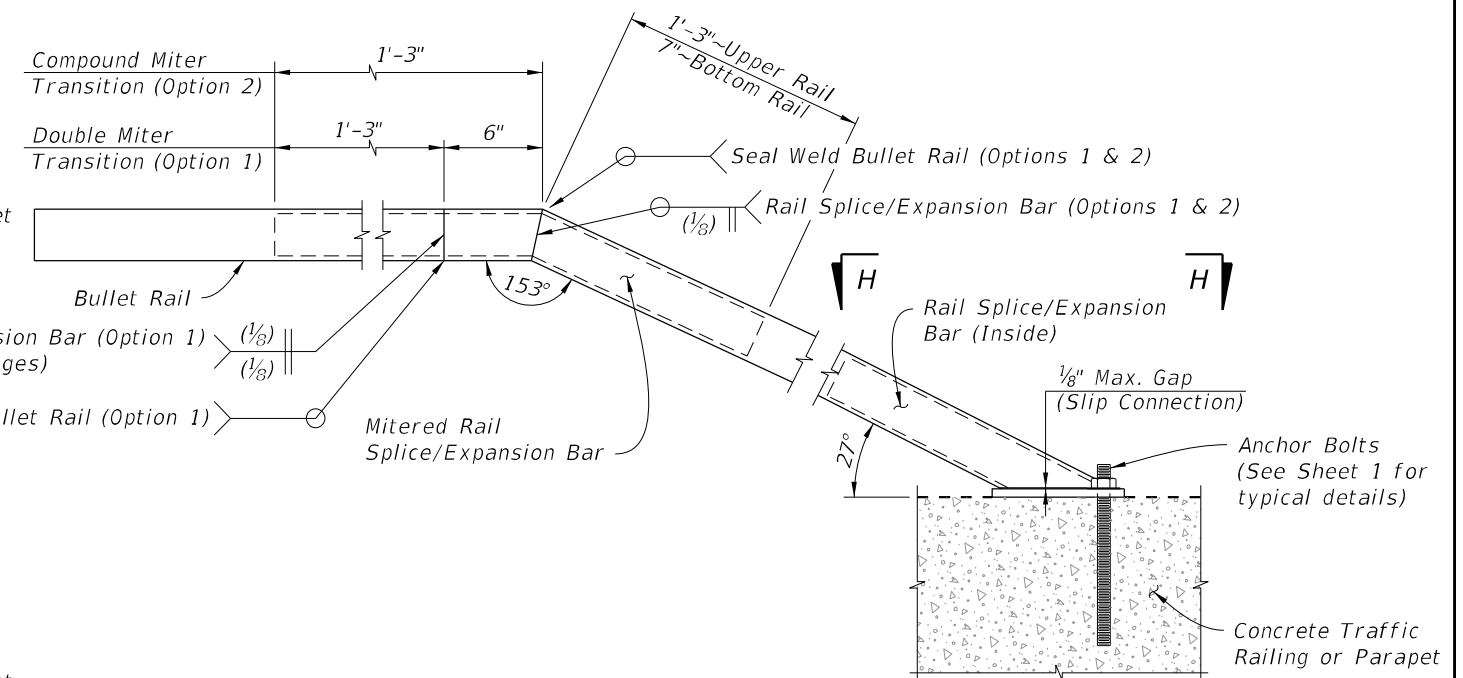
PARTIAL PLAN OF TAPERED END TRANSITIONS
(Single Rail Shown, Double or Triple Rail Similar)



VIEW G-G TRANSITION BASE PLATE
(Bullet Rail not shown for Clarity)



VIEW H-H TRANSITION BASE PLATE
(Bullet Rail not shown for Clarity)



ELEVATION OF TAPERED END TRANSITION
(Single Rail Shown, Double or Triple Rail Similar)

