# **APPENDIX J**

### Preliminary Stormwater Management Report



#### MIAMI-DADE COUNTY PARKS, RECREATION, AND OPEN SPACES (MDPROS)

#### LUDLAM TRAIL CORRIDOR FROM SW 80<sup>th</sup> STREET TO 400 FEET NORTH OF NW 7<sup>th</sup> STREET MIAMI-DADE COUNTY, FLORIDA P&R CONTRACT NO. 43502-15-001-14470619

P&R PROJECT NO. RFQ 786B FM NO. 444236-1-22-01 ETDM NO. 14369

#### PRELIMINARY STORMWATER MANAGEMENT REPORT

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#### 1 INTRODUCTION

The Miami-Dade County Parks, Recreation and Open Spaces (MDPROS) Department is conducting a Project Development and Environment (PD&E) Study for a continuous public bicycle and pedestrian pathway along the Ludlam Trail Corridor in Miami-Dade County. The project is funded under a Local Agency Program (LAP) Agreement with the Florida Department of Transportation (FDOT) which requires compliance under the National Environmental Policy Act (NEPA). This Preliminary Stormwater Management Report documents the existing conditions, design criteria, and proposed conditions from a drainage point of view.

#### 1.1 Purpose and Scope

The purpose of the proposed Ludlam Trail Corridor is to provide a new route/pathway in Miami-Dade County to encourage the use of alternate modes of transportation and enhance overall connectivity and accessibility to schools, parks, transit stations, and bus stops for more than 30,000 people living within two miles of the proposed project corridor. The proposed project would provide a safe, dedicated, and direct means of non-motorized transportation to and from areas of residences, work, schools, parks, and shopping centers, and would serve bicyclists, pedestrians, and other non-motorized vehicle users.

#### 1.2 **Project Description**

MDPROS is proposing to develop a 5.6-mile multi-use trail within the former Florida East Coast Industries (FECI) railroad corridor (i.e., the Ludlam Trail Corridor, or proposed project). As a future component of the Shared-Use Nonmotorized (SUN) Trail Network, this proposed publicly accessible trail would serve bicyclists, pedestrians, and users of other types of non-motorized vehicles. In addition, the proposed project is expected to provide a safe, dedicated, and direct means of non-motorized transportation to and from areas of residences, work, schools, parks, and shopping centers.

The proposed project limits extend along a segment of the former FECI rail corridor from SW 80th Street to 400 feet north of NW 7th Street, between 69th and 70th Avenues (**See Appendix A**). The project occurs entirely within the former FECI right-of-way (ROW). The ROW for the proposed Ludlam Trail Corridor is approximately 100 feet wide for most of its length, although it narrows to between 75 or 80 feet in some areas. Further, at up to nine (9) roadway crossings, the ROW at the roadway crossing narrows to approximately 40 feet. These roadway crossings include the following: SW 72nd Street/Sunset Drive; SW 56th Street /Miller Drive; SW 40th Street /Bird Road; SW 24th Street/Coral Way; SW 23rd Street; SW 22nd Street; SW 21st Street; SW 8th Street/Calle Ocho; and W Flagler Street.) (**See Appendix B for project Typical Sections**). The proposed project study area extends through sections of the City of Miami, the City of South Miami, and unincorporated Miami-Dade County, and is immediately adjacent to the City of West Miami.

The proposed Ludlam Trail Corridor is located within three (3) South Florida Water management District (SFWMD) Drainage Basins:

- 1) From the Begin Project at SW 80<sup>th</sup> Street to SW 40<sup>th</sup> Street within the C-2 Canal Basin
- 2) From SW 40<sup>th</sup> Street to SW 8<sup>th</sup> Street within the Coral Gables Basin
- 3) From SW 8<sup>th</sup> Street to the End Project just north of NW 7<sup>th</sup> Street within the Tamiami East Basin

This Preliminary Stormwater Management Report presents how the applicable drainage criteria will be addressed as a result of the proposed Ludlam Trail Corridor improvements.

#### 2 DRAINAGE SYSTEM IMPROVEMENTS

There are currently no existing stormwater management systems for the Ludlam Trail Corridor. Stormwater Runoff is permitted to flow directly onto the existing adjacent green areas for storage, where it eventually infiltrates into the ground.

