

MIAMI-DADE COUNTY

MIAMI-DADE PUBLIC LIBRARY SYSTEM



ADDENDUM 9

Miami Lakes Library Branch

EXTERIOR IMPROVEMENTS AND COMPREHENSIVE INTERIOR RENOVATIONS

C23-MDPLS-02-ESP

2025

ADDENDUM NO. 9

14-JUL-2025

PROJECT: **Miami Lakes Library Branch
Exterior Improvements and Comprehensive
Interior Renovations
6699 Windmill Gate Rd.,
Miami, Florida 33014**

BID DUE DATE: **6-AUG-2025 - Wednesday**

FROM: Miami-Dade Public Library System (MDPLS)
Capital Program Division 101 West Flager
Miami, FL 33130

TO: Prospective Bidders and Interested Parties

This Addendum forms part of the project solicitation documents and will be incorporated into the Contract Documents, as applicable. Insofar as the Original Contract Documents, Drawings and Specifications are inconsistent, this Addendum shall govern. Please acknowledge receipt of this Addendum, at the time of bid submittal to Miami-Dade County, in the space provided on the "Acknowledgement of Addenda Form" provided with the project solicitation documents. Failure to acknowledge receipt of all addenda may be cause for disqualification.

Miami-Dade County's "Cone of Silence", Section 2-11.1(t) of the Code of Miami-Dade County, approved by the Board of County Commissioners, specifically prohibits communication in regard to this bid solicitation with County staff except as allowed by the Code. The period covered by the "Cone of Silence" is defined in the Code.

Bidders must file a copy of any written communication with the Clerk of the Board, which shall be available to any person upon request. Miami-Dade Public Library System (MDPLS) shall respond in writing and file a copy with the Clerk of the Board, which shall be made available to any person upon request. Written communications for questions, Request for Information (RFI) and addendums may also be in the form of e-mail addressed to Malka Rodriguez at CGA@MDPLS.ORG with copy to the Clerk of the Board at clerk.board@miamidade.gov.

ADDENDUM 9

Please be reminded that access to back of house and restricted areas are not allowed after the mandatory site visit held on 11-JUN-2025. Contractors can visit the Branch without impacting normal operations, requesting access to restricted areas, or damaging the building or any areas.

Q184. Plan Sheet S001 containing general structural notes is missing the Structural Steel Notes and the Wood Trusses Notes, which seem important in this project. Please provide missing structural Notes.

A184. Please review in the enclosures the new added Sheet S002 with added notes.

Q185. For example, regarding the structural steel scope, are the exterior steel columns and beams for the trellises to be Hot Dipped Galvanized? How about the steel beams above the corner windows and the columns? How about the steel columns at the entry doors? Please clarify.

A185. All exposed steel element needs to be Galvanized.

Q186. Plan Sheet D101 does not show the demolition of the roof truss areas above the entry foyer and the existing multipurpose rooms. Please add notes on structural roof demolition areas required.

A186. Please review revised drawing D102

Q187. Plan Sheet S101 includes Note on the Ground Floor Slab calling for such slab to be 4" thick and to reinforced with rebars #4 @ 12" O.C. Please confirm the reinforcement for the concrete floor slab.

A187. Confirmed. This is a slab on grade.

Q188. Plan Sheet S101 does not a floor slab depression at the area of the new restrooms for the Children's Department. If such restrooms are to be ceramic tile and require a slope to the floor drains, the slab requires a 2" depression area. Please clarify if restrooms are tile floors and if depression is required.

Q188. GC to coordinate in the field to ensure a maximum elevation change in floor finish from the restroom to the adjacent room. If a depression is needed, drop the slab on grade 2" and remain the slab thickness of 4".

Q189. Plan Sheet S102 for Roof trusses are missing a large area of Piggy-Back trusses required above the existing roof areas at the north addition area. See Plan A3.0 showing the correct shaded area for the new trusses. Please add missing truss area to structural roof plan.

A189. Please review revised design drawing S102

Q190. Plan Sheet S301, S302 and S303 all seems to indicate a “Waterproofing membrane” below the concrete floor slab. Is this Note referring to the usual 6 mill vapor retarder visqueen layer mentioned in Note #4 of plan S101? Or is the intent to provide an actual “waterproofing foundation membrane” on top of the base rock layer? If an actual “waterproofing membrane” is required provide manufacturer and products to be used on top of the rock layer. Please clarify.

A190. *Please use visqueen layer. No waterproof membrane needed.*

Q191. Plan Sheet S302 showing sections 4/S302 and 6/S302 do not show the steel beam required at the storefront on the corner window seats. Please add steel beams or notes regarding the structural steel columns and beams at the window seats.

A191. *The steel beams and columns are noted on Sheet S102. The steel beams above have been added to the sections on S302 and S303. All other required elements shall be included in the storefront shop drawings. GC to provide engineered shop drawings for review and approval.*

Q192. Drawings S301-A5.3, we are requesting clarification regarding the waterproofing membrane shown under the building’s concrete slab foundation. The architectural and structural drawings reference the membrane in multiple sections, but do not specify:

- The type of waterproofing system required
- Whether it should be a sheet-applied or fluid applied membrane
- The thickness, manufacturer, or performance criteria.

A192. *Please review revised drawings S301-visqueen layer is requested not waterproof membrane.*

ENCLOSURES – Drawings:

1. S001 – dated 1-JUL-2025 – Rev 02
2. S002 – dated 1-JUL-2025 – Rev 02
3. S102 – dated 1-JUL-2025 – Rev 02
4. S301 – dated 1-JUL-2025 - Rev 02
5. S302 – dated 1-JUL-2025 – Rev 02
6. S303 - dated 1-JUL-2025 – Rev 02
7. D102 – dated 1-JUL-2025 – Rev 02

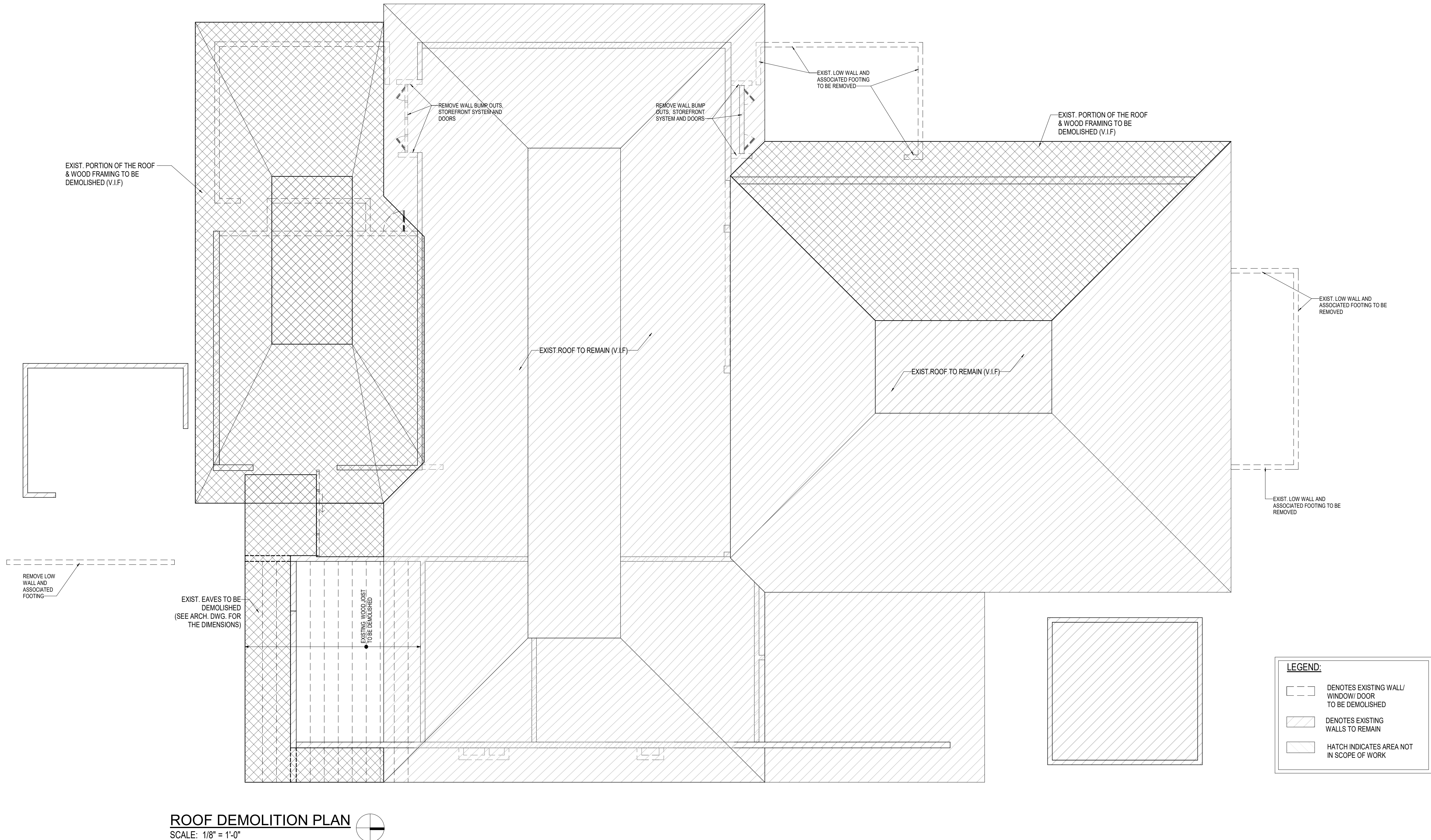
REMINDER:

Solicitation remains under the Cone of Silence. Please request information via email to: Cga@mdpls.org and copy the Clerk of the Board at clerkbcc@miamidade.gov.

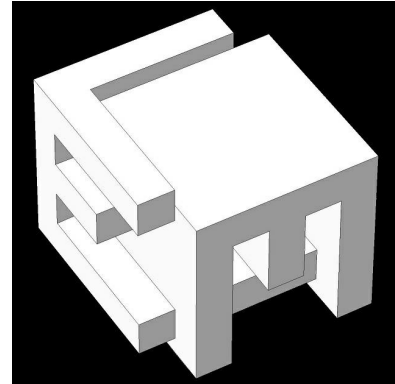
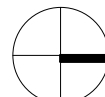
END OF ADDENDUM NO. 9

Cc: Clerk of the Board (clerkbcc@miamidade.gov)
Lisa Thompson
Erik Myers

CONTRACT NO: C23-MDPLS-02-ESP
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ROOF DEMOLITION PLAN
SCALE: 1/8" = 1'-0"



ELM ARCH

8950 SW 74TH Court
Suite 1204
Miami, FL 33156
786-391-2646

Qualifier: Erik Lloyd Myers
State of Florida: AR 93574

Sign & Seal: Erik Lloyd Myers
State of Florida: AR 93574

OWNER:

Miami Lakes Branch Library

6699 Windmill Gate Road
Miami Lakes, FL 33014

TITLE: ROOF LEVEL DEMOLITION PLAN

| REVISION | | |
|----------|-------------|----------|
| 2 | REVISION 02 | 07-01-25 |

DATE
04-03-2023
PROJECT NUMBER
19119

SHEET NUMBER
D102
OF 2

YHCE
YOUSSEF HACHEM
CONSULTING ENGINEERING, INC.
CA 28553
99 NW 27 AVE
MIAMI, FL 33125
TEL: (786) 287-9120
FAX: (305) 969-9453
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YOUSSEF HACHEM, Ph.D., P.E.
No. 43302

STRUCTURAL NOTES

GENERAL NOTES

ALL WORK SHALL BE IN CONFORMANCE WITH STRUCTURAL DRAWINGS, SPECIFICATIONS AND THE REQUIREMENTS OF THE 2020 FLORIDA BUILDING CODE (THE BUILDING CODE REFERENCED IN THE FOLLOWING NOTES).

1.-TO THE BEST OF OUR KNOWLEDGE, THE STRUCTURAL DRAWINGS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE GOVERNING BUILDING CODE.

2.-CONSTRUCTION IS TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE NOTED ABOVE AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL CODES, STANDARDS, REGULATIONS, AND LAWS

3.-THE CONTRACTOR SHALL COMPARE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS BEFORE COMMENCING WITH THE WORK AND SHALL NOTIFY THE ARCHITECT AND ENGINEER OF ANY DISCREPANCIES REQUIRING CLARIFICATION OR REVISIONS. DO NOT SCALE STRUCTURAL DRAWINGS, REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS NOT SHOWN. SEE "DIMENSION" SECTION OF GENERAL NOTES FOR ADDITIONAL NOTES.

4.-THE CONTRACTOR SHALL USE STRUCTURAL DRAWINGS IN CONJUNCTION WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS TO COORDINATE LOCATION OF DEPRESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, REGLETS, BOLT SETTINGS, SLEEVES, DIMENSIONS, ETC. (DRAWINGS ARE NOT TO BE SCALED).

5.-DISCREPANCIES BETWEEN INFORMATION PRESENTED WITHIN PROJECT SPECIFICATIONS AND WITHIN STRUCTURAL NOTES ON PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BY THE CONTRACTOR PRIOR TO PRESENTING HIS OR HER BID. IF SUCH A DISCREPANCY IS DISCOVERED SUBSEQUENT TO BIDDING, THE CONTRACTOR SHALL BE RESPONSIBLE FOR IMPLEMENTING THE OPTION SUBSEQUENTLY SELECTED BY THE ENGINEER AT NO ADDITIONAL COST.

6.-CONTRACTORS SHALL BE RESPONSIBLE FOR FINAL VERIFICATION OF ALL DIMENSIONS, ELEVATIONS, CLEARANCES, ETC. OF THE FRAMING SHOWN ON THE STRUCTURAL DRAWINGS AGAINST INFORMATION PROVIDED BY MANUFACTURER OF SELECTED MECHANICAL EQUIPMENT PRIOR TO PROCEEDING WITH ANY RELATED PORTION OF WORK. ITEMS REQUIRING SUCH REVIEW SHALL INCLUDE ELEVATORS (ELEVATOR PITS, BEAMS ABOVE ELEVATORS DOORS, ETC.), ESCALATORS, DUCTS, COOLING TOWERS, ETC. CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ANY REMEDIAL WORK AND FOR ITS IMPACT ON THE WORK SCHEDULE RESULTING FROM FAILURE TO PROVIDE EARLY NOTIFICATION OF SUCH CONFLICTS TO THE DESIGN TEAM.