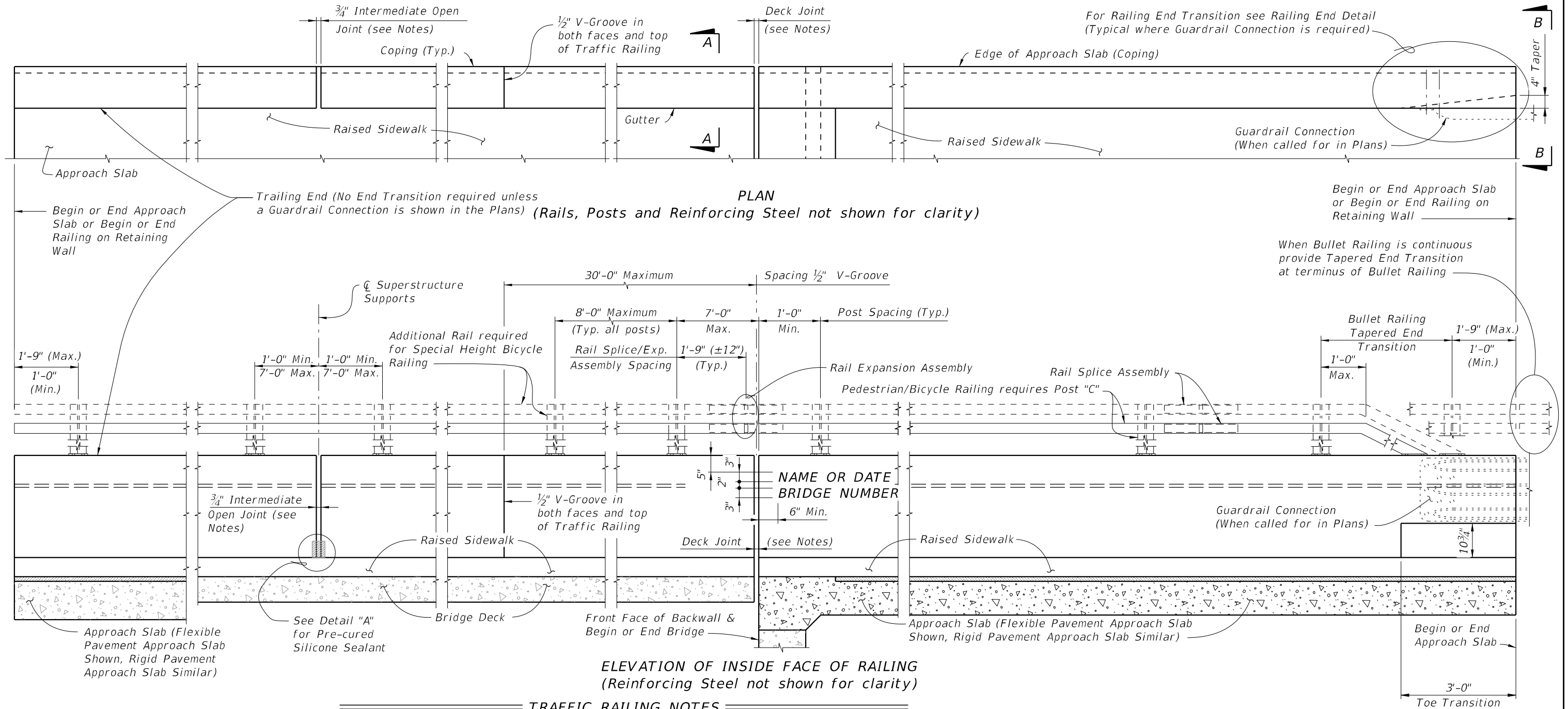
RAILING NOTES:

- Work this Index with Index 521-423, 521-427, 521-428, 521-820 and 515-021 and Specification Section 515.
- Shop Drawings: Submit shop drawings prior to fabrication.
 - Include post and rail splice/expansion assembly location for curved alignments with radii < 40 feet and for all end terminations.
- Materials:
 - Supply Aluminum materials in accordance with Specification Section 965 and the following:
Wrought Aluminum Post: ASTM B221, Alloy 6061-T6 or 6351-T5
Rail End Cap: ASTM B26 sand cast aluminum alloy 356.0-F
Plate and Bars: ASTM B209 Alloy 6061-T6
Rails: ASTM B221 Alloy 6061-T6 or 6351-T5.
Stop Pins: Press-fit aluminum or stainless steel pins or tubes
 - Stainless Steel Fasteners: ASTM F-593, Alloy Group 2 (316).
 - Bearing Pads: Plain or Fiber Reinforced meeting Specification Section 932 for Ancillary Structures.
 - Anchor Bolts: Galvanized ASTM A307 Grade 36 Hex Head. Galvanized ASTM 1554 Grade 55 Threaded rods for Adhesive Anchors.
- Layout:
 - Posts shall be uniformly spaced with reasonable consistency.
 - Tapered End Transitions are required at the terminus of the approach ends of Bullet Railing mounted on a Traffic Railing. Bullet Railings on concrete parapets shielded by a traffic railing do not require Tapered End Transitions unless noted otherwise in the Plans.
 - Adjust post spacing's to avoid parapet obstacles, such as armor expansion plates, by 9 inches minimum.
 - Rails shall be continuous over a minimum of 3 posts, except that lengths less than 12 feet need only be continuous over 2 posts.
 - Space splices at 40 feet maximum. Splice all rails in a given railing section at about the same center line.
 - Provide rail expansion assemblies in panels between posts on either side of a bridge expansion joint. Rail expansion assemblies are similar to the rail splice assemblies with increased space at the expansion assembly to allow for movement equal to 1.5 times the bridge joint opening or 1" greater than the expected joint movement.
- Installation:
 - Set rails near bridge expansion joints to allow for expected movement.
 - Cutting of reinforcing steel is permitted for post installed anchors.
- Payment: Includes the full cost of installed bullet railing. Cost of the Concrete Parapet or Traffic Railing is separate.