The proposed project improvements, however, will introduce new impervious areas which will increase the stormwater runoff volume. In addition, the new impervious areas will reduce the storage of runoff available from the existing ground surface due to the configuration of the proposed trail. In order to compensate for this increase of runoff and decrease of surface storage, the proposed Ludlam Trail Corridor will include swales (See Appendix B) that must be designed to retain the required water quality treatment volume, retain the increase of stormwater runoff due to the increase of impervious area, and also be able to account for any lost ground surface storage of stormwater runoff.

This Preliminary Stormwater Management Report will quantify the required water quality treatment volume and the required stormwater runoff volume needed to be retained onsite due to the increase of impervious area. Due to the scope of this preliminary study phase, however, the required stormwater retention volume due to the decrease of ground surface storage cannot be determined, but must be taken into account during the final design phase of the project. This will ensure that adjacent property owners will not be adversely affected by the proposed Ludlam Trail Corridor improvements.

#### **3 DESIGN CRITERIA AND PARAMETERS**

This section outlines the South Florida Water Management District (SFWMD) and Miami-Dade County Department of Regulatory and Economic Resources (DRER) stormwater quantity and quality criteria applicable to the proposed drainage systems. The criteria and parameters outlined in this section are derived from the applicable published regulations, permit design manuals, and design standards.

#### 3.1 Design High Water Elevation

The design high water elevation for most projects within Miami-Dade County is typically derived from the Public Works Department Design Standard W.C. 2.2, which is determined from the average October groundwater level data available from 1960 to 1975. Design Standard W.C. 2.2 shows the wet season groundwater elevation within the 3.0 ft-NGVD (1.44 ft-NAVD) elevation contour (See Appendix D).

#### 3.2 Stormwater Quantity Criteria

The stormwater quantity criteria will be based on the most stringent requirement between the SFWMD and DRER criteria.

#### 3.2.1 Design Storms

The DRER's criteria outlined in the Policy for Design of Drainage Structures in Miami-Dade County (December 1980) requires that drainage systems for those other than four lane roads in high density, high traffic areas be designed for a 5-year frequency design storm.

Miami-Dade County DRER also requires that proposed drainage systems meet the offsite discharge requirements per the SFWMD 25 year – 72 hour rainfall event.

#### Table 3-1: Design Storm Events

Design Storm Event			
5 year - 24 hour			
25 year - 72 hour			

#### 3.2.2 Spread

There are no applicable spread criteria for the subject project.

#### 3.3 Stormwater Quality Criteria

The design high water elevation for most projects within Miami-Dade County is typically derived from the Public Works Department Design Standard W.C. 2.2, which is determined from the average October groundwater level data available from 1960 to 1975. Design Standard W.C. 2.2 shows the wet season groundwater elevation within the 3.0 ft-NGVD (1.44 ft-NAVD) elevation contour.

The SFWMD requires that all projects meet state water quality standards, as set forth in Chapter 17-302, Florida Administrative Code (FAC). To assure that these criteria are met, the Project

must meet the following volumetric retention/detention requirements, as describe in the SFWMD Permit Volume IV:

- For wet detention systems, the first inch of runoff from the project or the total runoff from 2.5 inches times the percent impervious, whichever is greater, must be detained on-site. A wet detention system is a system that maintains the control elevation below one foot from the seasonal high groundwater elevation and does not bleed-down more than onehalf inch of detention volume in 24 hours.
- 2. Dry detention systems must only provide 75 percent of the required wet detention volume. Dry detention systems must maintain the control elevation at least one foot above the seasonal high groundwater elevation.
- 3. Retention systems must only provide 50 percent of the wet detention volume.
- 4. For projects with more than 50 percent of imperviousness, discharge to the receiving water bodies must be made through baffles, skimmers, or other mechanisms suitable of preventing oil and grease from discharging to / or from the retention / detention areas.