7.-POTENTIAL CONFLICTS, ERRORS OR OMISSIONS PRESENT WITHIN THE DRAWINGS (WHETHER WITHIN STRUCTURAL DRAWINGS OR BETWEEN STRUCTURAL, ARCHITECTURAL, AND M.E.P. DRAWINGS) SHALL BE IDENTIFIED BY THE CONTRACTOR DURING HIS/HER EARLY REVIEW OF THE PROJECT DOCUMENTS. SUCH CONFLICTS, ERRORS OR OMISSIONS SHALL BE COMMUNICATED TO THE ARCHITECT IN WRITING PRIOR TO COMMENCEMENT OF WORK. IN THE EVENT OF FAILURE TO PROVIDE SUCH A NOTICE AND SUFFICIENT TIME FOR A RESPONSE, THE CONTRACTOR SHALL BECOME RESPONSIBLE FOR COST OF ALL WORK OR REMEDIAL WORK RESULTING FROM SUCH CONFLICTS, ERRORS OR OMISSION, AS WELL AS FOR ITS IMPACT ON THE PROJECT SCHEDULE.

8.-ALL COSTS OF INVESTIGATION AND/OR REDESIGN, DUE TO CONTRACTOR MISLOCATION OR STRUCTURAL ELEMENTS OR OTHER LACK OF CONFORMANCE WITH THE PROJECT DOCUMENTS, SHALL BE AT THE CONTRACTOR'S EXPENSE.

9.-IN THE EVENT THAT CERTAIN DETAILS OF THE CONSTRUCTION ARE NOT FULLY SHOWN OR NOTED ON THE DRAWINGS, THEIR CONSTRUCTION SHALL BE OF THE SAME TYPE AS FOR SIMILAR CONDITIONS WHICH ARE SHOWN AND NOTED, SUBJECT TO THE STRUCTURAL ENGINEER'S APPROVAL. DETAILS LABELED "TYPICAL" APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED, WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION. QUESTIONS REGARDING THE APPLICABILITY OF TYPICAL DETAILS SHALL BE RESOLVED BY THE PROJECT ARCHITECT.

10.-SEE THE ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:

- 10.1-SIZE AND LOCATIONS OF ALL CONCRETE CURBS, FLOOR DRAINS, SLOPES, INSERTS, ETC. EXCEPT AS SHOWN.
- 10.2-SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS EXCEPT AS SHOWN.
- 10.3-SIZE AND LOCATION OF ROOF AND FLOOR OPENINGS, FLOOR AND ROOD FINISHES, TYPES OF WATER PROOFING AND DAMP PROOFING.
- 10.4-FINISHED FLOOR AND EXTERIOR ELEVATIONS.
- 10.5-DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.
- 10.6-FIRE PROTECTION REQUIREMENTS.
- 10.7-MISC. STEEL TUBES, CHANNELS, ANGLES, AND PLATES FOR METAL PANEL WALL AND CURTAIN WALL SUPPORT.

10.8-EMBEDS FOR WISC METAL FRAMING AND CLADDING ANCHORAGE.

10.9-SIZE AND LOCATIONS OF MASONRY, DRYWALL, NON-LOAD BEARING PARTITIONS AND EXTERIOR WALL. PROVIDE SLIP CONNECTIONS THAT ALLOW VERTICAL MOVEMENT AT THE HEADS OF ALL SUCH PARTITIONS, CONNECTIONS SHALL BE DESIGNED TO SUPPORT THE TOP OF THE WALLS Laterally FOR THE CODE-REQUIRED LATERAL LOAD.

11.-SEE THE MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR THE FOLLOWING:

- 11.1-PIPE AND DUCT RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., EXCEPT AS SHOWN OR NOTED.
- 11.2-ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.
- 11.3-CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES.
- 11.4-ANCHOR BOLTS FOR MOTOR MOUNTS, EXCEPT AS SHOWN OR NOTED.
- 11.5-SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES (HOUSEKEEPING PADS). NOTE THAT HOUSEKEEPING PADS SHOWN ON THE STRUCTURAL DRAWINGS ARE APPROXIMATE AND ARE INCLUDED FOR GENERAL REFERENCE ONLY.

12.-OPENINGS, POCKETS, ETC., LARGER THAN 6" SHALL NOT BE PLACED IN CONCRETE SLABS, DECKS, OR WALLS UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS. NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW OPENINGS, POCKETS, ETC., LARGER THAN 6" WHICH ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS, BUT WHICH ARE LOCATED IN STRUCTURAL MEMBERS.

13.-ALL SUSPENDED MECHANICAL, ELECTRICAL, OR OTHER SYSTEM LOADS EXCEEDING 100 POUNDS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS. ANY REINFORCEMENT, ETC. REQUIRED BY SUCH LOADS SHALL BE BY THE TRADE REQUIRING THE EQUIPMENT.

14.-YHCE, CONSULTING ENGINEERS, INC. SHALL NEITHER HAVE CONTROL OVER OR CHARGE OF, NOR BE RESPONSIBLE FOR, THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, SINCE THESE ARE SOLELY THE CONTRACTOR'S RIGHTS AND RESPONSIBILITIES UNDER THE CONTRACT DOCUMENTS HAS SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S OR ANY SUBCONTRACTOR'S FAILURE TO PERFORM THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL PROTECT ADJACENT PROPERTY, HIS OWN WORK, AND THE PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR JOBSITE SAFETY INCLUDING ALL SHAN REQUIREMENTS.

15.-THE STRUCTURE WAS DESIGNED TO BE SELF-SUPPORTING AND STABLE FOLLOWING INSTALLATION OF ALL COMPONENTS AS INDICATED ON THE DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE METHOD AND SEQUENCE OF ERECTION PROCEDURES (INCLUDING IMPLEMENTATION OF TEMPORARY SHORING, BRACING, ETC.) AND TO ENSURE SAFETY THROUGH THE PERIOD OF CONSTRUCTION. CONTRACTOR AGREES THAT HE WILL HOLD OWNERS, ARCHITECT, ENGINEER, AND/OR ANY OF THEIR EMPLOYEES OR AGENTS, HARMLESS FROM ANY AND ALL DAMAGE AND CLAIMS WHICH MAY ARISE BY A REASON OF ANY NEGLIGENCE ON THE PART OF THE CONTRACTOR, OR ANY OF HIS SUBCONTRACTORS, OR ANY MATERIAL AND EQUIPMENT SUPPLIERS, AND/OR ANY OF THEIR EMPLOYEES OR AGENTS, IN THE PERFORMANCE OF THIS CONTRACT. IN CASE ANY ACTION IS BROUGHT AGAINST THE OWNER, OR ARCHITECT, OR ENGINEER, OR ANY OF THEIR EMPLOYEES OR AGENTS, CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR DEFENSE THEREOF, TO THE FULL SATISFACTION OF THE LATTER PARTY.

16.-DO NOT PLACE OR APPLY CONCRETE WITHOUT STRUCTURAL SHOP DRAWINGS MECHANICAL/ARCHITECTURAL SHOP DRAWINGS RELATED TO THE CONCRETE WORK. RELATED ITEMS INCLUDE LOCATIONS OF OPENINGS, PIPE SLEEVES, REGLETS, DOVETAIL SLOTS, DRIPS, INSERTS FOR MECHANICAL EQUIPMENTS, HUNG CEILINGS, AND ANY OTHER ITEMS REQUIRED TO BE INSTALLED AND/OR TO BE COORDINATED BY THE ARCHITECTURAL/MECHANICAL TRADES.

17.-CONTRACTOR IS TO PROVIDE DURING CONSTRUCTION AND MAKE ALLOWANCE FOR DESIGN, DETAILING, AND PURCHASE, DURING BID PHASE FOR ALL MISCELLANEOUS STEEL REQUIRED FOR THE SUPPORT OF ARCHITECTURAL FEATURES THAT ARE NOT STRUCTURAL ITEMS TO THE BASE STRUCTURE. SUCH ITEMS INCLUDE MECH HANGINGS, CEILING, AND CURTAIN WALL SUPPORTS.

18.-SUPPLEMENT SKETCHES/DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO FORWARD A COPY OF ALL CORRESPONDENCE AFFECTING THE STRUCTURE TO THE PROJECT'S INSPECTOR THROUGHOUT THE DURATION OF CONSTRUCTION

CONCRETE:

A. ALL CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 318-14 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."

B. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE AS FOLLOWS:

| | |
|--------------------|--------------|
| STRUCTURAL BEAMS | 5000 PSI MIN |
| COLUMNS | 5000 PSI MIN |
| ALL OTHER CONCRETE | 3000 PSI |

C. FORMWORK SHALL COMPLY WITH ACI 347R-14 "RECOMMENDED PRACTICE FOR CONCRETE WORK."

D. MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF ANY CONCRETE WORK. SUBMIT STATISTICAL DATA FOR EACH CLASS OF CONCRETE.

E. NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE.

F. THE OWNER SHALL CONTRACT AN INDEPENDENT TESTING LABORATORY TO PER- FORM CONCRETE CYLINDER TESTS AS FOLLOWS: SIX CYLINDER TESTS FOR ANY 50 CUBIC YARDS OF CONCRETE POURED, OR FRACTION THEREOF FOR EACH CLASS OF CONCRETE POURED EACH DAY. ONE CYLINDER SHALL BE TESTED AT 3 DAYS AND 7 DAYS, THREE AT 28 DAYS, AND ONE RESERVED TO BE TESTED AT 56 DAYS IF REQUIRED. FOLLOW ASTM STANDARDS FOR SAMPLING AND TESTING. ONE SLUMP TEST SHALL BE TAKEN FOR EACH SET OF TEST CYLINDERS CAST. SLUMP TEST SHALL CONFORM WITH ASTM C 143. NO CONCRETE TEST WILL BE ACCEPTED IF CONCRETE IS TEMPERED WITH IN ANY WAY AFTER SAID TEST IS PERFORMED. REPEAT TEST IF WATER IS ADDED AFTER INITIAL SAMPLING.

G. TRANSPORTING, PLACING, CURING AND DEPOSITING OF CONCRETE SHALL COMPLY WITH ACI 301-16. SPECIFICATIONS FOR STRUCTURAL CONCRETE.

H. CONSTRUCTION JOINTS IN STRUCTURAL SLABS AND BEAMS SHALL BE LOCATED AT 1/3 OF THE SPAN WITH REINFORCING CONTINUOUS ACROSS THE JOINT. PROVIDE A CONTINUOUS 2 X 4 SHEAR KEY AT SLABS. AT BEAMS PROVIDE A 1 1/2" DEEP SHEAR KEY WITH A WIDTH 8" SMALLER THAN THE BEAM WIDTH AND A DEPTH 8" SMALLER THAN THE BEAM DEPTH. LOCATIONS SHALL BE APPROVED BY STRUCTURAL ENGINEER OF RECORD BEFORE POUR.

I. CONCRETE USED AT BALCONIES AND TERRACES SHALL HAVE A WATER CEMENT RATIO OF 0.40.

K. MAXIMUM WATER/CEMENT RATIO FOR CONCRETE CONTAINING A SUPERPLASTICIZER ADMIXTURE SHALL BE 0.40. SLUMP AFTER ADDITION OF SUPERPLASTICIZER SHALL BE 6" +/- 1"

L. MINIMUM CONCRETE COVER FOR REINFORCEMENT:

I. CONCRETE EXPOSED TO EARTH OR WEATHER

| | |
|---------------------|--------|
| #5 BARS AND LARGER | 2" |
| #5 BARS AND SMALLER | 1 1/2" |

II. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WTH EARTH:

| | |
|-------------------|----------------------------|
| BEAMS AND COLUMNS | 1 1/2" TO TIES OR STIRRUPS |
|-------------------|----------------------------|

WIND ANALYSIS DESIGN PARAMETERS:

- 1.-WIND DESIGN HAS BEEN DONE IN ACCORDANCE WITH ASCE 7-16 AND 2020 FBC (7TH EDITION), HVHZ.
- 2.-EXPOSURE 1B
- 3.-RISK CATEGORY: II
- 4.-INTERNAL PRESSURE COEFFICIENT, Gcwp: -0.18
- 5.-ULTIMATE WIND VELOCITY, V= 175 MPH

DIMENSIONS

1.-WHILE THE POSITION OF MOST CONCRETE, STEEL, AND DECKING MEMBERS ARE DEFINED DIRECTLY ON THE STRUCTURAL DRAWINGS THERE ARE INSTANCES WHERE REFERENCE MUST BE MADE TO ARCHITECTURAL OR OTHER DRAWINGS TO DEDUCE A DIMENSION. THE CONTRACTOR IS RESPONSIBLE FOR SUCH DIMENSIONAL COORDINATION AND CROSS REFERENCING.

2.-WITH THE POSITION OF MOST CONCRETE, MASONRY, STEEL, AND DECKING MEMBERS THUS FIXED, THE CONTRACTOR SHALL STILL NEED TO DEDUCE AND COMPUTE OTHER DIMENSIONS THAT ARE DERIVATIVE FROM THE BASIC DIMENSIONS. THESE MAY INCLUDE TRUE DISTANCE BETWEEN WORK POINTS, TRUE LENGTH, AND ORIENTATION OF MEMBERS, AND SO ON. SUCH DERIVATION OR DIMENSIONS IS THE RESPONSIBILITY OF THE CONTRACTOR.

3.-TO ENSURE ACCURACY OR THESE DERIVED DIMENSIONS, THE CONTRACTOR IS TO PRODUCE LAYOUT DRAWINGS FOR COORDINATION WITH OTHER TRADES, AS WELL AS DETAILED SHOP DRAWINGS, ALTHOUGH THEY WILL NOT BE CHECKED, THESE LAYOUT DRAWINGS ARE TO BE SUBMITTED AT THE SAME TIME AS THE RELEVANT SHOP DRAWING.

SHORING AND RESHORING

1.-SHORING AND RESHORING DRAWINGS SHALL BE PREPARED BY A STATE OF FLORIDA REGISTERED SPECIALTY ENGINEER WITH A MINIMUM OF TEN YEARS OF EXPERIENCE IN SHORING AND RESHORING DESIGN AND DETAILING.