10/17/2024 10:23:55 AM

LAST REVISION	DESCRIPTION:	FDOT	FY 2025-26 STANDARD PLANS	PEDESTRIAN/BICYCLE BULLET RAILING DETAILS	INDEX	SHEET
11/01/22					515-022	3 of 3

10/17/2024 10:29:08 AM



LAST REVISION	DESCRIPTION:	FDOT	FY 2025-26 STANDARD PLANS	TRAFFIC RAILING - (32" VERTICAL SHAPE)	INDEX	SHEET
11/01/20					521-423	1 of 3

CONVENTIONAL REINFORCING STEEL BENDING DIAGRAMS						
BILL OF REINFORCING STEEL			ROADWAY CROSS-SLOPE	ØA		
MARK	SIZE	LENGTH		LOW GUTTER	HIGH GUTTER	
S	5	As Req'd.	0% to 2%	90°	90°	
T	5	9'-0"	2% to 6%	87°	93°	
X	5	5'-10"	6% to 10%	84°	96°	

STIRRUP BAR 5T

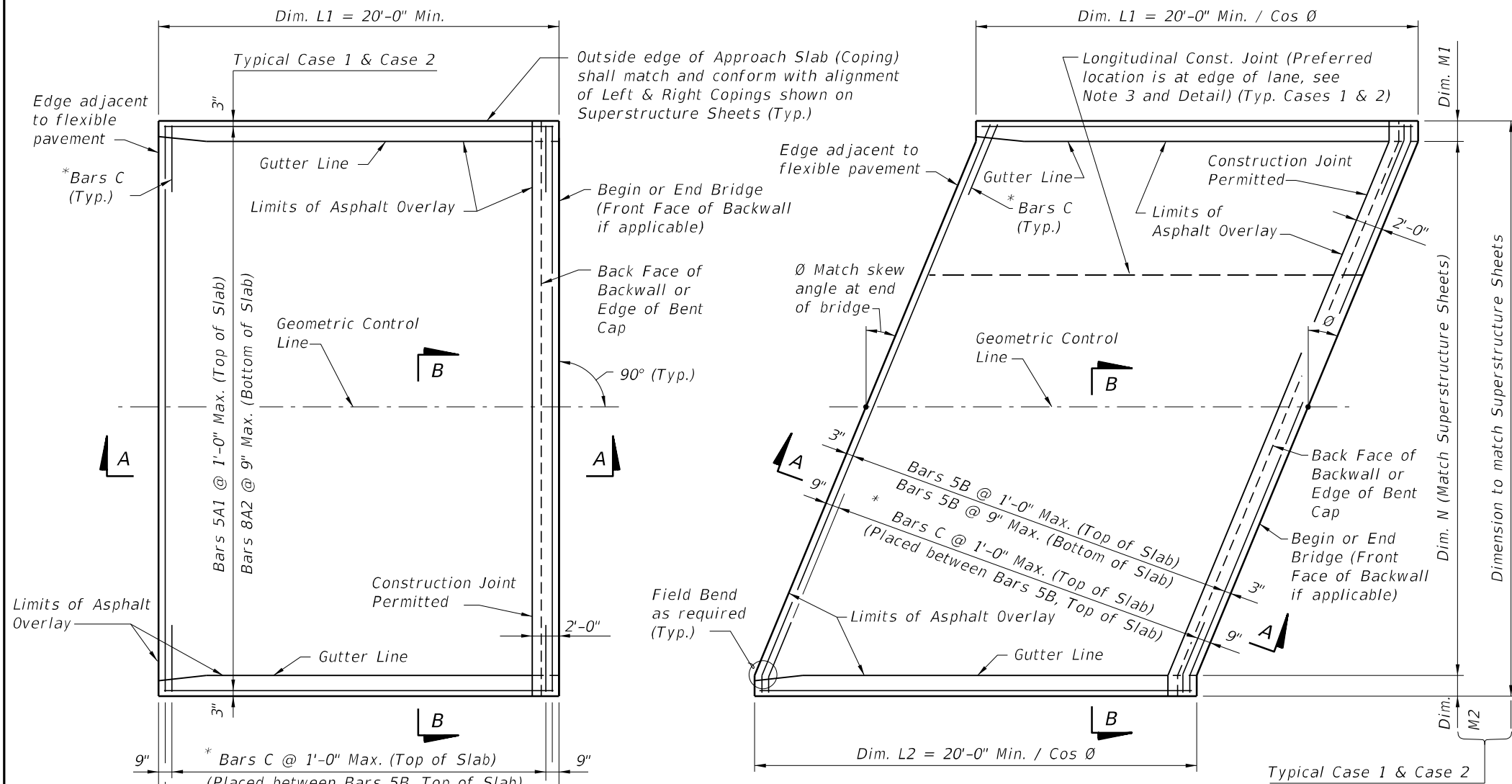
STIRRUP BAR 5X

BAR 5S

REINFORCING STEEL NOTES:

- All bar dimensions in the bending diagrams are out to out.
- The 3'-8³/₄" vertical dimensions shown for Bars 5T and 5X are based on a bridge deck with a 6" thick x 6' wide raised sidewalk at low side of deck, 2% deck