DRER also requires that all projects meet the state water quality standards. To assure that this criteria is met, 100 percent of the first inch of runoff must be retained on-site. The volume is equivalent to retaining one inch of runoff from the furthest hydrologic point in the project. The methodology for estimating this volume is outlined in DRER's Policy for Design of Drainage Structures as follows:

 $V = 60 C i A T_t$ 

Where:

V = Required stormwater quality volume [cubic feet]

- C = Runoff coefficient [dimensionless]
- i = Storm intensity [inches per hour]
- A = Total tributary area [acres]
- Tt = Duration of storm whose runoff is polluted and contaminated [minutes]
  - = T<sub>1"</sub> + T<sub>c</sub>

Where:

2940 F <sup>-0.11</sup>

T<sub>1"</sub> =

2940 F

308.5 C - 60.5 (0.5895 + F <sup>-0.67</sup>)

Where:

F = Storm frequency [years] C = Runoff coefficient

T<sub>c</sub> = Time of concentration [minutes]

i = Storm intensity [inches per hour]

i =  $\frac{308.5}{48.6 \text{ F}^{-0.11} + \text{T}_{\text{t}} (0.5895 + \text{F}^{-0.67})}$ 

Where:

F = Storm frequency [years]

T<sub>t</sub> = Duration of storm whose runoff is polluted and contaminated [minutes]

For projects such as the Ludlam Trail Corridor, DRER requires that the one-inch of runoff be retained for a rainfall event with a 5-year frequency.

All Water Quality calculations with adherence to the above criteria are provided in Appendix C of this report.

For the proposed drainage design, the more stringent criteria of either the SFWMD or Miami-Dade County DRER will be used.

#### 4 FLOODPLAIN IMPACTS

In accordance with Part 2, Chapter 13 of the PD&E Manual, the project area is generally located outside of the 100-year floodplain, in flood insurance rate Zone X. However, from SW 43<sup>rd</sup> Street to SW 40<sup>th</sup> Street, the project is located within Zone AH with a Base Flood Elevation of 9.00, and from the Tamiami Canal (C-4) to the northern Project Terminus, the project is located within Zone AH with a Base Flood Elevation of 7.00 (See APPENDIX E).

The proposed drainage improvements associated with the project will perform hydraulically in a manner equal to or greater than the existing conditions (no treatment), and backwater surface elevations are not expected to increase. Thus, there will be no significant adverse impacts on natural and beneficial floodplain values. There will be no significant change in flood risk, and there will not be a significant change in the potential for interruption or termination of emergency service or emergency evacuation routes. Any localized flooding will not increase as the result of this project. Therefore, it has been determined that any encroachment upon the base floodplain is not significant.

#### 5 DRAINAGE ANALYSIS RESULTS

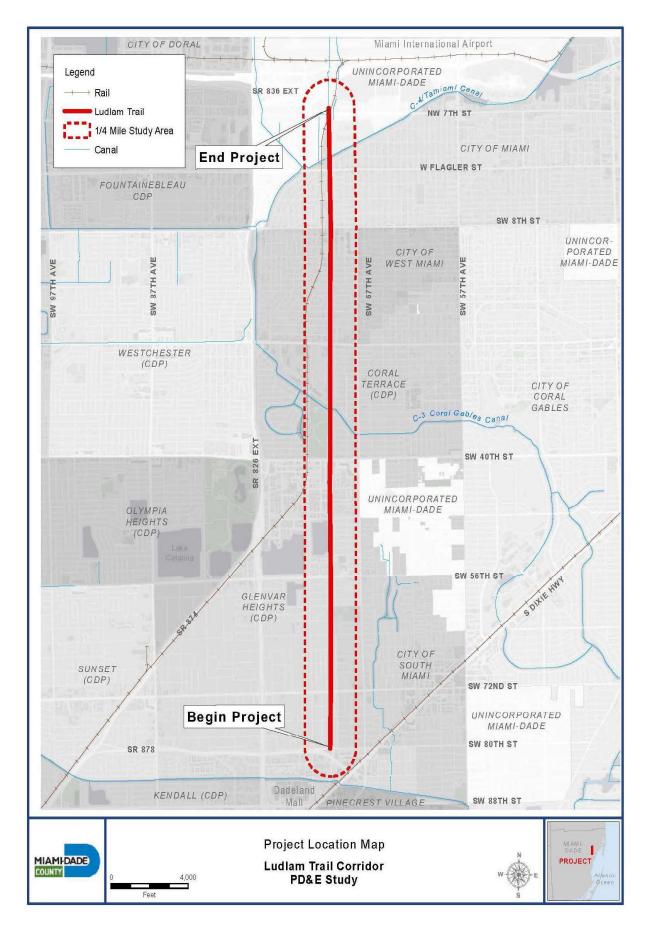
The results of the drainage analysis have been summarized and presented in Appendix C of this report.