2.-SHORING AND RESHORING DRAWINGS SHALL INCLUDE AT LEAST THE FOLLOWING ITEMS:

- 2.1-LOCATION, SIZE, TYPE AND CAPACITY OF ALL SHORING.
- 2.2-LOCATION, SIZE, TYPE, AND CAPACITY OF ALL RESHORING.
- 2.3-LOCATION, SIZE, AND TYPE OF ALL BLOCKING, MUD SILLS, TEMPORARY LATERAL BRACING AND OTHER ACCESSORIES REQUIRED TO ADEQUATELY AND SAFELY SUPPORT AND BRACE THE STRUCTURE DURING CONSTRUCTION.
- 2.4-INSTALLATION PROCEDURE, SEQUENCE OF INSTALLATION, LOAD RELIEF AND REMOVAL OF ALL SHORING AND RESHORING.
- 3.-SHORING AND RESHORING SUBMITTAL FOR APPROVAL SHALL INCLUDE AT LEAST TWO COPIES FOR THE BUILDING DEPARTMENT, ONE FOR THE ENGINEER OF RECORD, ONE FOR THE THRESHOLD INSPECTOR, AND ONE FOR THE ARCHITECT.
- 4.-DESIGN, DETAIL, AND ERECT FORMS, SHORING AND RESHORING IN COMPLIANCE WITH ACI 308-14, PROJECT SPECIFICATIONS, AND THESE NOTES. FORMS, SHORING AND RESHORING SHALL BE DESIGNED FOR THE WEIGHT OF THE FLOOR OR ROOF, A CONSTRUCTION LOAD OF 50 PSF, AND FOR THE CUMULATIVE LOADS OF THE SUPPORTED HORIZONTAL CONCRETE MEMBERS. USE A DESIGN FACTOR OF SAFETY OF 3 FOR WOOD SHORES AND 2 FOR METAL SHORES.
- 5.-THE MAXIMUM SUPERIMPOSED CONSTRUCTION LOAD APPLIED TO FLOORS SUPPORTING SHORES OR RESHORES SHALL NOT EXCEED 75% OF THE DESIGN LIVE LOAD SPECIFIED FOR SLABS (AND JOISTS WHERE APPLICABLE) AND 80% OF THE LIVE LOAD SPECIFIED FOR BEAMS. NO CONSTRUCTION LOAD SHALL BE APPLIED TO ANY MEMBER UNTIL THE CONCRETE IS A MINIMUM OF 14 DAYS OLD AND THE 7 DAY STRENGTH IS THE SPECIFIED 28 DAY STRENGTH.
- 6.-FORMS MAY BE REMOVED 72 HOURS AFTER CONCRETE POUR PROVIDED THAT CONCRETE STRENGTH IS 70% OF THE SPECIFIED 28 DAY STRENGTH AND NOT LESS THAN 3500 PSI. RESHORE EACH BAY IMMEDIATELY AFTER FORMS ARE STRIPPED AND REMOVED. REMOVAL OF FORMS IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. REMOVAL OF FORMS SHALL BE CARRIED OUT IN SUCH A WAY AS TO NOT DAMAGE THE STRUCTURE, INSURE SAFETY AND PREVENT CREEP DEFLECTION OF STRUCTURAL MEMBERS.

SHOP DRAWINGS AND SUBMITTALS

1.-THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR THE ARCHITECT'S REVIEW PRIOR TO COMMENCEMENT OF WORK.

2.-SHOP DRAWINGS WILL BE REVIEWED FOR COMPLIANCE WITH CONTRACT DOCUMENTS, CONSTRUCTION METHODS, DIMENSIONS AND OTHER TRADE REQUIREMENTS BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE PROJECT ENGINEER. DRAWINGS WITHOUT CONTRACTOR'S APPROVAL STAMP SHALL BE RETURNED WITHOUT ENGINEER'S REVIEW.

3.-IN CASE OF A CONFLICT, INFORMATION PRESENTED ON STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER THAT WITHIN SHOP DRAWINGS, UNLESS SPECIFICALLY ADDRESSED BY THE ENGINEER IN WRITING.

4.-THROUGH THE PROCESS OF A CURSORY REVIEW, ENGINEER ASSUMES NO RESPONSIBILITY FOR DIMENSIONS, QUANTITIES, ERRORS OR OMISSIONS. ANY ERRORS OR OMISSIONS RESPECTIVE OF ENGINEER'S COMMENTS OR DURATION OR THE REVIEW SHALL BE THE RESPONSIBILITY OF AND MUST BE CORRECTED BY THE CONTRACTOR AT NO ADDITIONAL SERVICE CHARGE EVEN IF SUCH WORK WAS DONE IN ACCORDANCE WITH THE SHOP DRAWINGS.

5.-THE ENGINEER ASSUMES TEN WORKING DAYS FOR SHOP DRAWING REVIEW TIME (FROM THE DATE OF RECEIPT), IN CASES WHERE THE VOLUME OF SUBMITTED SHOP DRAWINGS IS VIEWED AS EXCESSIVE, THE ENGINEER RESERVES THE RIGHT TO NOTIFY THE OWNER, ARCHITECT, AND THE CONTRACTOR OR THE ADDITIONAL TIME REQUIRED TO PERFORM A QUALITY REVIEW. ALL STRUCTURAL SHOP DRAWINGS AND PRODUCT SUBMITTALS SHALL BE SUBMITTED ELECTRONICALLY IN FULL SIZE PDF FORMAT. HARD COPY SUBMITTAL WILL NOT BE ACCEPTED.

6.-REPRODUCTION OR ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR RESUBMITTED AS SHOP DRAWINGS IS PROHIBITED. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND RETURNED.

7.-ON FIRST SUBMITTAL, CLEARLY FLAG AND CLOUD ALL DIFFERENCES FROM THE CONTRACT DOCUMENTS ON RESUBMITTED, FLAG AND CLOUD ALL CHANGES AND ADDITIONS TO PREVIOUS SUBMITTAL; ONLY CLOUDED ITEMS WILL BE REVIEWED FOR RESUBMITTED SHOP DRAWINGS.

8.-THE CONTRACTOR SHALL SUBMIT, FOR REVIEW, DRAWINGS AND CALCULATIONS FOR ALL OF THE FOLLOWING ASSEMBLIES. THE DESIGN OF THESE ASSEMBLIES IS THE RESPONSIBILITY OF THE CONTRACTOR'S DELEGATED/ SPECIALTY ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. ALL SUBMITTALS SHALL BEAR THIS ENGINEER'S SEAL & SIGNATURE. REVIEW SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECT PARAMETERS AS INDICATED ON THE DRAWINGS AND THE GENERAL NOTES.

8.1-NON-LOAD BEARING STUD WALL AND CURTAIN WALL SYSTEMS AND RELATED CONNECTIONS: DESIGN SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES. REFER TO SPECIFICATION FOR DETAILED REQUIREMENTS.

8.2-PROVIDE COMPLETE SHORING AND RE-SHORING DRAWINGS PREPARED BY OR UNDER THE DIRECT SUPERVISION OR A DELEGATED/SPECIALTY ENGINEER AND CONFORMING TO THE REQUIREMENTS OF THE SPECIFICATIONS AND THE BUILDING CODE.

9.-DELEGATED/SPECIALTY ENGINEER SUBMITTALS: SUBMITTALS SHALL BE PREPARED IN ACCORDANCE WITH THE BUILDING CODE, ENGINEERS NAME, LICENSE NUMBER AND BUSINESS ADDRESS SHALL BE LEGIBLY INDICATED ON ALL SIGNED AND SEALED DOCUMENTS. SPECIALTY ENGINEER SHALL BE SOLELY RESPONSIBLE FOR DIRECT CONTACT WITH THE BUILDING DEPARTMENT WHILE OBTAINING BUILDING DEPARTMENT'S APPROVAL FOR HIS/HER PORTION OR WORK (INCLUDING PROVIDING RESPONSES TO REVIEW COMMENTS, SUPPLYING ADDITIONAL CALCULATIONS AND PLANS, ATTENDING MEETINGS, ETC). DELEGATED/SPECIALTY ENGINEER IS DEFINED AS ONE WHO SPECIALIZES IN AND UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS OR STRUCTURAL SYSTEMS INCLUDED IN A SPECIFIC SUBMITTAL PREPARED FOR THIS PROJECT AND IS AN EMPLOYEE OR OFFICER OR, OR CONSULTANT TO, THE CONTRACTOR OR FABRICATOR RESPONSIBLE FOR THE SUBMITTAL.

10.-IN ADDITION TO THE ABOVE, THE STRUCTURAL ENGINEER'S REVIEW OF DELEGATED/SPECIALTY ENGINEER SUBMITTAL IS LIMITED TO VERIFYING THAT THE SPECIFIED STRUCTURAL SUBMITTAL HAS BEEN FURNISHED, SIGNED AND SEALED BY THE DELEGATED/ SPECIALTY ENGINEER AND THAT THE DELEGATED/SPECIALTY ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE. THE DELEGATED/SPECIALTY ENGINEER IS SOLELY RESPONSIBLE FOR HIS/HER DESIGN, INCLUDING BUT NOT LIMITED TO THE ACCURACY OF HIS/HER CALCULATIONS AND COMPLIANCE WITH THE APPLICABLE CODES AND STANDARDS.

11.-TOWER CRANE (SHOP DRAWINGS REQUIRED): FOUNDATIONS AND BRACING FOR THE CRANE SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER. SIGNED AND SEALED SET OF SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED TO THE THRESHOLD INSPECTOR FOR REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION. LOCATION SHALL BE APPROVED BY THE ENGINEER/ARCHITECT. ANY ALTERED, RE-DESIGNED OR REWORKED CRANE AS A RESULT OF THE CONSULTING CRANE AND THE STRUCTURE SHALL BE BEING TO THE CONTRACTOR BY THE ENGINEER OR RECORD AT AN HOURLY RATE. COST ADDITIONAL MATERIALS AND LABOR FOR THE STRUCTURAL REVISIONS ASSOCIATED WITH PLACEMENT OF CRANE SHALL BE COVERED BY THE CONTRACTOR.

DEMOLITION:

SUBMITTALS

A. SUBMIT SCHEDULE INDICATING PROPOSED SEQUENCE OF OPERATION FOR SELECTIVE DEMOLITION WORK TO OWNER FOR REVIEW AND APPROVAL PRIOR TO COMMENCEMENT OF WORK. INCLUDE METHOD OF DEMOLITION AND PLAN FOR REMODELING WORK. COORDINATION FOR SHUT-OFF, COPPING, CONTINUATION OF UTILITY SERVICES AS REQUIRED, TOGETHER WITH DETAILS FOR DUST AND NOISE CONTROL.

B. CERTIFICATION: SUBMIT COPY OF DEMOLITION FORM CURRENT LICENSES.

C. COORDINATE WITH OWNERS CONTINUING OCCUPANCY OF PORTIONS OF EXISTING BUILDING.

PROJECT RECORD DOCUMENTS

A. ACCURATELY RECORD ACTUAL LOCATIONS OF GAPPED UTILITIES, SUBSURFACE OBSTRUCTIONS, AND UNANTICIPATED STRUCTURAL, MECHANICAL AND ELECTRICAL ELEMENTS UNCOVERED DURING DEMOLITION.

QUALITY ASSURANCE

A. ORGANIZE AND PERFORM DEMOLITION WORK TO AVOID DAMAGE TO CONSTRUCTION INTENDED TO REMAIN.

B. DEMOLITION AND TRANSPORTATION OF DEBRIS SHALL COMPLY WITH APPLICABLE CODES AND REGULATIONS GOVERNING THESE OPERATIONS. FEES ARE PAID BY THE CONTRACTOR.

C. CONDUCT DEMOLITION AND REMOVAL OPERATIONS IN AN EXPEDIENT MANNER, WITH PRECAUTIONS TAKEN TO PREVENT DEMOLITION SITE FROM BEING AN ATTRACTIVE NUISANCE.

D. NOTIFY THE OWNER AND A/E OF ANY CONDITIONS CAPABLE OF AFFECTING THE SAFETY OF OCCUPANTS OF ADJACENT BUILDINGS, THE NORMAL USE OF THESE FACILITIES, OR THE PHYSICAL CONDITION OF THE STRUCTURES.

-IN CASE OF ACCIDENTAL DISRUPTION OF UTILITIES OR THE DISCOVERY OF PREVIOUSLY UNKNOWN UTILITIES, STOP WORK IMMEDIATELY AND NOTIFY THE OWNER AND A/E.

-DO NOT CONTINUE WORK UNTIL OWNER, A/E AND CONTRACTOR AGREE ON A PLAN TO CORRECT THE SITUATION OR IDENTIFY UTILITY SERVICE LINE.

REGULATORY REQUIREMENTS

A. CONFORM TO FLORIDA BUILDING CODE 2014 FIFTH EDITION WITH LATEST REVISIONS, FOR DEMOLITION WORK. SAFETY OF STRUCTURES MUST BE CONTROLLED AND SAFEGUARDS REQUIRED DURING CONSTRUCTION.

B. NOTIFY AFFECTED UTILITY COMPANIES BEFORE STARTING WORK AND COMPLY WITH THEIR REQUIREMENTS.

C. DO NOT CLOSE OR OBSTRUCT EGRESS WIDTH TO EXISTS.

D. DO NOT DISABLE OR DISRUPT BUILDING FIRE OR LIFE SAFETY SYSTEMS WITHOUT THREE (3) DAY PRIOR WRITTEN NOTICE TO THE OWNER.

JOB CONDITIONS

A. OWNER WILL BE CONTINUOUSLY OCCUPYING AREAS OF BUILDING AND SITE IMMEDIATELY ADJACENT TO AREAS OF SELECTIVE DEMOLITION. CONDUCT DEMOLITION WORK IN MANNER THAT WILL MINIMIZE DISRUPTION OF OWNERS NORMAL OPERATIONS. PROVIDE MINIMUM OF FIVE (5) WORKING DAYS ADVANCE NOTICE TO OWNER OF DEMOLITION ACTIVITIES, WHICH WILL SEVERELY IMPACT OWNER'S NORMAL OPERATIONS.

B. EXISTING WORK NOT SPECIFIED FOR REMOVAL THAT IS TEMPORARILY REMOVED, DAMAGED EXPOSED OR IN ANY WAY DISTURBED OR ALTERED BY REMOVAL WORK SHALL BE REPAIRED, PATCHED OR REPLACED TO THE OWNER AND A/E SATISFACTION AT NO ADDITIONAL COST TO THE BOARD.

C. PROVIDE BARRIERS AND WARNING DEVICES TO PROTECT THE PUBLIC AND USERS OF ADJACENT FACILITIES.

PREPARATION

A. WHERE SELECTIVE DEMOLITION WHICH WILL CREATE EXCESSIVE DUST OCCURS IMMEDIATELY ADJACENT TO OR WITHIN OCCUPIED PORTIONS OF BUILDING, CONSTRUCT DUST-PROOF PARTITIONS OR BARRIERS TO MITIGATE SPREAD OF AIRBORNE DUST OR DEBRIS.

B. PROVIDE WEATHER-PROOF CLOSURES FOR EXTERIOR OPENINGS RESULTING FROM SELECTIVE DEMOLITION WORK.

C. IDENTIFY STUB OFF AND DISCONNECT UTILITY SERVICES THAT ARE NOT INDICATED TO REMAIN. REMAINING UTILITIES SHALL BE KEPT ACTIVE.