cross slope and a counter 2% raised sidewalk cross slope. If the raised sidewalk thickness, width or cross slopes vary from the above amounts, adjust these vertical dimensions accordingly to achieve a 6" minimum embedment into the bridge deck.
- The reinforcement for the railing on a Retaining Wall shall be the same as detailed with ØA = 90°.
- All reinforcing steel at the open joints shall have a 2" minimum cover.
- Bars 5S may be continuous or spliced at the construction joints. Bar splices for Bars 5S shall be a minimum of 2'-2".
- The Contractor may utilize Welded Wire Reinforcement (WWR) when approved by the Engineer. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.

10/7/2016 9:48:31 AM



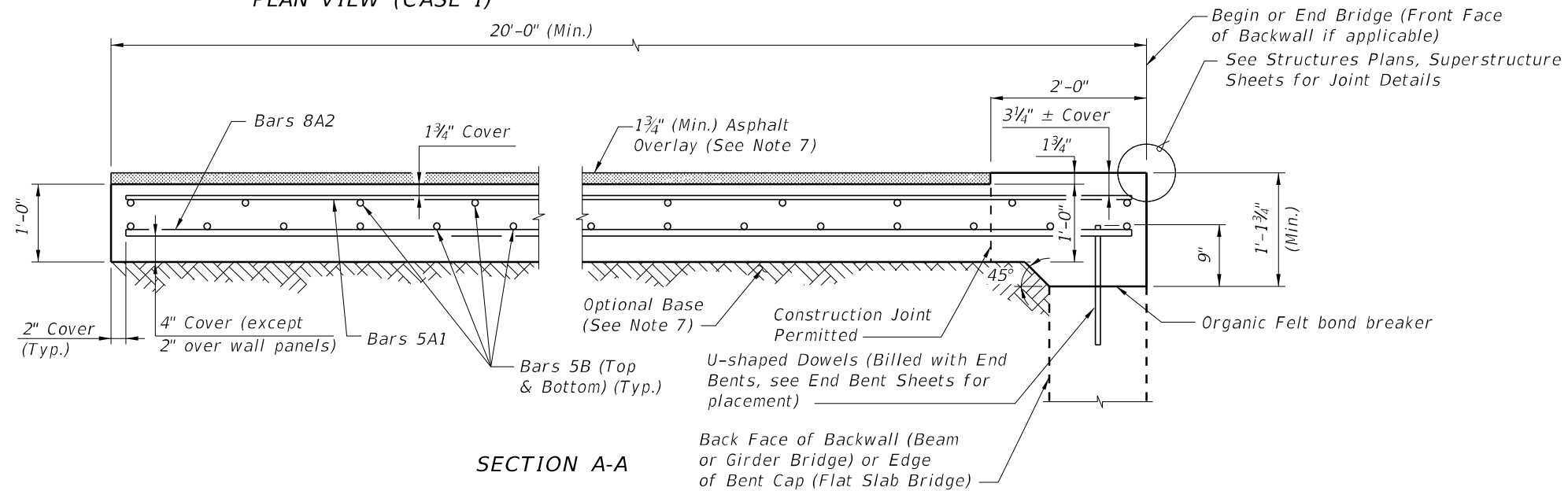
* NOTE: Bars C are required as shown when either a Traffic Railing or the Traffic Railing/Noise Wall are used at the edge of the Approach Slab.