Table C-1 gives a description of the different drainage basins and their boundary limits

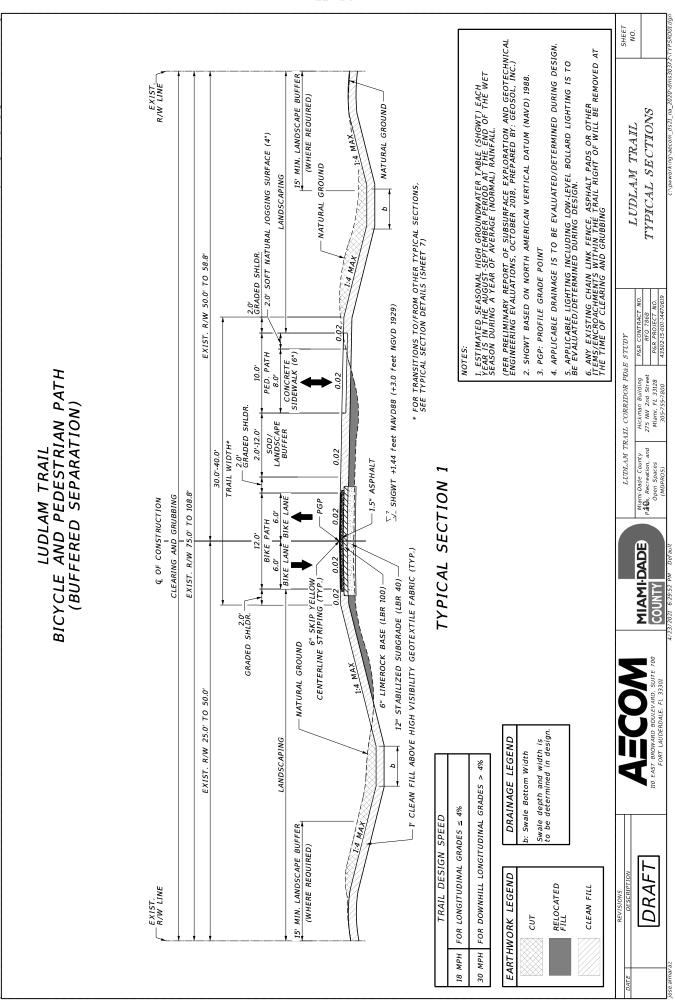
Tables C-2A, C-2B, and C-2C provides the required water quality treatment volumes that need to be retained by the proposed dry retention swales. The required water quality retention volumes are categorized by the areas located within each South Florida Water management District (SFWMD) Drainage Basin.