DEMOLITION

A. DISCONNECT, REMOVE, CAP AND IDENTIFY DESIGNATED UTILITIES WITHIN DEMOLITION AREAS.

B. DEMOLISH IN AN ORDERLY AND CAREFUL MANNER. PROTECT EXISTING SUPPORTING STRUCTURAL MEMBERS AND ALL ITEMS TO REMAIN.

C. PROMPTLY REMOVE DEBRIS TO AVOID IMPOSING EXCESSIVE LOADS ON SUPPORTING WALLS, FLOORS OR FRAMING.

D. IF UNANTICIPATED MECHANICAL, ELECTRICAL OR STRUCTURAL ELEMENTS WHICH CONFLICT WITH INTENDED FUNCTION OR DESIGN ARE ENCOUNTERED INVESTIGATE MEASURE BOTH NATURE EXTENT OF THE CONFLICT. SUBMIT REPORT TO OWNER IN WRITTEN ACCURATE DETAIL.

DISPOSAL OF DEMOLISHED MATERIALS

A. REMOVE DEBRIS, RUBBISH OTHER MATERIALS RESULTING FROM SELECTIVE DEMOLITION OPERATIONS FROM BUILDING SITE. LEGALLY TRANSPORT AND DISPOSE OF MATERIALS OFF SITE ON A REGULAR BASIS.

B. ACCUMULATION OF DEBRIS ON THE SITE WILL NOT BE ALLOWED.

C. IF HAZARDOUS MATERIALS ARE ENCOUNTERED DURING DEMOLITION OPERATIONS, COMPLY WITH APPLICABLE REGULATIONS LAWS, ORDINANCES CONCERNING REMOVAL, HANDLING AND PROTECTION AGAINST EXPOSURE OF ANY ENVIRONMENTAL POLLUTION.

D. BURNING OF REMOVED MATERIALS WILL NOT BE PERMITTED ON PROJECT SITE.

REPAIR

A. REPAIR DEMOLITION PERFORMED IN EXCESS OF THAT REQUIRED. RETURN DAMAGED STRUCTURES, SURFACES TO REMAIN TO CONDITIONS EXISTING PRIOR TO COMMENCEMENT OF SELECTIVE DEMOLITION WORK. REPAIR ADJACENT CONSTRUCTION SURFACES SOILED OR DAMAGED BY SELECTIVE DEMOLITION WORK, TO MATCH EXISTING.

CLEAN UP

A. UPON COMPLETION OF SELECTIVE DEMOLITION WORK, REMOVE TOOLS, EQUIPMENT, DEMOLISHED MATERIALS FROM SITE. REMOVE PROTECTIONS, LEAVE ROOF AREAS BROOM CLEAN.

MASONRY NOTES

1.-ALL MASONRY WORK SHALL BE IN CONFORMANCE WITH THE SPECIFICATION SECTION 04200, THE FOLLOWING NOTES, AND THE BUILDING CODE.

1.1-ACI 530/ASCE 5, "BUILDING CODE REQUIREMENTS OR CONCRETE MASONRY STRUCTURES"

1.2-ACI 530.1/ASCE 6, "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY."

2.-ALL HOLLOW CONCRETE MASONRY UNITS (C.M.U.) SHALL BE NORMAL WEIGHT UNITS CONFORMING TO ASTM C90, TYPE 1. AGGREGATES SHALL CONFORM TO ASTM C331, ALL HOLLOW AND SOLID C.M.U. SHALL ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 1800 PSI AS DETERMINED BY A.S.T.M. C-140, YIELDING A PRISM STRENGTH (FM) OF 1500 PSI MINIMUM AS DETERMINED BY ASTM E447. USE 50% SOLID, NOMINAL 16" OR 12"x8"x16" CMU UNITS PER PLAN. SAW OUT UNITS WHICH ARE NOT IN MULTIPLES OF 8" COURSING. UNITS SHALL BE AT LEAST 8" LONG. BOND CORNERS BY LAPPING ENDS 8" IN SUCCESSIVE VERTICAL COURSES.

3.-EXCEPT WHERE STACK BOND IS INDICATED ON THE ARCHITECTURAL DRAWINGS, E.G. AT MAIN ENTRY AND LOBBY, LAY UNITS IN RUNNING BOND USING TWO-CORE C.M.U. THROUGHOUT THE PROJECT EXCEPT WHERE SOLID C.M.U. IS SPECIFIED IN THE CONTRACT DOCUMENTS OR REQUIRED FOR MAINTAINING A FIRE-RATED ASSEMBLY.

4.-PRIOR TO MASONRY CONSTRUCTION ONE SET OF THREE MASONRY PRISMS SHALL BE BUILT AND TESTED IN ACCORDANCE WITH A.S.T.M. E447, THE MATERIALS AND WORKSMANSHIP USED TO BUILD THE PRISMS SHALL BE REPRESENTATIVE OF THOSE THAT WILL BE CONSTRUCTED WITHIN THE ACTUAL PROJECT CONSTRUCTION. THE TEST RESULTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW WITHIN 24-HOURS OF THE TEST.

5.-MASONRY CONSTRUCTION SHALL NOT COMMENCE UNTIL TEST RESULTS HAVE BEEN APPROVED BY BOTH THE ARCHITECT/ENGINEER AND THE GENERAL CONTRACTOR. ALL COSTS ASSOCIATED WITH THIS PRE-CONSTRUCTION TESTING SHALL BE BORNE BY THE GENERAL CONTRACTOR.

6.-MINIMUM EQUIVALENT SOLID THICKNESS, AS DETERMINED BY A.S.T.M. 0140, OF INDIVIDUAL C.M.U. AND FIRE-RATED C.M.U. WALLS SHALL BE AS FOLLOWS:

| | |
|--------------------|--------------------------------------|
| 6.1-4" CMU: 2.28" | 6.5-ONE HOUR FIRE-RATED WALL: 3.0" |
| 6.2-6" CMU: 3.21" | 6.6-TWO HOUR FIRE-RATED WALL: 4.5" |
| 6.3-8" CMU: 4.50" | 6.7-THREE HOUR FIRE-RATED WALL: 5.7" |
| 6.4-12" CMU: 5.70" | 6.8-FOUR HOUR FIRE-RATED WALL: 6.7" |

7.-WHERE A C.M.U. WALL IS SPECIFIED IN THE CONTRACT DOCUMENTS AS HAVING A PARTICULAR FIRE-RATING, THE MINIMUM EQUIVALENT SOLID THICKNESS, AS SHOWN HEREIN ABOVE, ASSOCIATED WITH THE SPECIFIED FIRE-RATING SHALL BE MAINTAINED FOR THE LULL HEIGHT AND LENGTH OF THE WALL WHETHER OR NOT THE NOMINAL C.M.U. THICKNESS VARIES WITHIN THE WALL.

8.-ALL MORTAR SHALL CONFORM TO ASTM C270, TYPE N OR S, EXCEPT USE TYPE M MORTAR BELOW GRADE, WITH THE FOLLOWING CONSTITUENTS AND PROPORTIONS:

- 8.1-PORTLAND CEMENT: ASTM C150 TYPE 1.
- 8.2-HYDRATED LIME ASTM C207, TYPE S.
- 8.3-SAND: ASTM 0144
- 8.4-WATER: POTABLE.
- 8.5-COLOR: AS PER ARCHITECT/ENGINEER.
- 8.6-PROPORTIONS: ONE PART PORTLAND CEMENT, 1/4 TO 1/2 PARTS HYDRATED LIME, 2 1/2 TO 3 PARTS SAND - ALL MEASURED BY VOLUME OF CEMENT.
- 8.7-MASONRY CEMENT, BLENDED HYDRAULIC CEMENTS, ELY ASH, POZZOLANS AND GROUND GRANULATED BLAST FURNACE SLAG SHALL NOT BE USED.

9.-MORTAR HEAD AND BED JOINTS SHALL BE 3/8" FOR THE THICKNESS. REMOVE MORTAR PROTRUSIONS EXTENDING 1/2" OR MORE INTO THE CELLS TO BE GROUTED.

- 9.1-SOLID UNITS SHALL BE SET WITH LULL HEAD AND BED JOINTS.
- 9.2-HOLLOW UNITS SHALL BE SET WITH LULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL LACE SHELLS
- 9.3-FACE SHELL, WEBS ARE TO BE FULLY MORTARED IN ALL COURSES OF PIERS, COLUMNS, AND PILASTERS
- 9.4-FULLY MORTAR IN THE STARTING COURSE AND WHERE AN ADJACENT CELL IS TO BE GROUTED.

10.-ALL GROUT SHALL CONFORM TO ASTM C476 WITH A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 3000 PSI AT THE END OF 28-DAYS AS DETERMINED BY ASTM C1019, A SLUMP AT POINT OF DISCHARGE OF 8 INCHES TO 10 INCHES AS DETERMINED BY ASTM C143 AND WITH THE FOLLOWING CONSTITUENTS AND PROPORTIONS:

- 10.1-PORTLAND CEMENT: ASTM C150, TYPE 1. FLY ASH, POZZOLANS, AND GROUND IRON BLAST-FURNACE SLAG SHALL NOT BE USED.
- 10.2-AGGREGATES: ASTM C404.
- 10.3-WATER: POTABLE.
- 10.4-LINE GROUT PROPORTIONS: ONE PART PORTLAND CEMENT, 2 1/4 TO 3 PARTS FINE AGGREGATE ALL MEASURED BY VOLUME OF CEMENT.
- 10.5-COARSE GROUT PROPORTIONS: ONE PART PORTLAND CEMENT, 2 1/4 TO 3 PARTS FINE AGGREGATE, 1 TO 2 PARTS COARSE AGGREGATE (3/4" MAXIMUM STONE SIZE) - ALL MEASURED BY VOLUME OF CEMENT.

11.-ALL GROUTING PROCEDURES SHALL CONFORM TO ASTM C-476 AND NOMA "TEN SERIES #23A". ALL GROUT SHALL BE "FINE" UNLESS OTHERWISE SPECIFIED ON PLANS OR DETAILS.

11.1-MINIMUM COMPRESSIVE STRENGTH SHALL BE 3000 PSI IN 28 DAYS.

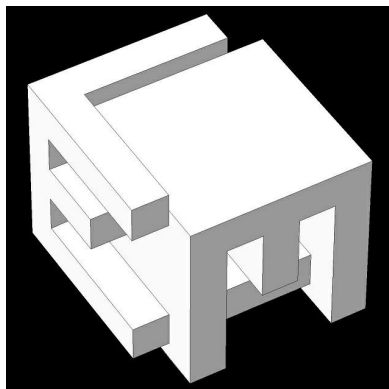
STRUCTURAL NOTES

STRUCTURAL STEEL

A. STRUCTURAL STEEL SHALL COMPLY WITH AISC "SPECIFICATIONS FOR DESIGN, FABRICATION AND ERECTION FOR STRUCTURAL STEEL BUILDINGS," NINTH EDITION.
B. STRUCTURAL STEEL SHAPES AND PLATES SHALL CONFORM TO ASTM A 36, Fy 36 KSI, U.O.N.
C. STRUCTURAL STEEL TUBES SHALL CONFORM TO ASTM A500, GRADE B, Fy=46 KSI.
D. STRUCTURAL STEEL PIPES SHALL CONFORM TO ASTM A53, TYPE S, GRADE B, Fy= 35 KSI.
E. ANCHOR BOLTS SHALL CONFORM TO EITHER ASTM A 307 OR ASTM A 36.
F. FRAMING BOLTS SHALL CONFORM TO ASTM A325, WITH HARDENED WASHERS AND HEX NUTS.
G. ALL EXTERIOR STEEL SHAPES, PLATES, NUTS, BOLTS, WASHERS SHALL BE HOT-DIPPED GALVANIZED.
H. THE STEEL STRUCTURE IS DESIGNED AS A WHOLE WITH THE FLOOR AND ROOF DIAPHRAGM ACTION BRACING THE FRAMES AGAINST GRAVITY AND LATERAL AND VERTICAL WIND FORCES. PROVIDE ALL TEMPORY BRACING AS REQUIRED IN ORDER TO MAINTAIN STEEL STRUCTURE STABLE UNTIL THE STRUCTURE IS COMPLETE.
I. SPLICING OF STEEL MEMBERS IS NOT ALLOWED, UNLESS SPECIFIED IN STRUCTURAL DRAWINGS OR APPROVED BY ENGINEER OF RECORD.
J. ALL BOLTS, NUTS AND WASHERS SHALL BE NEW, RUST-FREE, CLEAN AND WELL LUBRICATED.
K. BOLT HOLES THROUGH STEEL MEMBERS SHALL BE SHOP-DRILLED, CUT OR PUNCHED. DO NOT USE TORCH OR FLAME TO CUT OR ENLARGE HOLES.
L. ALL STRUCTURAL STEEL TUBE OR PIPE COLUMNS SHALL BE FILLED WITH 3000 PSI. CONCRETE GROUT. PROVIDE 1/4" DIAMETER WEEP HOLES EACH SIDE "3" FROM TOP AND BOTTOM OF COLUMN.
M. DO NOT PAINT PARTS OF STEEL MEMBERS TO BE EMBEDDED IN CONCRETE AND SURFACES TO BE IN CONTACT WITH CONCRETE.
N. FOR FIREPROOFING OF STRUCTURAL STEEL MEMBERS SEE ARCHITECTURAL DRAWINGS.
O. SEE ARCHITECTURAL, MECHANICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL MISCELLANEOUS STRUCTURAL STEEL NOT SHOWN IN STRUCTURAL DRAWINGS.
P. FOR PAINTING OF NON-GALVANIZED STRUCTURAL STEEL SEE STRUCTURAL STEEL PROJECT SPECIFICATIONS.
Q. WELDING SHALL BE DONE WITH E-70 ELECTRODES, UNLESS OTHERWISE NOTED, CONFORMING TO AWS D1.1. R. ALL SHOP AND FIELD WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1: STRUCTURAL WELDING CODE, LATEST EDITION. ALL WELDERS SHALL BE AWS-CERTIFIED. SUBMIT WELDER CERTIFICATES TO ARCHITECT/ENGINEER FOR APPROVAL BEFORE ANY SHOP OR FIELD-WELDING IS STARTED.