GENERAL NOTES

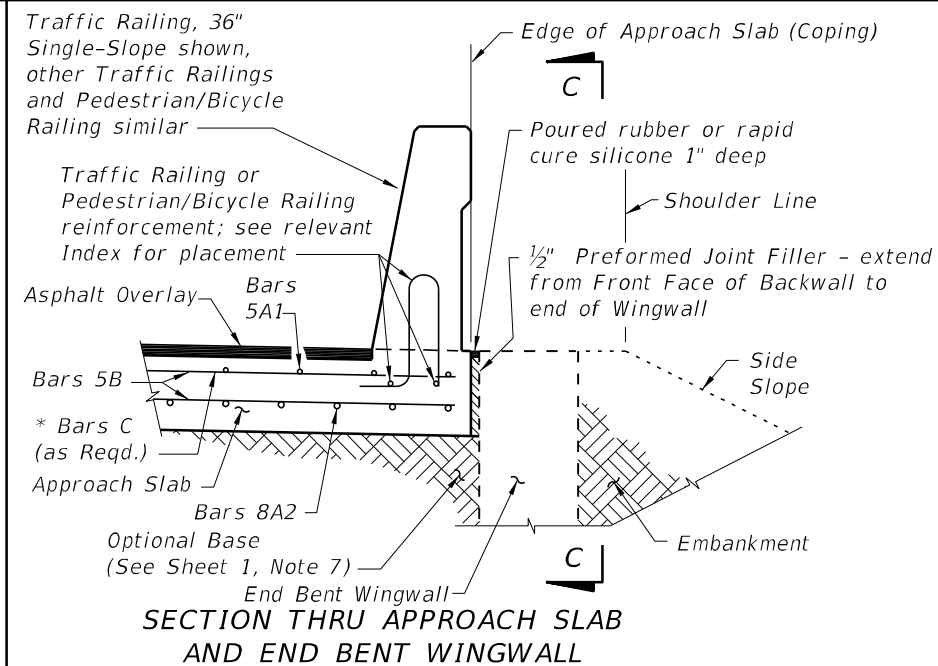
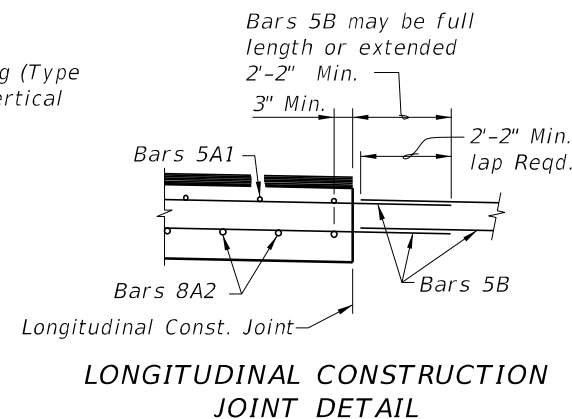
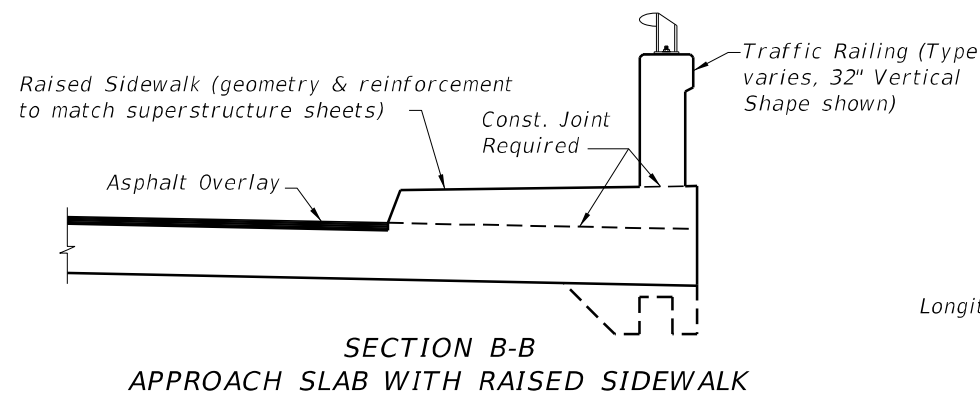
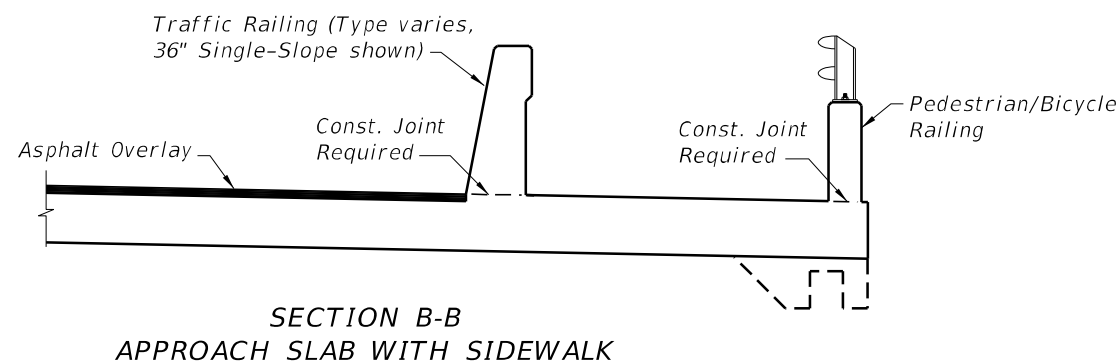
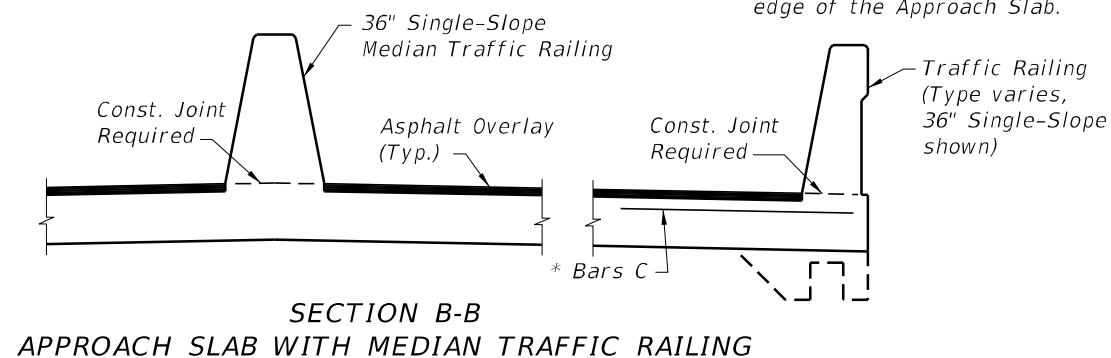
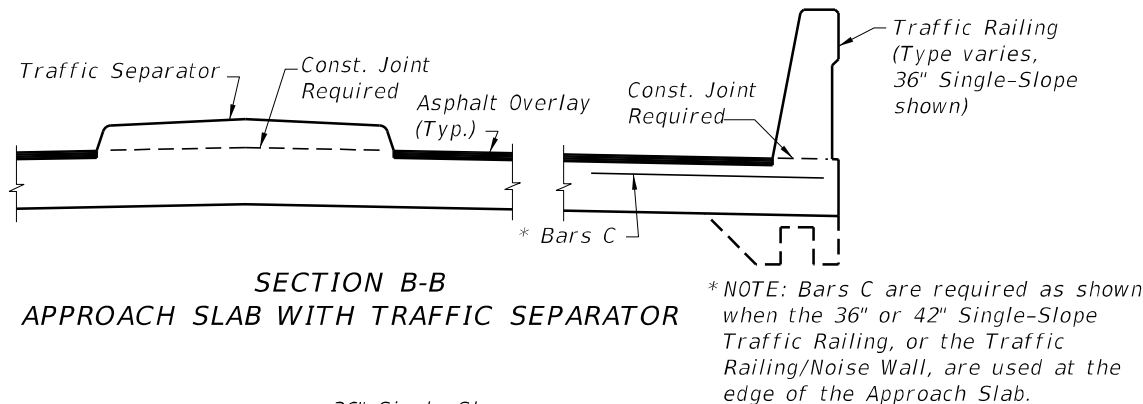
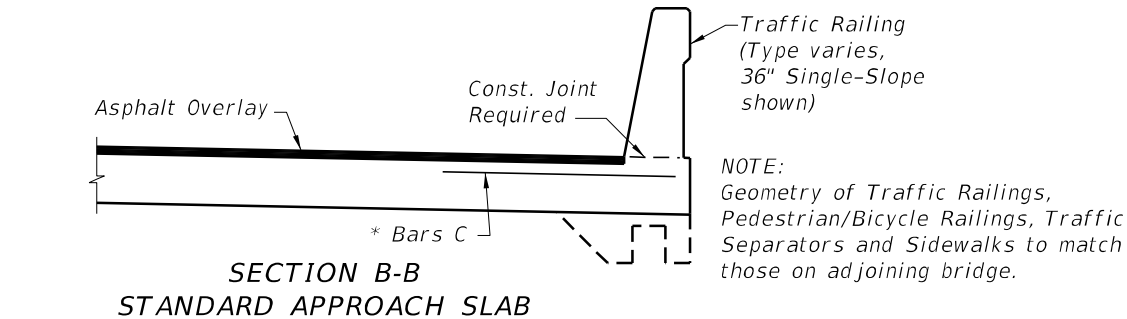
1. **SURFACE TREATMENT:** As an option to Class 4 Floor Finish (Bridge Floor Grooving) per Section 400 a hand tined or heavy broomed finish may be permitted on the concrete portion of the riding surface. Sidewalk areas shall receive a broomed finish. The top surface of the concrete beneath the asphalt overlay shall be raked.
2. **CONDUIT:** If required, see Structure Plans for Conduit Details.
3. When a longitudinal construction joint is necessary or allowed by the Engineer, the transverse steel shall be extended as shown in the Longitudinal Construction Joint Detail.
4. The plan view for CASE 1 applies when the skew angle (θ) = 0°.
5. The plan view for CASE 2 applies where the skew angle (θ) is > 0°. The slab shown represents a skew to the right for an approach slab at begin bridge; approach slab at the end of bridge or a left skew shall be treated similarly.
6. Welded Wire Reinforcement (WWR) for the edge of Approach Slabs on retaining walls is not included in the estimated quantity for reinforcing steel and is considered incidental to the work. WWR must consist of Deformed wire meeting the requirements of Specification Section 931.
7. Continue the asphalt pavement over the approach slab and match the friction course type used on the roadway. See the Roadway Plans for asphalt overlay and optional base details.
8. Approach slabs shown in Plan View Cases 1 and 2 represent a typical approach slab with edge barriers and no sidewalks. Provide railings, parapets and raised sidewalks as detailed in the Contract Plans.