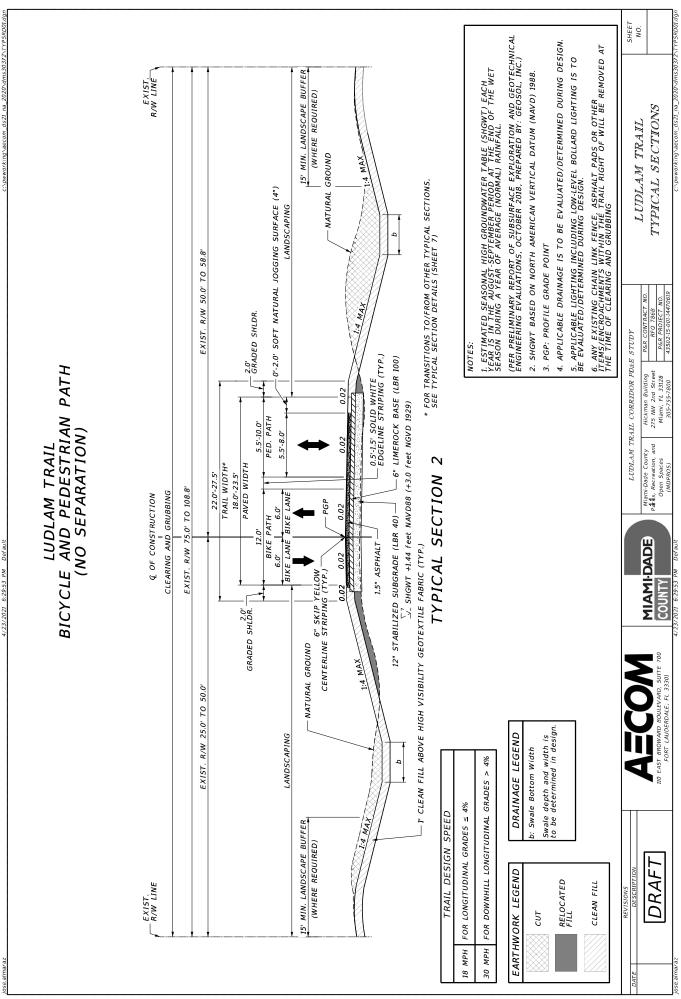
Table C-3 compares the post-development stormwater runoff volume to the pre-development stormwater runoff volume during the SFWMD's 25 year -72 hour design storm. This is the volume of runoff that needs to be retained by the proposed dry retention swales in order to account for the increase of stormwater runoff due to the increase of impervious area.