STRUCTURAL WOOD

1. WOOD DESIGN IN CONFORMANCE WITH NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION PER AF & PA 2018 STANDARDS AND CHARTER 9 OF FLORIDA BUILDING CODE-RESIDENTIAL TH EDITION (2020).
2. TO BE AIR DRIED, WELL SEASONED AND GRADE MARKED AT MILL.
3. ALL STRUCTURAL WOOD TO BE SURFACED 4 SIDES (S-4-S) AND A MAXIMUM MOISTURE CONTENT OF 19% UNLESS OTHERWISE NOTED.
4. ALL LUMBER AND PLYWOOD IN CONTACT WITH CONCRETE, STUCCO, MASONRY OR OTHER CEMENTITIOUS MATERIALS SHALL BE TREATED TO COMPLY WITH AWPA STANDARD LP-2.
5. STORE ALL LUMBER ABOVE GRADE OR FLOOR. STACK TO ALLOW PROPER AIR CIRCULATION AND PROTECT FROM WETTING WITH SUITABLE COVER.
6. ALL WOOD TRUSSES SHALL BE DESIGNED FOR THE SUPERIMPOSED LOADS GIVEN ON PLAN PLUS WEIGHT OF THE TRUSS, ALSO SHOULD BE DESIGNED TO RESIST GIVEN UPLIFT LOADS.
SUBMIT PLAN(S) AND CALCULATIONS SIGNED AND SEALED BY A FLORIDA REGISTERED ENGINEER FOR A/E REVIEW PRIOR TO FABRICATION. THE SUBMITTALS SHALL INCLUDE THE PROJECT IDENTITY, THE LOADING AND DESIGN CRITERIA; TRUSS DETAIL AND TRUSS FRAMING PLAN SHEETS SHALL IDENTIFY EACH TRUSS AND LIST THE DESIGN CRITERIA AND LOADING SPECIFY ALL MEMBER SIZES, BRACING ANCHORAGE, CONNECTIONS, TRUSS LOCATIONS AND OTHER NECESSARY TEMPORARY AND PERMANENT FABRICATION AND ERECTION INFORMATION. EACH DRAWING SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF THE FLORIDA REGISTERED ENGINEER WHO PREPARED THE DRAWINGS AND CALCULATIONS.
ROOF TRUSSES SHALL BE DESIGNED FOR COMPONENT & CLADDING LOADS UNDER ASCE 7-16. DO NOT MODIFY WOOD TRUSS LAYOUT WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER OF RECORD AND THE APPROVAL OF REVISIONS TO THE MASTER PERMIT BY THE BUILDING DEPARTMENT.
7. PLYWOOD ROOF SHEATHING:
A. PLYWOOD ROOF SHEATHING SHALL BE 19/32" EXPOSURE 1 WITH 32/16 APA SPAN RATING.
B. LAY PANELS, CONTINUOUS OVER TWO OR MORE SPANS AND WITH FACE GRAIN PERPENDICULAR TO PRIMARY FRAMING MEMBERS. END JOINTS SHALL OCCUR AT CENTER OF PRIMARY FRAMING MEMBER WITH BOTH PANELS FASTENED TO IT. END JOINTS SHALL BE STAGGERED.
C. FASTEN PLYWOOD ROOF SHEATHING PANELS TO ALL SUPPORTING MEMBERS USING 8d RING SHANK NAILS SPACED AT 6" ON CENTER AND INTERMEDIATE SUPPORTS. AT A "EDGE" ZONE ALL AROUND THE BUILDING PERIMETER AND AT GABLE END FASTEN PLYWOOD ROOF SHEATHING PANELS TO ALL SUPPORTING MEMBERS USING 8d RING SHANK NAILS SPACED AT 4" ON CENTER.
D. INSTALL PANELS WITH STRENGTH DIRECTION PERPENDICULAR TO TRUSSES/JOISTS CONTINUOUS OVER TWO OR MORE SPANS WITH FACE GRAIN PERPENDICULAR TO SUPPORTS.
PROVIDE FULL 2x4 BLOCKING AT ALL PLYWOOD DECK PANEL EDGES THROUGHOUT. "BLOCKING" REFERS TO INSTALLATION OF 2x4 MEMBERS INSTALLED WITH THE 4" SIDE HORIZONTAL BETWEEN THE TRUSS TOP CHORDS TO PROVIDE EDGE SUPPORT FOR PLYWOOD SHEATHING.
NAILING THE SHEATHING TO THE BLOCKING SHALL BE: 2x4 BLOCKING SHALL BE ATTACHED TO TRUSS TOP CHORD BY TOE-NAILING USING 2-8d RING SHANK NAILS AT EACH END.
8. PLYWOOD FLOOR SHEATHING: (FOR FLOOR APPLICATIONS AND IF REQUIRED FOR ATTICS)
A. USE 3/4" PLYWOOD FLOOR SHEATHING WITH 48/24 SPAN RATING.
B. ALL EDGES OF PLYWOOD PANELS SHALL BE CONTINUOUSLY SUPPORTED BY SUB-JOISTS.
C. LAY PLYWOOD PANELS CONTINUOUS OVER TWO OR MORE SPANS AND WITH FACE GRAIN PERPENDICULAR TO THE TRUSSES. END JOINTS SHALL OCCUR AT CENTER OF JOIST WITH BOTH PANELS FASTENED TO JOIST. END JOINTS SHALL BE STAGGERED. HOLD EDGES OF PANELS 1/2" AWAY FROM MASONRY WALLS.
D. FASTEN PLYWOOD SUB-FLOOR PANELS TO SUPPORTING TRUSSES WITH 10d COMMON NAILS AT 4" O.C. MAXIMUM AT ALL SUPPORTS & 2" O.C. ALONG THE EXTERIOR.
9. FASTENERS A. FASTENERS SHALL BE OF THE TYPE AND SIZE INDICATED IN THIS DRAWING. B. ALL FASTENERS INCLUDING EXPANSION ANCHORS, SLEEVE ANCHORS, STRAPS, NAILS, SCREWS, ETC SHALL BE GALVANIZED (U.O.N.).
10. NAILING REQUIREMENTS: USE 8D RING-SHANK NAILS. RING-SHANK NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS:
1. 0.113 INCH NOMINAL SHANK DIAMETER.
2. RING DIAMETER OF 0.012 OVER SHANK DIAMETER.
3. 16 TO 20 RINGS PER INCH.
4. 0.280 INCH FULL ROUND HEAD DIAMETER.
5. 23/8 INCH NAIL LENGTH.
NAILING SPACING: 4" O.C. AT PANEL EDGES.
8" O.C. AT INTERMEDIATE SUPPORTS.
NAILS SHALL BE CORROSION RESISTANT CONFORMING TO ASTM F 1667 & ASTM A 641.



ELM ARCH

8950 SW 74TH Court
Suite 1204
Miami, FL 33156
786-391-2646

Qualifier: Erik Lloyd Myers
State of Florida: AR 93574

Sign & Seal: Erik Lloyd Myers
State of Florida: AR 93574

OWNER:

Miami Lakes Branch Library

6689 Windmill Gate Road
Miami Lakes, FL 33014

TITLE: GENERAL NOTES

REVISION

2 REVISION 02 07-01-25

DATE

04-03-2023

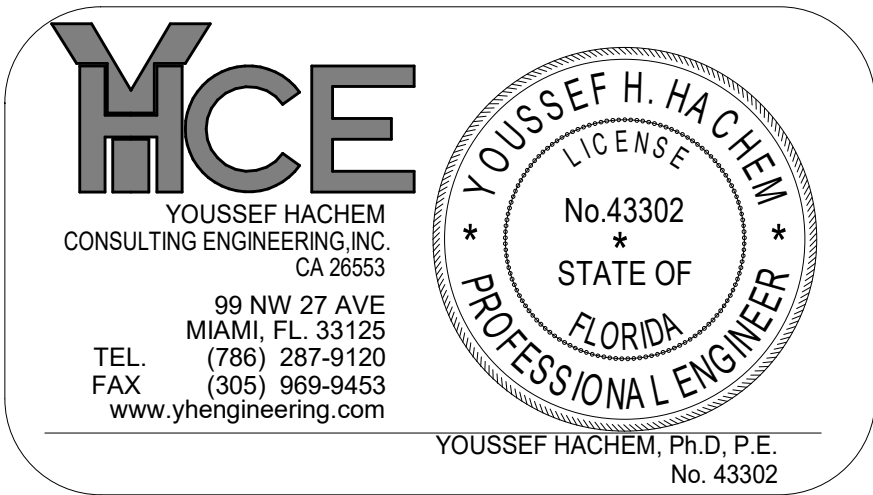
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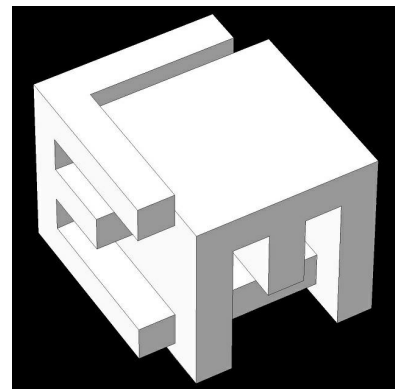
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SHEET NUMBER

S002

OF





8950 SW 74TH Court
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Qualifier: Erik Lloyd Myers
State of Florida: AR 93574
Sign & Seal: Erik Lloyd Myers
State of Florida: AR 93574

OWNER:

Miami Lakes Branch Library

6699 Windmill Gate Road
Miami Lakes, FL 33014

TITLE: ROOF FRAMING PLAN

REVISION
2 REVISION 02 07-01-25

DATE
04-03-2023
PROJECT NUMBER
19119
SHEET NUMBER
S102
OF

NOTES:

1- DIMENSIONS SHOWN SHALL BE FIELD-VERIFIED. ANY DISCREPANCIES SHALL BE NOTED AND THE ENGINEER OF RECORD NOTIFIED BEFORE CONTINUING WITH THE WORK.
2- CONTRACTOR TO SUBMIT SIGNED AND SEALED CALCULATIONS AND SHOP DRAWINGS FOR WINDOWS AND DOORS BY FLORIDA PROFESSIONAL ENGINEER BEFORE FABRICATION FOR APPROVAL BY THE ENGINEER OF RECORD TO SHOW COMPLIANCE WITH THE RESIDENTIAL FLORIDA BUILDING CODE 2020 (SEVENTH EDITION).
3- TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUTES.
4- TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTIVE TREATMENT TO NEW CONSTRUCTION. SEE SECTION 202, "REGISTERED TERMITICIDE," UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES". PER RESIDENTIAL F.B.C. 2020 (SEVENTH EDITION), SECTION R318, R318.1
5- ALL ELECTRICAL, MECHANICAL AND PLUMBING PENETRATIONS THROUGH STRUCTURAL MEMBERS SHALL BE COORDINATED BY THE GENERAL CONTRACTOR. LOCATION AND DIMENSIONS OF EQUIPMENTS TO BE VERIFIED BY SPECIFIC VENDOR PRIOR TO INSTALLATION. SPECIFIC VENDOR IS RESPONSIBLE FOR CHECKING ADEQUANCY OF EQUIPMENT WEIGHTS WITH STATED LOADS USED FOR STRUCTURAL DESIGN. IF LOADS EXCEED THOSE, VENDOR WILL RESPONSIBLE FOR UPDATE THE DESIGN ACCORDINGLY. PERMITTING TASKS AND TIMELINE COMPLIANCE ASSOCIATED WITH NEW DESIGN WILL BE VENDOR RESPONSABILITY AS WELL.

WOOD TRUSS NOTES:

1. TRUSS MANUFACTURER TO PROVIDE DEAD, LIVE, AND WIND UPLIFT REACTIONS FOR ALL TRUSSES AND GIRDERS.
2. STRAP ALL "PIGGYBACK" TRUSSES WITH 14 GA. BY 1" GALV. STEEL STRAPS TO EA. SUPPORTING TRUSS W/ 4-16d NAILS INTO "PIGGYBACK" TRUSS AND INTO SUPPORTING TRUSSES. STRAP BY NU-VUE.
3. AS THE ENGINEER OF RECORD OF THIS DESIGN, I HEREBY STATE THAT THE NET WIND UPLIFT REACTIONS SHOWN IN THIS PLAN SHALL SUPERSEDE THOSE SHOWN IN THE TRUSS MANUFACTURER SHOP DRAWINGS AND ENGINEERING CALCULATIONS. THE NET WIND UPLIFT REACTIONS WERE CALCULATED BY MYSELF, CONSIDERING LOCATION, TRIBUTARY AREAS, HEIGHT, AND ROOF SLOPE IN ACCORDANCE WITH THE ASCE 7-10 CODE FOR A 175 MPH WIND VELOCITY. THE CONNECTORS NOTED IN PLAN EXCEED THE NET WIND UPLIFT REACTIONS SHOWN IN PLAN.
4. MAIN ROOF PLYWOOD SHEATHING SHALL BE EXTENDED UNDER ALL VALLEY TRUSSES.
5. BRACING FOR THE ROOF SYSTEM IS DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURE AND MEETS THE REQUIREMENTS OF CHAPTER 23 F.B.C. 2319.17.2.4
6. OVERSTRESSING OF WOOD FOR WIND DESIGN IS ALLOWED BUT NOT TO OVERSTRESS TRUSS METAL PLATES, WHICH IS NOT ALLOWED BY ASCE 7-16.
7. FOR WATER PROOFING DETAILS SEE ARCHITECTURAL DRAWINGS.

"X": DIAGONAL "X" BRACING COMPOSED OF 2x4'S NAILED TO TRUSSES W/2-16d NAILS @ EACH POINT OF CONTACT, AS PER TPI HIB 91, PAGE 4. SEE DETAIL ON S502.
"BR": CONT. 2x4 WOOD BRACING NAILED TO TOP OF EACH TRUSS BOTTOM CHORD W/2-16d NAILS SPACING EQUALLY AS NOTED, MAX. 10'-0". SEE DETAIL ON S502.