CROSS REFERENCES:

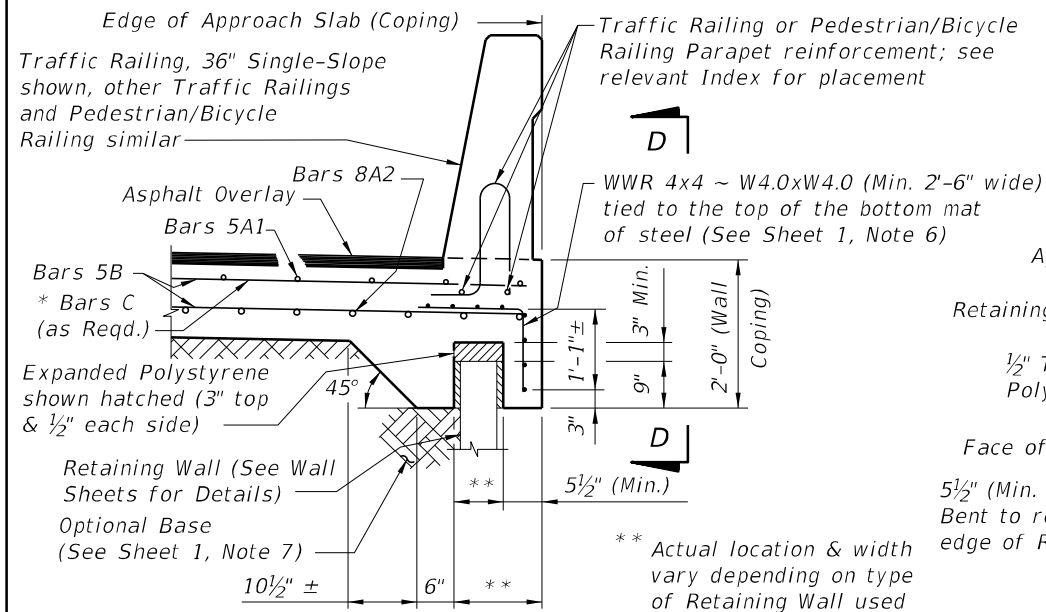
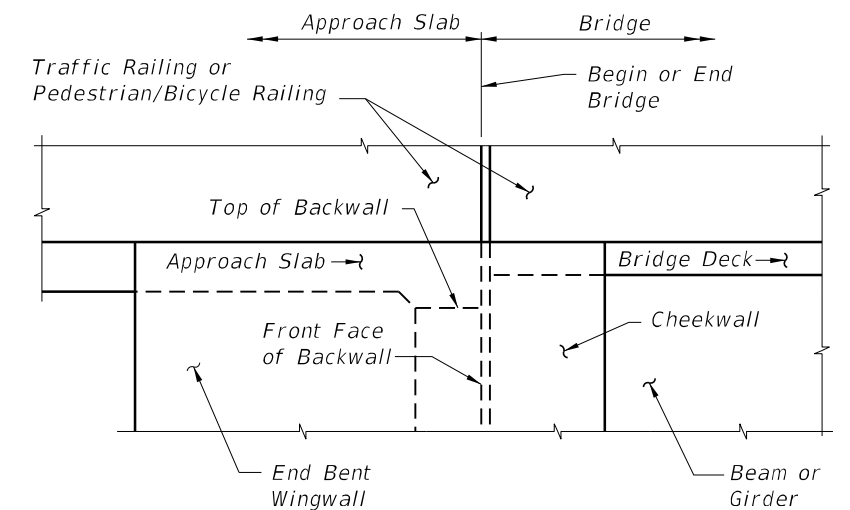
For Section B-B, Longitudinal Construction Joint Detail and Approach Slab Details see Sheet 2.