## APPENDIX A Location Map

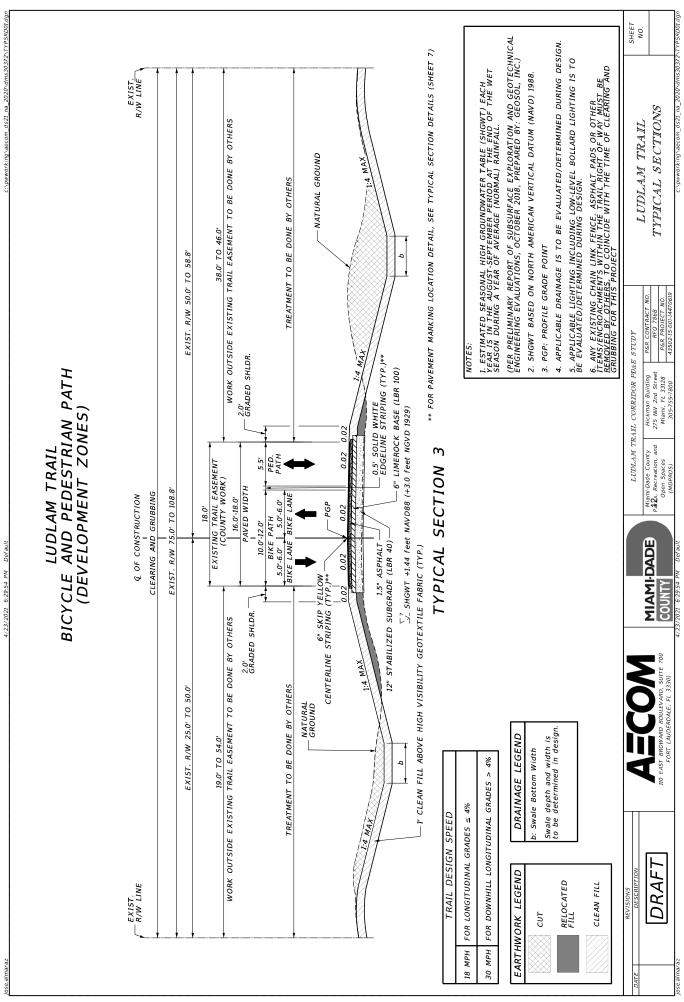


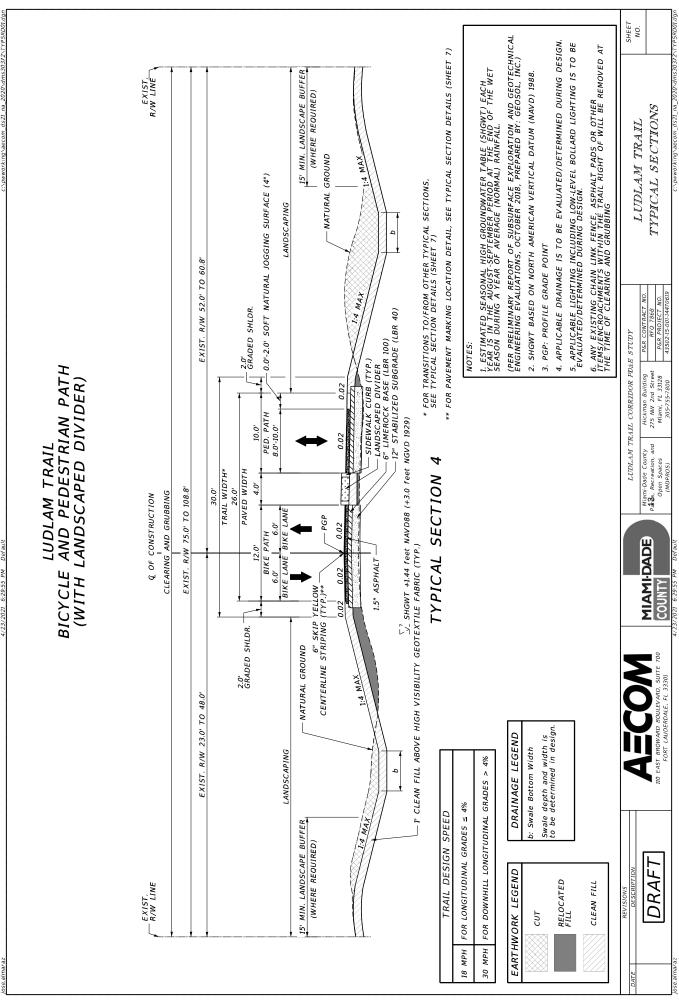
## APPENDIX B Typical Sections

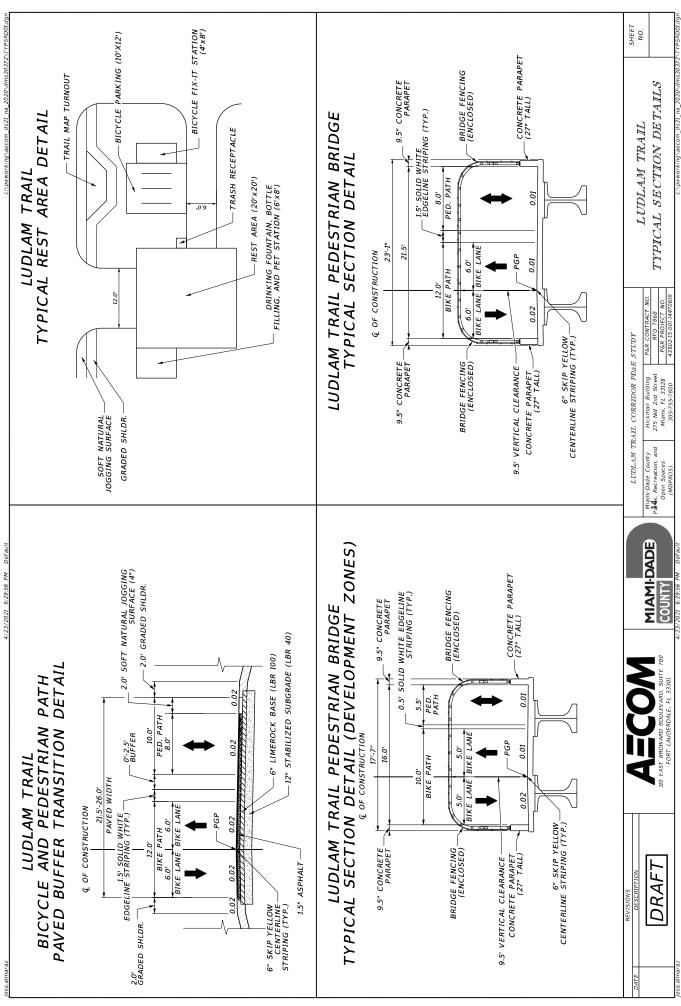




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