WOOD TRUSSES REACTION

RD+L: DENOTES DEAD LOAD PLUS LIVE LOAD WOOD JOIST REACTION
N.W.U.: DENOTES NET WIND UPLIFT WOOD JOIST REACTION

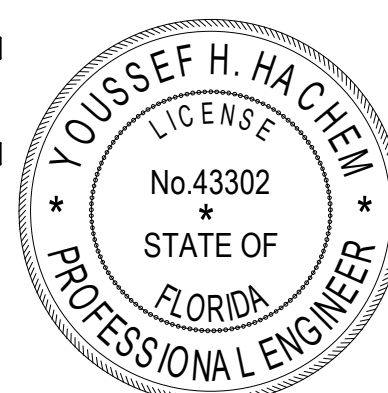
SUPERIMPOSED LOADS

| | | |
|------|------|--------|
| ROOF | DEAD | 30 PSF |
| | LIVE | 30 PSF |

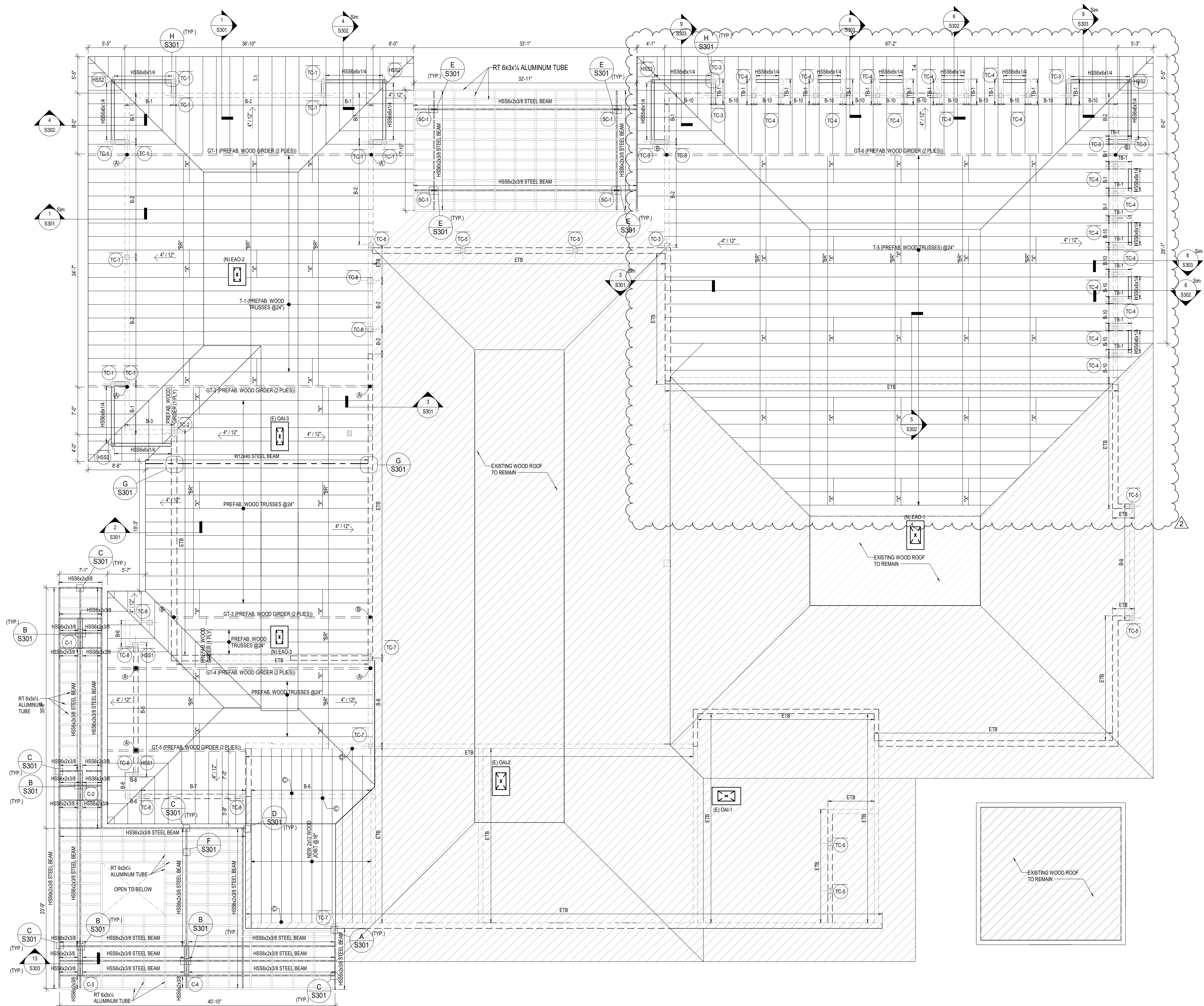
LEGEND:

- DENOTES EXISTING WALLS TO REMAIN
- DENOTES A NEW CONCRETE COLUMN OR WALL BELOW
- DENOTES A NEW COLUMN ENDING AT THIS LEVEL

HCE
YOUSSEF HACHEM
CONSULTING ENGINEERING, INC.
CA 28553
99 NW 27 AVE
MIAMI, FL 33125
TEL. (786) 287-9120
FAX (305) 969-9453
www.yhengineering.com

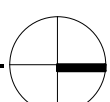


YOUSSEF HACHEM, Ph.D., P.E.
No. 43302



ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

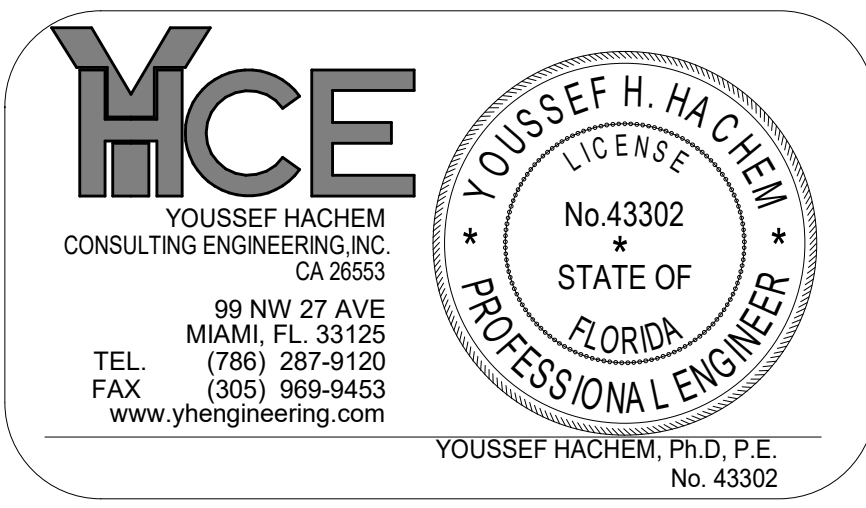


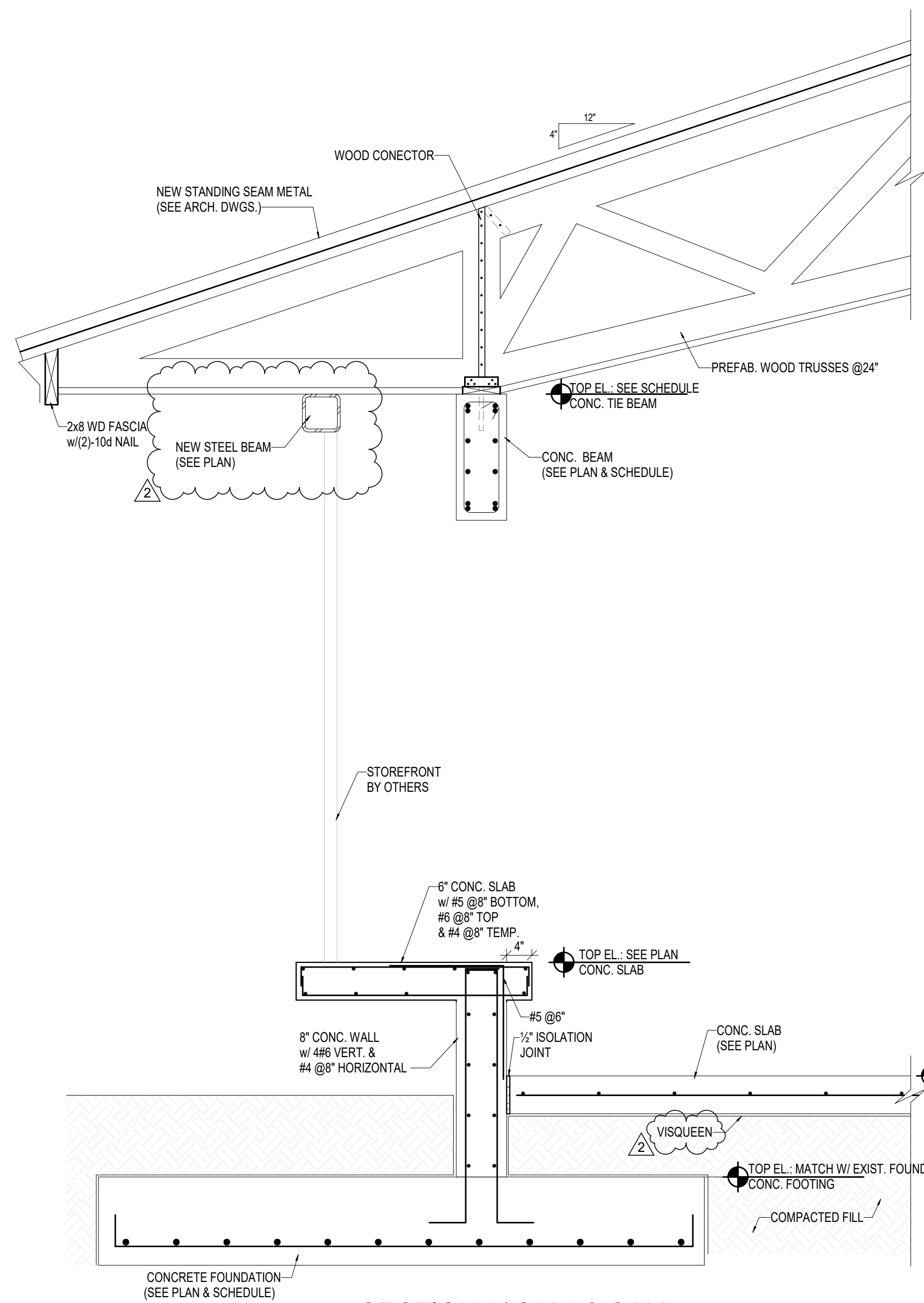


Sign & Seal: Erik Lloyd Myers
State of Florida: AR 93574

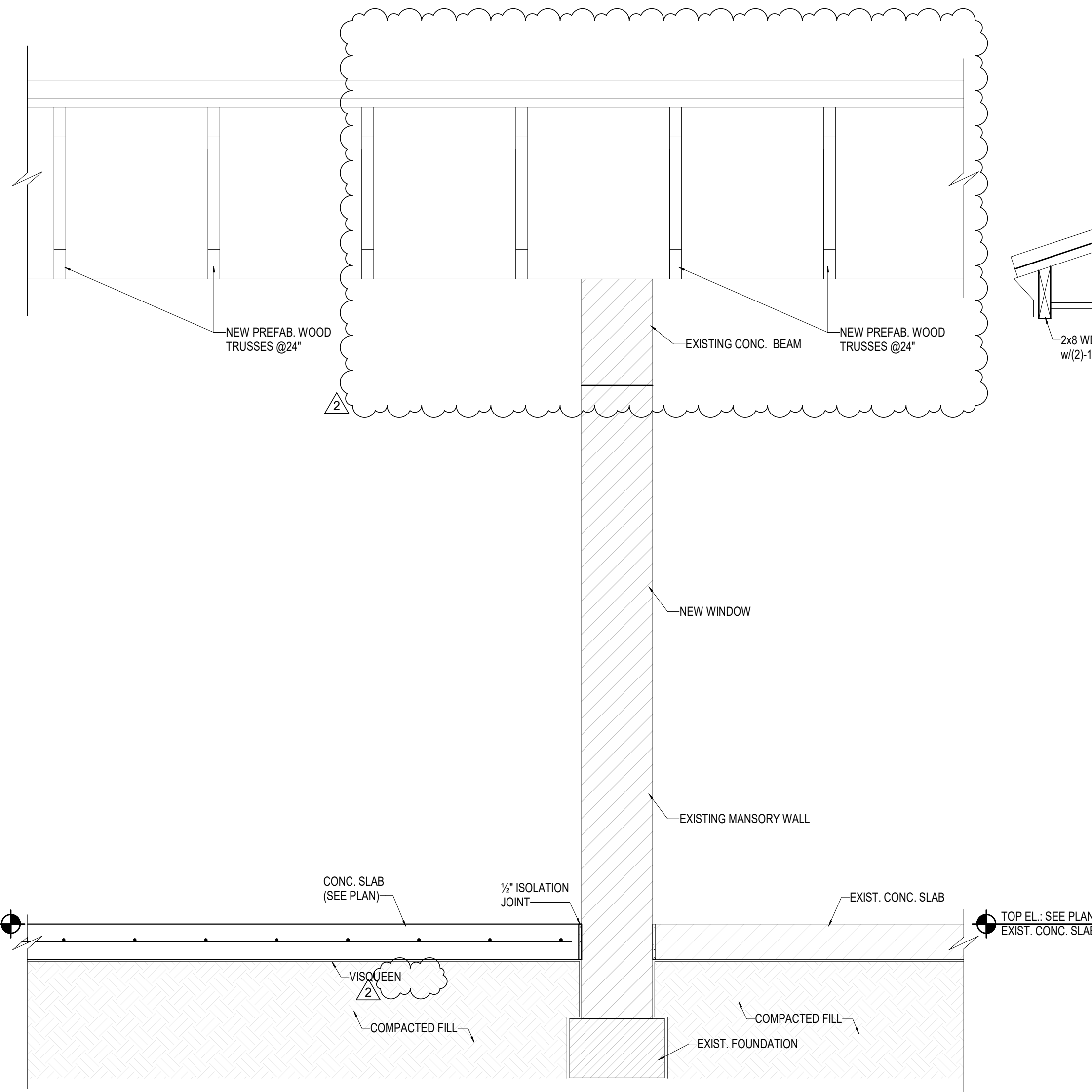
6699 Windmill Gate Road
Miami Lakes, FL 33014