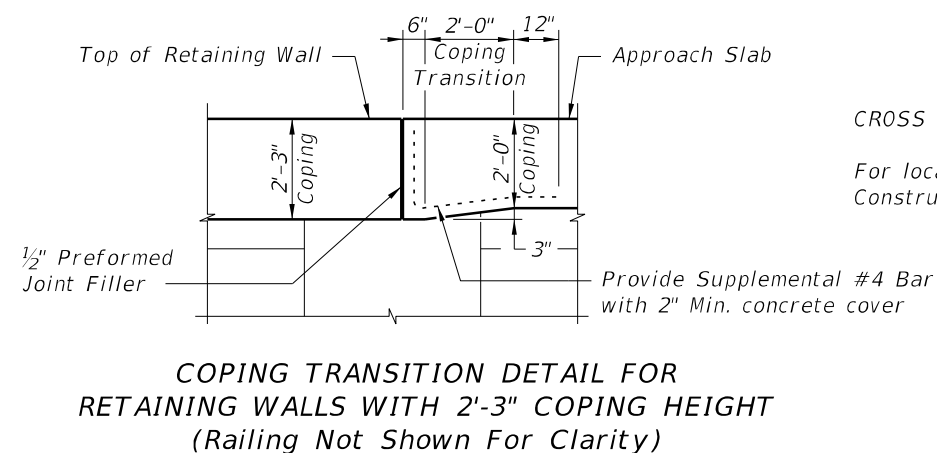
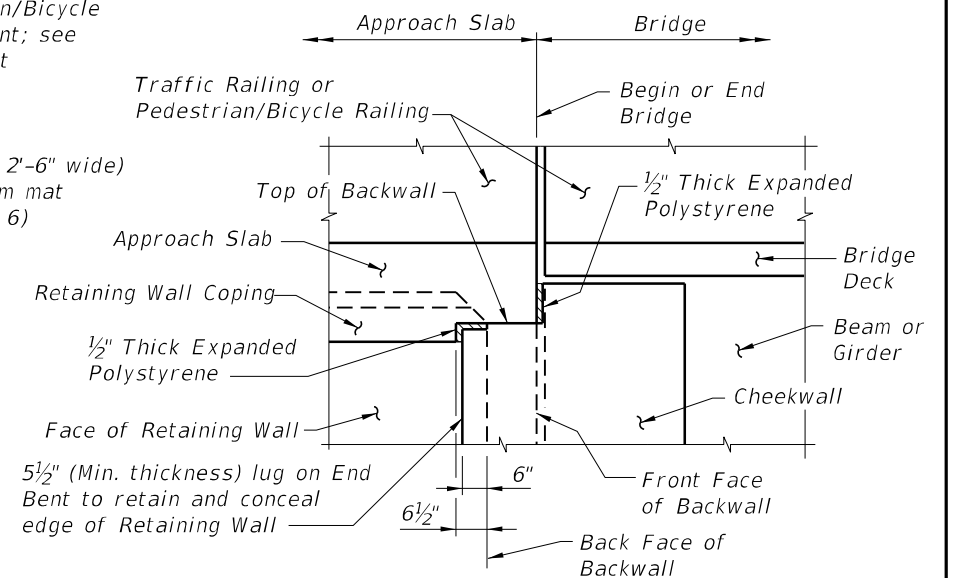
LAST REVISION		DESCRIPTION:	FDOT DEVELOPMENTAL DESIGN STANDARDS	APPROACH SLABS (20 FT.) (FLEXIBLE PAVEMENT APPROACHES)		INDEX NO. D20920	SHEET NO. 1 of 2
10/01/16	REVISION						



APPROACH SLAB WITH WINGWALL DETAILS



APPROACH SLAB WITH RETAINING WALL DETAILS



CROSS REFERENCES:
For location of Section B-B and Longitudinal Construction Joint see Sheet 1.

LAST REVISION 10/01/16	DESCRIPTION:	FDOT DEVELOPMENTAL DESIGN STANDARDS	APPROACH SLABS (20 FT.) (FLEXIBLE PAVEMENT APPROACHES)	INDEX NO. D20920	SHEET NO. 2 of 2
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