OF

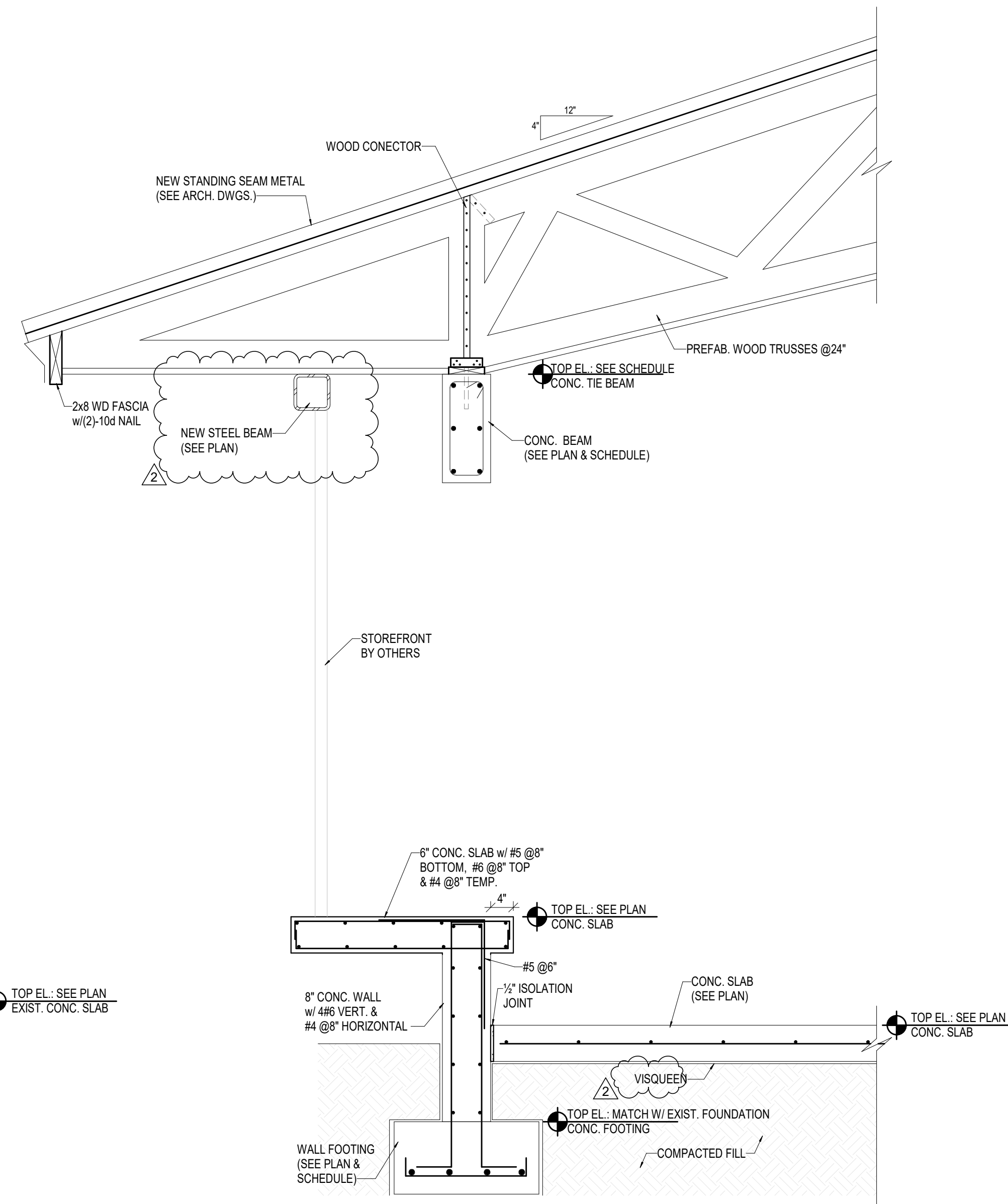




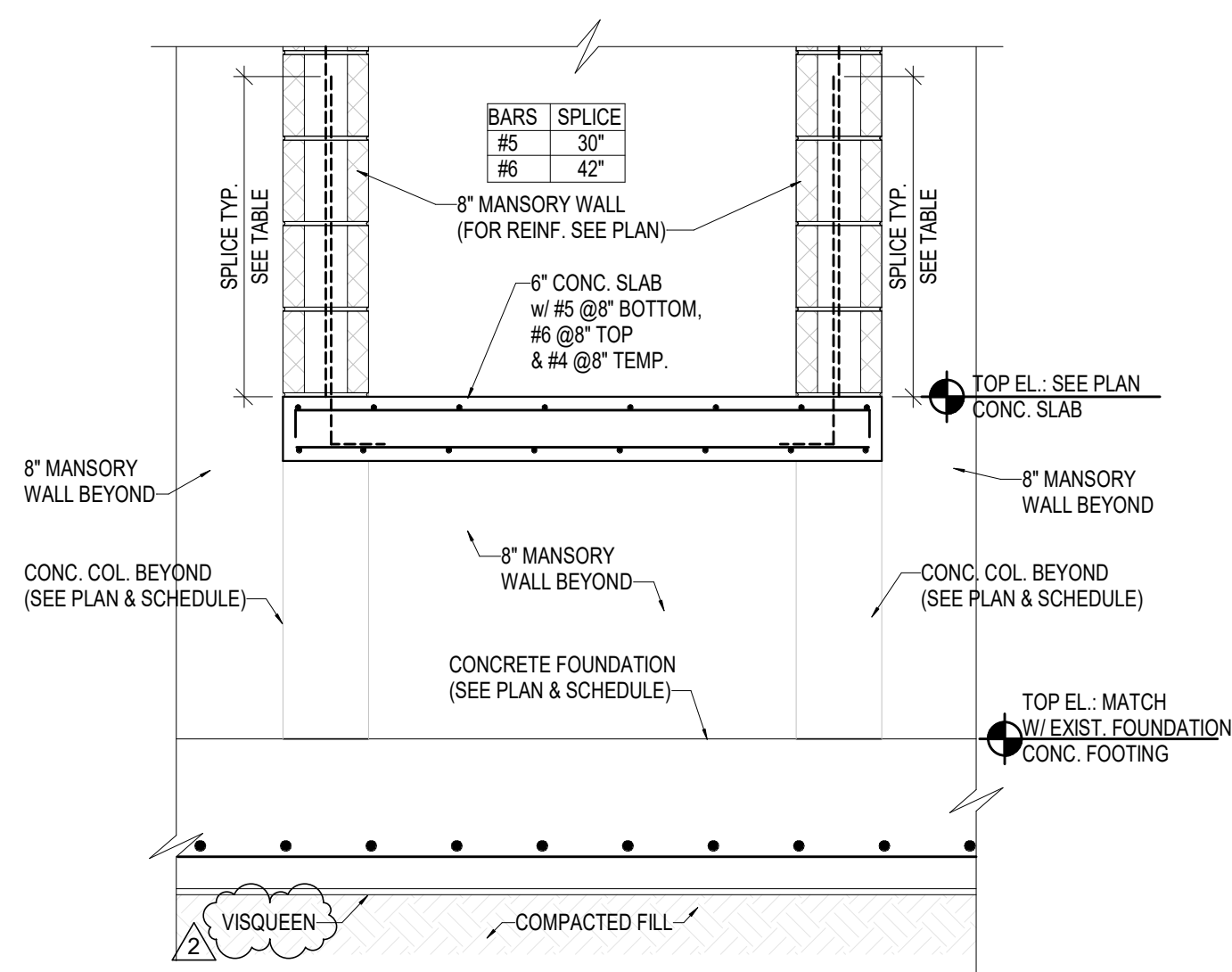
SECTION 4/ S101 & S102
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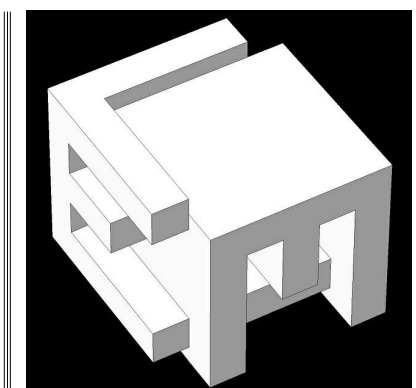
SECTION 5/ S101 & S102
SCALE: 3/4" = 1'-0"



SECTION 6/ S101 & S102
SCALE: 3/4" = 1'-0"



SECTION 7/ S101
SCALE: 3/4" = 1'-0"



ELM ARCH

8950 SW 74TH Court
Suite 1204
Miami, FL 33156
786-391-2646

Qualifier: Erik Lloyd Myers
State of Florida: AR 93574

Sign & Seal: Erik Lloyd Myers
State of Florida: AR 93574

OWNER:

Miami Lakes Branch Library

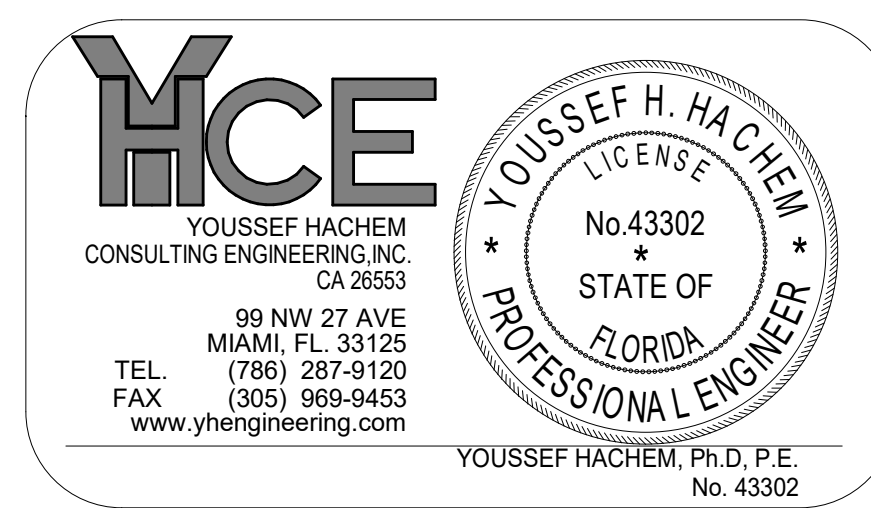
6689 Windmill Gate Road
Miami Lakes, FL 33014

TITLE: SECTIONS

REVISION
2 REVISION 02 07-01-25

DATE
04-03-2023
PROJECT NUMBER
19119
SHEET NUMBER

S302
OF





Sign & Seal: Erik Lloyd Myers
State of Florida: AR 93574

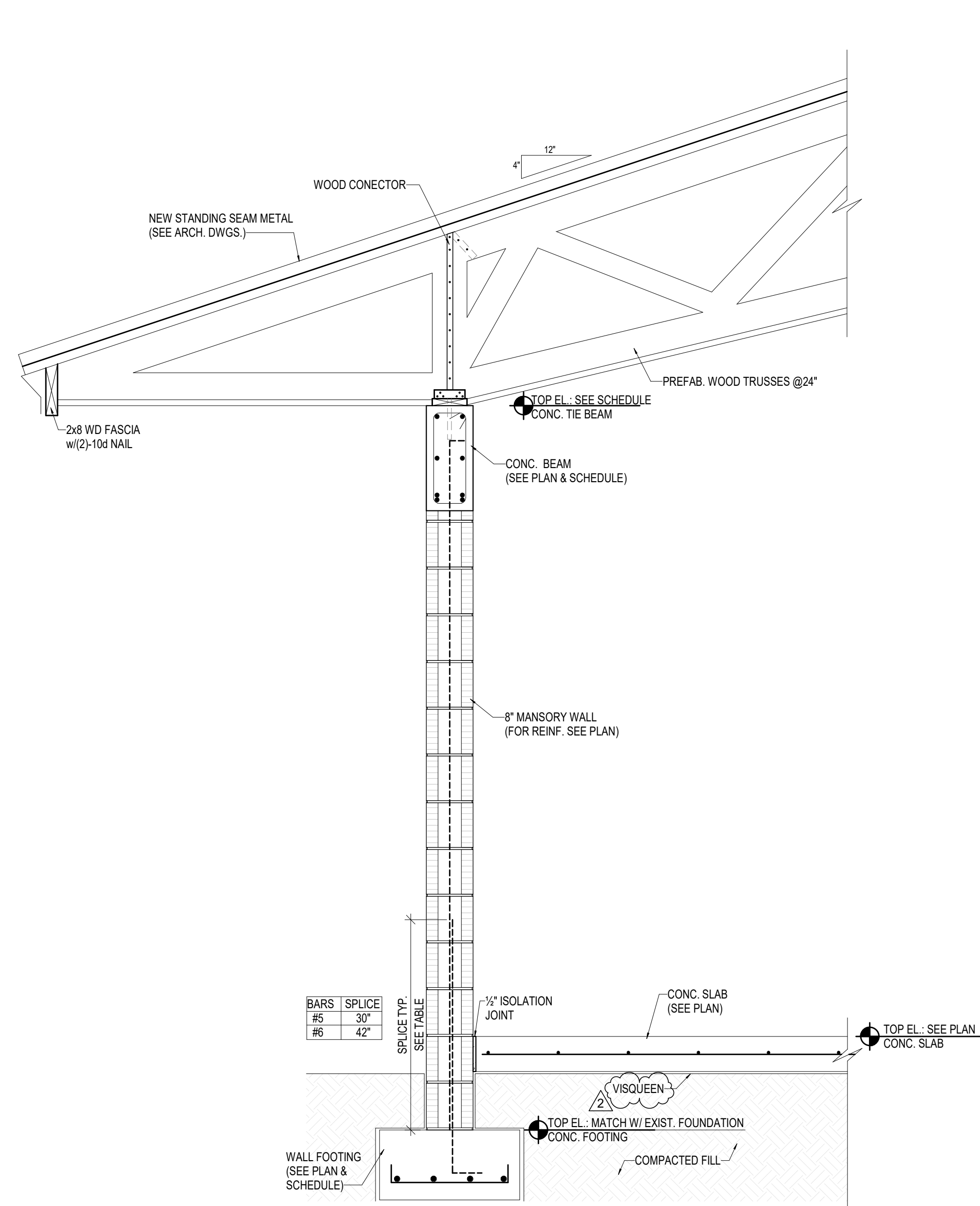
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|----------|-------------|----------|
| 2 | REVISION 02 | 07-01-25 |

DATE
04-03-2023

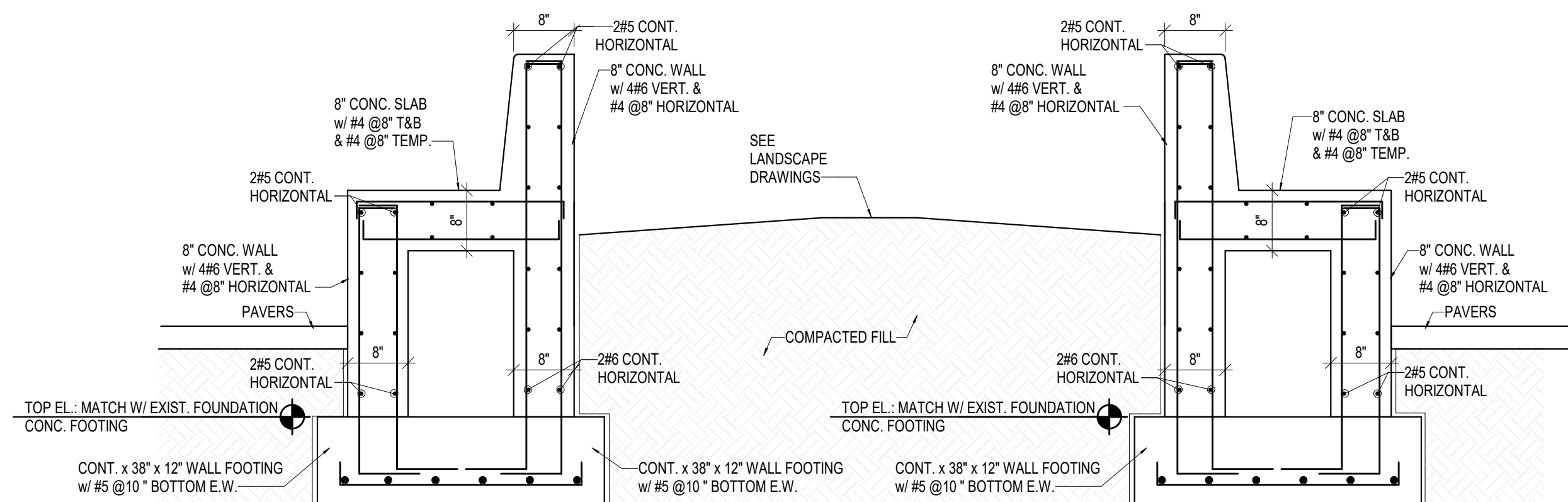
PROJECT NUMBER
19119

S303

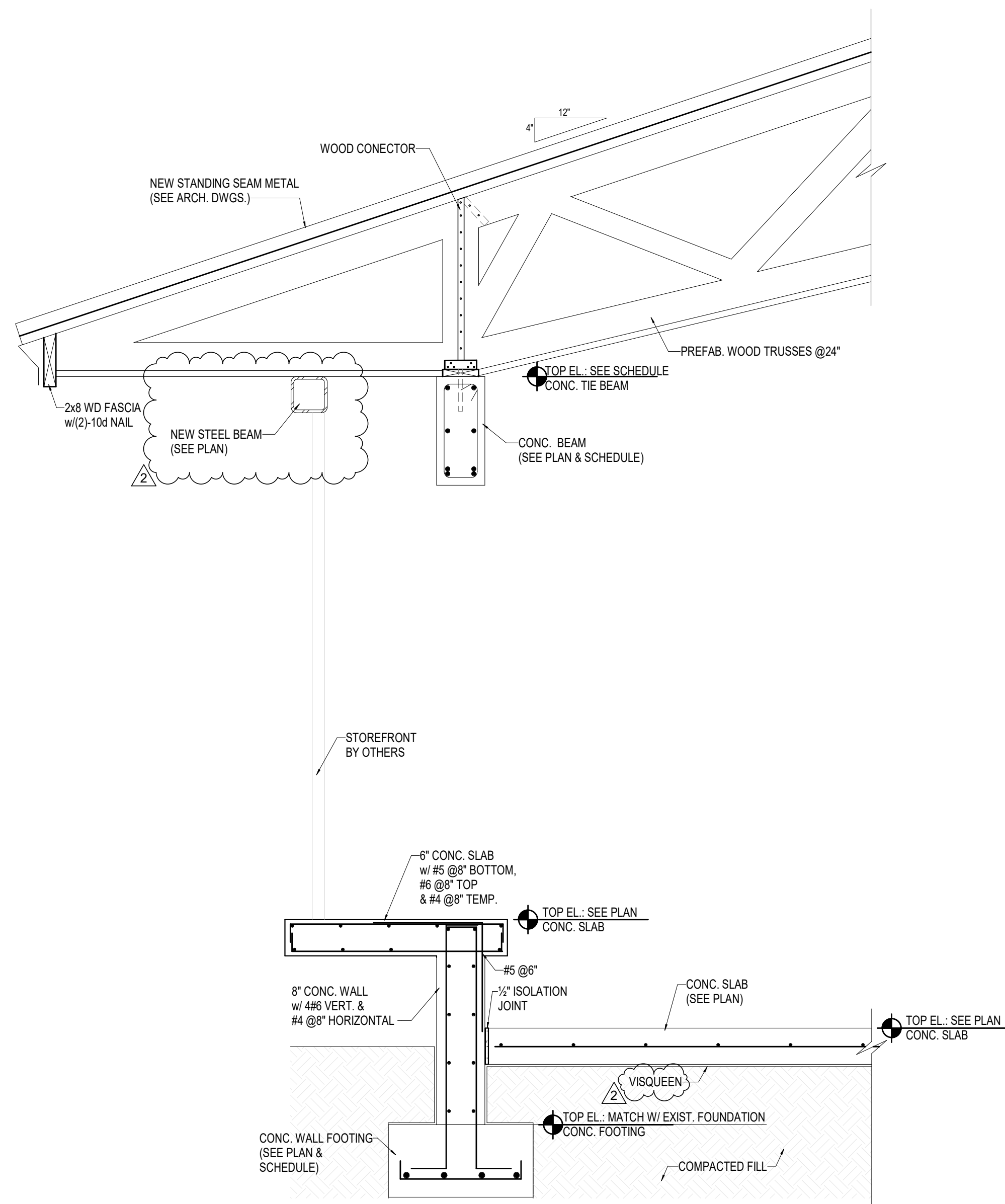
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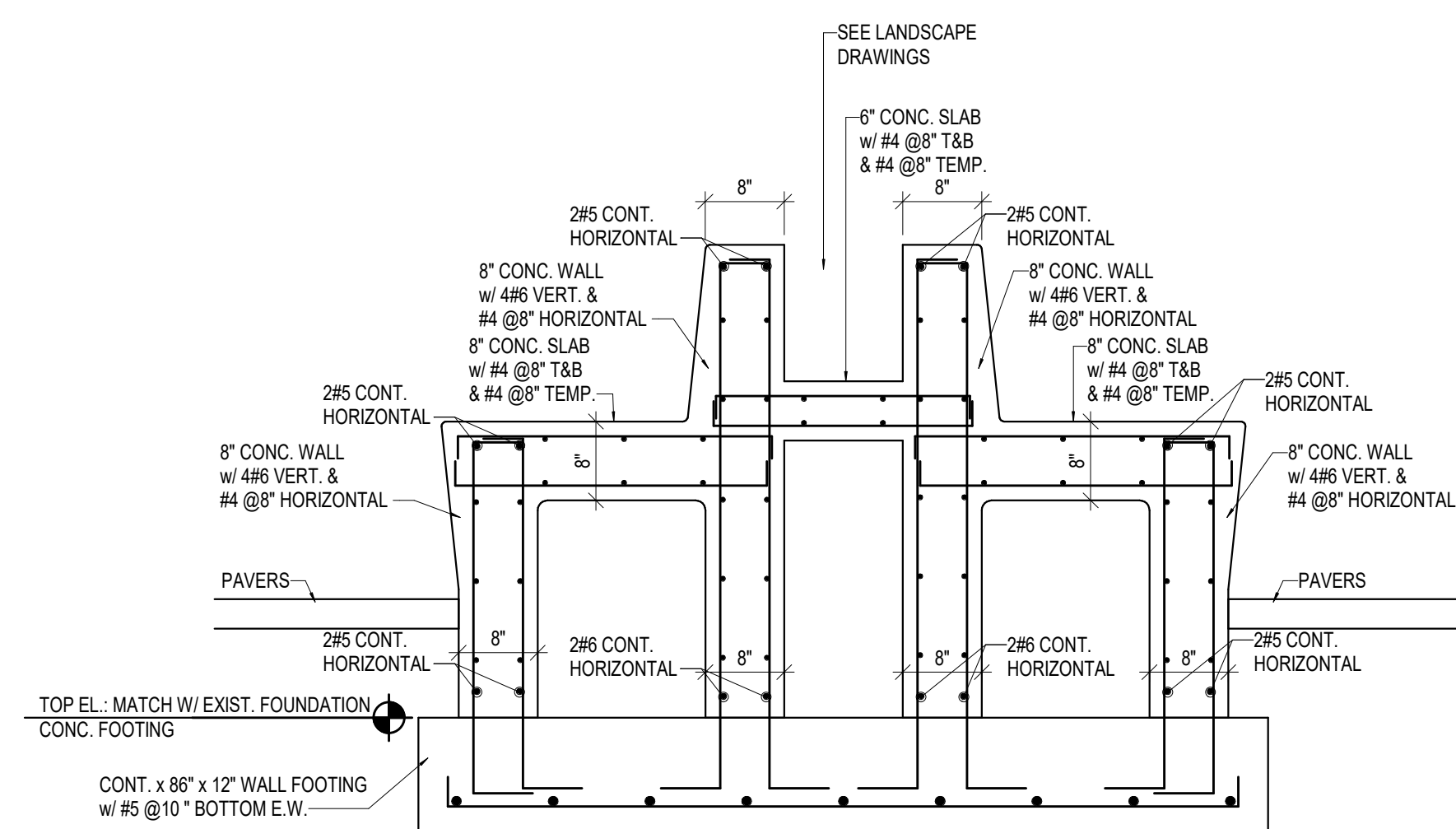
SECTION 8/ S101 & S102
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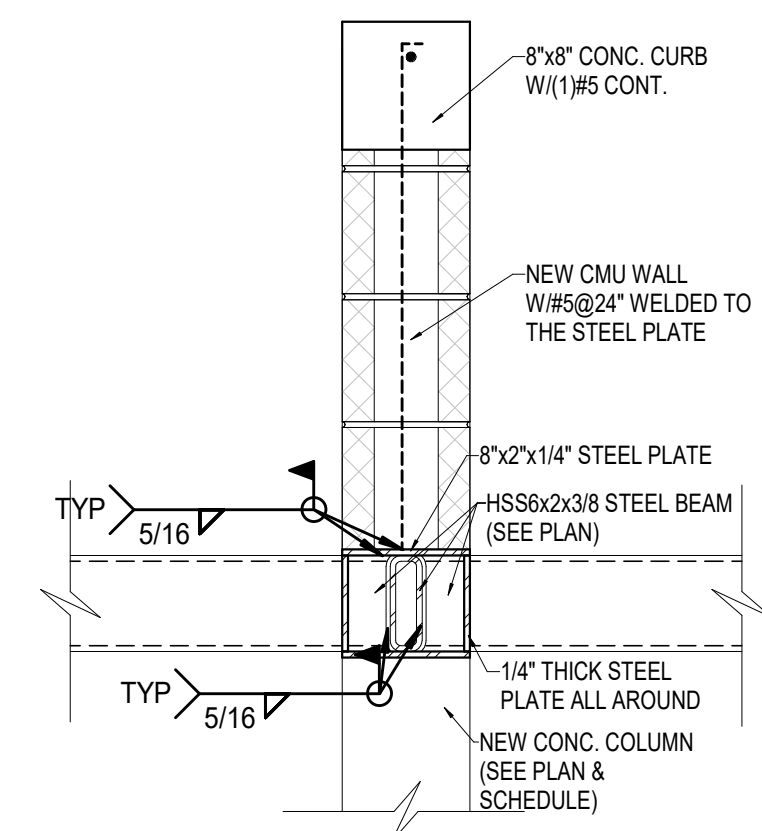
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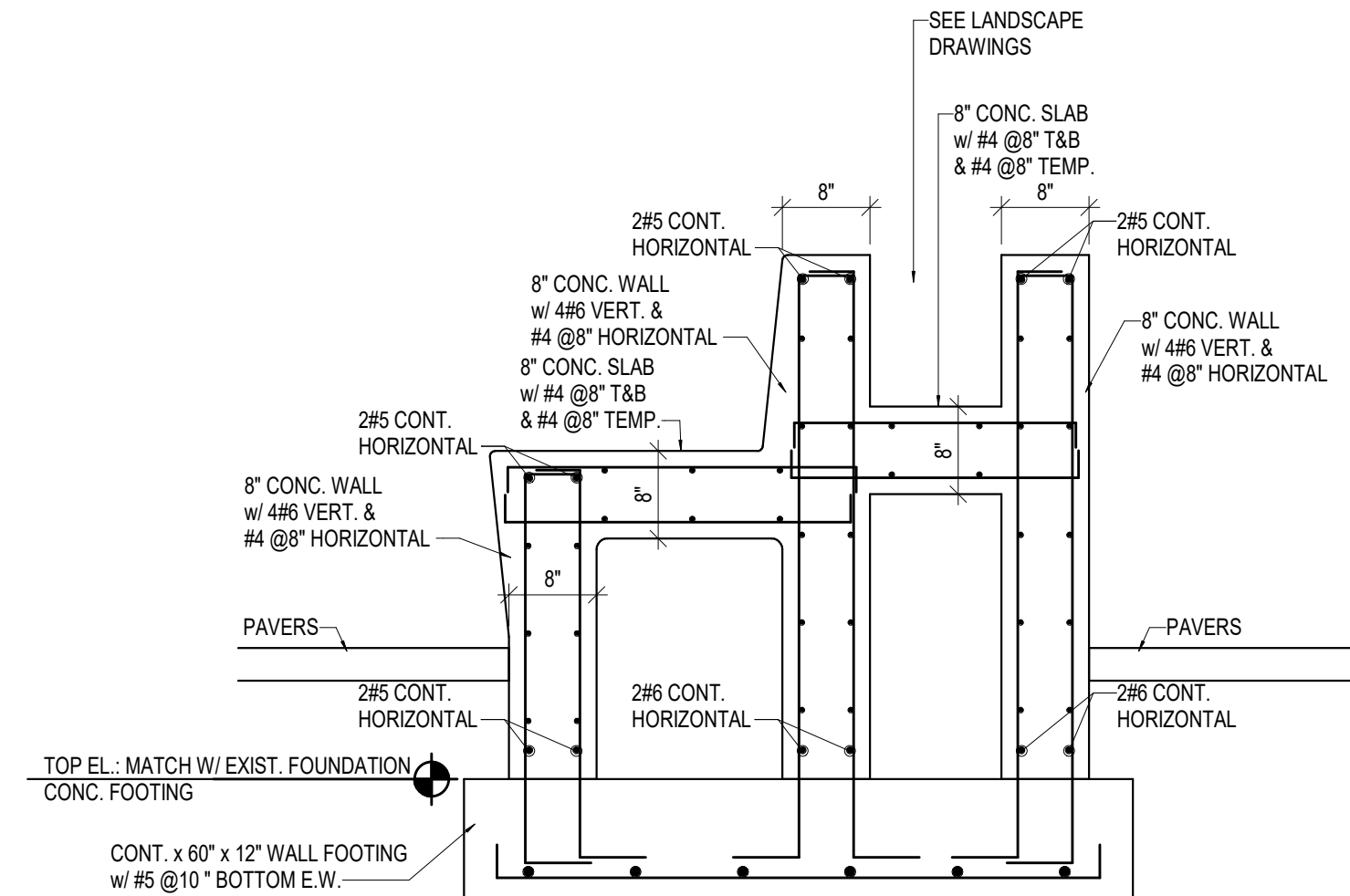
SECTION 9/ S101 &S102
SCALE: 3/4" = 1'-0"



SECTION 11/ S101
SCALE: 3/4" = 1'-0"



SECTION 13/ S102
SCALE: 1" = 1'-0"



SECTION 12/ S101
SCALE: 3/4" = 1'-0"

